

Broadfield Park Slopes ('The Slopes'), Rochdale

Design & Access Statement Appendix: Volume II

For Rochdale Development Agency



Contents

Appendix A	Surface Water Sampling	3
	(Note that the Geotechnical Design Report & Ground Investigation Report are available on request due to size limitations)	
Appendix B	Topo Survey	42
Appendix C	CCTV Drainage Survey	46
Appendix D	Hydrology	63

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An aerial photograph of a city street scene, overlaid with a semi-transparent blue filter. In the center, a large, historic church with a prominent clock tower and Gothic-style architecture stands on a corner. To the right, a modern, multi-story building with a flat roof and many windows is visible. The street is wide and has several lanes, with some vehicles and a bus visible. The overall scene is urban and well-maintained.

Appendix A

Geological Investigations

Our Ref: 372700-L01 (01)

20th December 2022

Price & Myers
Beehive Lofts,
Jersey Street
Manchester
M4 6JG

For the attention of Will York

Dear Will,

RE: BROADFIELD PARK SLOPES, ROCHDALE – SURFACE WATER SAMPLING

1. INTRODUCTION

On the instructions of Price and Myers, on behalf of Rochdale Development Agency (the Client), RSK Environment Limited (RSK) has carried out surface water sampling at Broadfield Park Slopes, The Esplanade, Rochdale, OL16 1AQ.

The site comprises Broadfield Park Slopes (“the Slopes”), a section of the Grade-II listed Victorian-era Broadfield Park which is located adjacent south of the Victorian Grade-I listed Rochdale Town Hall (the “Town Hall”) in the centre of Rochdale, Greater Manchester.

The Slopes and the adjacent Town Hall are currently subject to an ongoing regeneration project which aspires to return the site to its former status as a vibrant public space; including improvements to access, creating safe and desirable landscaped areas, restoring views between the surrounding historic buildings and rectifying drainage issues.

As part of the project, it is understood that consideration is being given to the reinstatement of Packer Spout as a public amenity. To enable this option to be considered in further detail, RSK was commissioned to undertake preliminary surface water sampling to provide an indicative assessment of the water quality.

2. LIMITATIONS

The comments given in this report and the opinions expressed are based on the conditions encountered during the site work and on the results of tests made in the laboratory. However, there may be conditions pertaining to the site that have not been disclosed by the investigation and therefore could not be taken into account. In particular, it should be noted that groundwater levels



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flows may vary from those reported due to seasonal, or other, effects and the limitations stated in the data should be recognised.

This report should be considered in the light of any changes in legislation, statutory requirement or industry practices that have occurred following the date of issue.

This report is subject RSK’s service constraints given in [Appendix A](#).

3. PREVIOUS REPORTS

The following reports detailing previous works at and within the immediate vicinity of the site were made available for review:

- Geotechnical & Environmental Associates Limited (GEA), “Desk Study and Ground Investigation Report: Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AZ”, ref.: J19177, dated November 2019
- Ambellia Consultancy, “Initial Hydrology Report: Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AZ”, rev.: 00, dated September 2021
- Ambellia Consultancy, “Report on April 2022 RTH Slopes Water Flow Investigations: Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AZ”, rev.: 00, May 2022
- Ambellia Consultancy, “Interim Report, Park Slopes Water Flow Investigations: Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AZ”, rev.: 00, June 2022

In addition, RSK has previously undertaken geotechnical desk study and intrusive investigation at the Broadfield Park Slopes site:

- RSK, Geotechnical Desk Study – Broadfield Park Slopes, The Esplanade, Rochdale, OL16 1A, report reference 372700-01 (00) dated October 2022.
- RSK, Ground Investigation Report – Broadfield Park Slopes, The Esplanade, Rochdale, OL16 1A, report reference 372700-02 (00) dated November 2022.

The above reports should be considered in conjunction with this letter report.

4. SITE DESCRIPTION AND SURFACE WATER SETTING

The Slopes cover an area of approximately 0.97Ha, ranging in elevation from 142.12m above Ordnance Datum (AOD) adjacent to St. Chad’s Church in the south, to 121.24m AOD at the boundary with the Town Hall car park in the north. A plan showing the location of the site is presented as **Figure 1**.

Vegetation on the slopes was poorly maintained and had become heavily overgrown in recent years. The site has recently been subject to an extensive programme of vegetation clearance, undertaken by the contractor as part of the enabling works for the regeneration project, leaving the



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Broadfield Park Slopes, The Esplanade, Rochdale, OL16 1AQ
372700 L01 (01)

Page 2 of 6



surface of the slope generally covered with scrub vegetation, which has re-established following the recent clearance works.

The nearest offsite surface watercourse is the River Roch, which flows in a southwesterly direction 30m to the north of the site. It is culverted approximately 6m below ground level, where it flows parallel with The Esplanade (i.e. immediately north of the Town Hall), however flows in an open channel at either end of the culvert.

Grade-II* listed St. Chad's Church sits on a plateau at the top of the Slopes and is retained at an elevation of approximately 141.78m AOD. Runoff from the church yard at St. Chad's is channelled via a masonry surface drain which runs beneath the church yard retaining wall and onto the Slopes, where it currently collects to form a pond (elevation approximately 130.88m AOD). Wetland vegetation is established around the margin of the pond suggesting that this is a permanent feature.

Packer Spout, a semi-oval shaped stone reservoir, is present in the east of the Slopes. Packer Spout was originally constructed in 1760 to collect spring water before distributing it to buildings in Packer Street, Fleece Street, and Church Lane.

Investigations undertaken by Ambellia (2021, 2022a, 2022b) have established the presence of 2no. spring lines within the current study site, which sit within the granular superficial aquifer (Glaciofluvial Sand and Gravel) at elevations of approximately 131m and 127m AOD respectively.

Water issuing from the spring lines, along with runoff from the above catchment areas (St. Chad's Church yard and Parsonage House), is channelled downslope by various different mechanisms. The site has been subject to an extensive and complicated history of water management, and a variety of different drainage systems have been used, all of which are understood to remain, though in varying states of disrepair.

A detailed inspection of the existing drainage systems is outside the scope of this report, and the drainage and water management systems within the slopes are the subject of an ongoing assessment by Ambellia, as described at length in the reports listed in Section 3.

5. GROUNDWATER

RSK have recently undertaken an intrusive investigation of the site, as reported in Ground Investigation Report, ref.: 372700-02 (00), November 2022.

Within the Ground Investigation Report the site was zoned on the basis of the spatial variation in ground conditions and topography encountered.

Within Zone A (Ground Model 1), groundwater springs are absent, considered to be due to the absence of cohesive units within the slope. However, groundwater is present at approximately 120 m AOD at the toe of the slope, rising to ~121.50 mAOD in line with the crest of the slope, and is

inferred to continue rising southward to an elevation of ~132 m AOD, following the local topography and consistent with the findings of borehole CP01.

Within Zones B and C (Ground Model 2) two groundwater spring lines are noted within the Glaciofluvial Deposits at around 132 m and 125 mAOD. Groundwater is present at approximately 120 m AOD at the toe of the slope, rising to ~132 mAOD within the upper portion of the slope (coincident with the reported level of spring line) and is considered to remain at around this elevation to approximately in line with the crest of the slope (in line with borehole CP01). It is inferred to continue rising southward to an elevation of ~135 mAOD, following the local topography.

Groundwater within the Pennine Coal Measures may be taken as being at approximately 109.85 m AOD based on the highest groundwater strike in this stratum results of the GEA investigation for all ground models.

6. SURFACE WATER SAMPLING

Two surface water samples (SW1 and SW2) were taken by RSK on the 7th September 2022 at the locations shown in **Figure 2**. The sampling locations were specified by Ambellia prior to RSK's attendance at the site.

Collection of a third sample (SW3) was attempted from a manhole within the grounds of Partnership House but this was found to be dry.

Samples were collected in containers appropriate to the anticipated testing suite required. The containers were filled to capacity and placed in a cool box to minimise volatilisation and dispatched to the laboratory in cool boxes under chain of custody documentation.

Chemical testing undertaken on water samples obtained is presented in **Table 1** with the laboratory testing results contained in [Appendix B](#).

Table 1 Summary of chemical testing of water samples

Sample type	Tests undertaken	No. of tests
Surface water	Dissolved metals (Al, Ca, Fe, Mg, Sb, As, B, Cd, Mn, Cr, Cu, Na, Pb, Ni, Se, Hg)	2
	pH and sulphur (SO4)	2
	Dissolved Solids	2
	Total Coliform & E. Coli	2
	Ammonia	2
	Nitrite, Nitrate, Chloride, Total Oxidised Nitrogen (NO3)	2
	Volatile Petroleum Hydrocarbon (VPH) and Extractable Petroleum Hydrocarbon (EPH)	2



	Polycyclic Aromatic Hydrocarbon (PAH)	2*
	Hardness	2

*The results for Polycyclic Aromatic Hydrocarbons (PAHs) in sample SW1 fell outside the laboratories QC check and the sample was sent back for repeat analysis.

7. GEO-ENVIRONMENTAL ASSESSMENT

7.1 Introduction

As noted in Section 1 of this report, the Slopes and the adjacent Town Hall are currently subject to an ongoing regeneration project which aspires to return the site to its former status as a vibrant public space. As part of the project it is understood that consideration is being given to the reinstatement of Packer Spout as a public amenity. To enable this option to be considered in further detail, RSK was commissioned to undertake preliminary surface water sampling to provide an indicative assessment of the water quality.

At this stage, a limited scope of sampling and assessment has been undertaken (i.e. 2no. samples). It should therefore be appreciated that the comments made within this report are of a preliminary nature, and that although the findings may be used to inform an initial appraisal of potential options and constraints, further sampling and analysis would be required to provide a more detailed examination of the water quality, should the reinstatement of Packer Spout proceed.

7.2 Assessment

On the basis of the potential future use of the water supply as a public amenity, there is the potential for a complete pollutant linkage via direct contact or ingestion of impacted water, intentional or otherwise.

The analytical results for the two water samples have therefore been directly assessed against Drinking Water Standards (DWS). The Generic Assessment Criteria (GAC) for DWS have been presented in [Appendix C](#).

The results of the direct comparison undertaken indicate a number of exceedances above the assessment criteria, as summarised in Table 2 below.

Table 2 Summary of surface water GQRA results

Determinand	GAC (DWS) (µg/l)	Surface water exceedances (sampling location and concentration µg/l)
Total Coliform presumptive	0	SW1 (29) & SW2 (>100)
Total Coliforms confirmed	0	SW1 (29) & SW2 (>100)
E.coli presumptive	0	SW2 (>100)

Determinand	GAC (DWS) (µg/l)	Surface water exceedances (sampling location and concentration µg/l)
E. coli confirmed	0	SW2 (>100)
Total Iron	200	SW2 (1330)
Total Lead	10	SW2 (33.5)
pH	6.5 to 9.5	SW1 (6.37)

From the above, it can be seen that Total Coliforms and E. Coli were detected in sample SW2 and Total Coliforms were detected in sample SW1. Sample SW2 also had elevated metal concentrations (iron and lead) in excess of the DWS. The pH of SW1 also fell marginally short of the required DWS range.

Based on the limited testing undertaken to date, the presence of Total Coliform, E. coli and heavy metals indicate that the sampled waters do not comprise a suitable drinking water resource without prior treatment.

On the basis of the above results, consideration should be given to management of the up-slope water system to determine whether improvements to the water quality can be made. Further consideration may otherwise be given to the use of Packer Spout for resources other than drinking water. Following a review of the potential options, it is recommended that further testing of the water sources to determine any trends in long term water quality is undertaken.

Yours sincerely,

Claire Siberry
Senior Geotechnical Engineer

Rowan Brown
Principal Geotechnical Engineer

Encl.

Figure 1

Site location plan

Figure 2

Exploratory hole location plan

[Appendix A](#)

Service Constraints

[Appendix B](#)

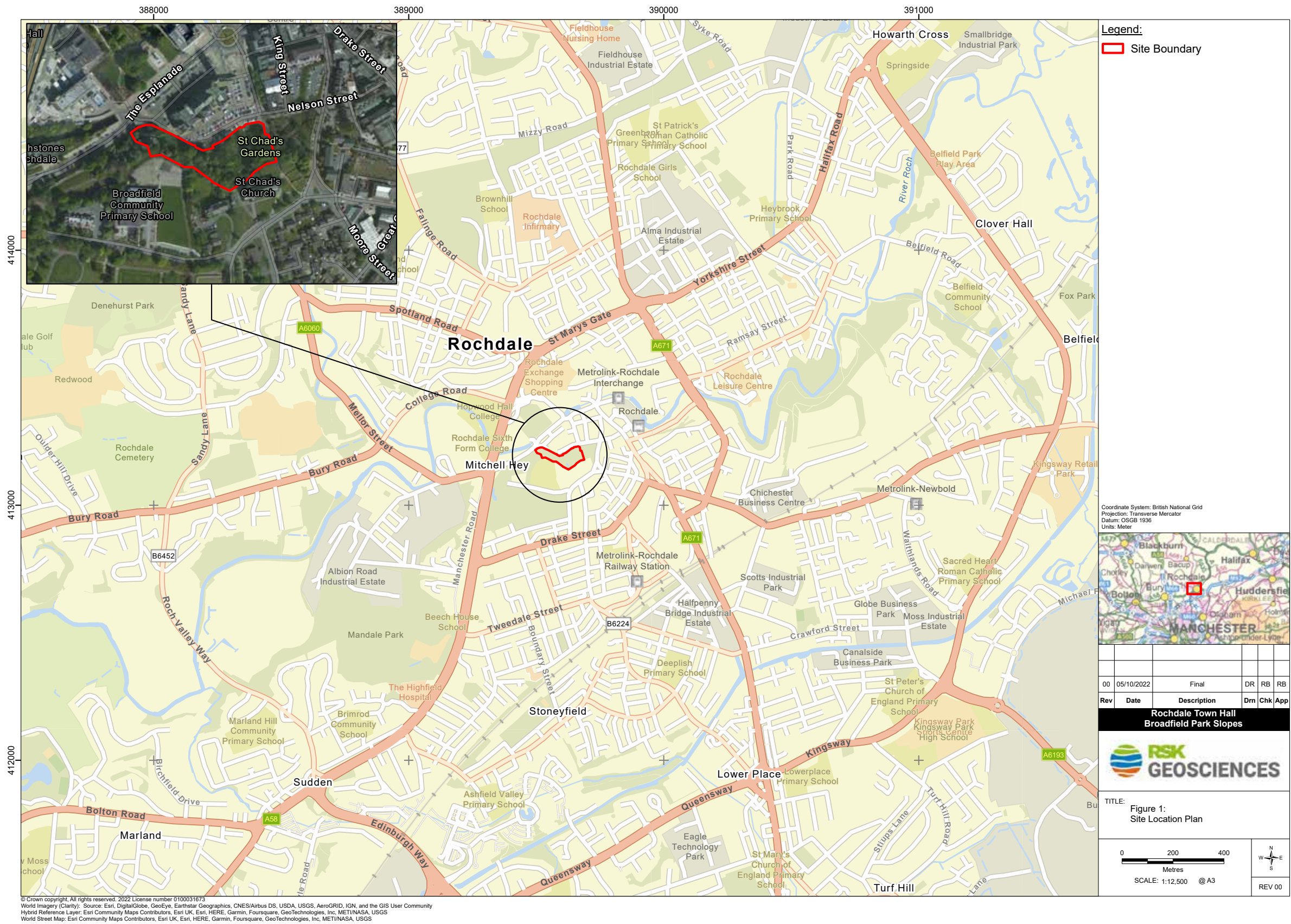
Laboratory Certificates for Surface Water Analysis

[Appendix C](#)

Generic Assessment Criteria for Controlled Waters



FIGURES





SAMPLING LOCATION PLAN

Client: Rochdale Development Agency

Site: Broadfield Park Slopes, Rochdale

Scale: NTS

Figure No: 2

Job No: 372700

Source: Dwg 30217-SK-CIV-01 P01



APPENDIX A Service Constraints

1. This report and the site investigation carried out in connection with the report (together the "Services") were compiled and carried out by RSK Environment Limited (RSK) for Rochdale Development Agency (the "Client") in accordance with the terms of a contract between RSK and the Client, dated 31st August 2022. The Services were performed by RSK with the reasonable skill and care ordinarily exercised by an environmental consultant at the time the Services were performed. Further, and in particular, the Services were performed by RSK taking into account the limits of the scope of works required by the client, the time scale involved and the resources, including financial and manpower resources, agreed between RSK and the Client.
2. Other than that, expressly contained in paragraph 1 above, RSK provides no other representation or warranty whether express or implied, in relation to the Services.
3. Unless otherwise agreed in writing, the Services were performed by RSK exclusively for the purposes of the Client. RSK is not aware of any interest of or reliance by any party other than the Client in or on the Services. Unless expressly provided in writing, RSK does not authorise, consent or condone any party other than the client relying upon the Services. Should this report or any part of this report, or otherwise details of the Services or any part of the Services be made known to any such party, and such party relies thereon that party does so wholly at its own and sole risk and RSK disclaims any liability to such parties. **Any such party would be well advised to seek independent advice from a competent environmental consultant and/or lawyer.**
4. It is RSK's understanding that this report is to be used for the purpose described in the introduction to the report. That purpose was a significant factor in determining the scope and level of the Services. Should the purpose for which the report is used, or the proposed use of the site change, this report may no longer be valid and any further use of or reliance upon the report in those circumstances by the client without RSK's review and advice shall be at the client's sole and own risk. Should RSK be requested to review the report after the date of this report, RSK shall be entitled to additional payment at the then existing rates or such other terms as agreed between RSK and the client.
5. The passage of time may result in changes in site conditions, regulatory or other legal provisions, technology or economic conditions which could render the report inaccurate or unreliable. The information and conclusions contained in this report should not be relied upon in the future without the written advice of RSK. In the absence of such written advice of RSK, reliance on the report in the future shall be at the Client's own and sole risk. Should RSK be requested to review the report in the future, RSK shall be entitled to additional payment at the then existing rate or such other terms as may be agreed between RSK and the client.
6. The observations and conclusions described in this report are based solely upon the Services which were provided pursuant to the agreement between the Client and RSK. RSK has not performed any observations, investigations, studies or testing not specifically set out or required by the contract between the client and RSK. RSK is not liable for the existence of any condition, the discovery of which would require performance of services not otherwise contained in the Services. For the avoidance of doubt, unless otherwise expressly referred to in the introduction to this report, RSK did not seek to evaluate the presence on or off site of asbestos, invasive plants, electromagnetic fields, lead paint, heavy metals, radon gas, persistent, bioaccumulative or toxic chemicals (including PFAS compounds) or other radioactive or hazardous materials, unless specifically identified in the Services.

7. The Services are based upon RSK's observations of existing physical conditions at the Site gained from a visual inspection of the site together with RSK's interpretation of information, including documentation, obtained from third parties and from the Client on the history and usage of the site, unless specifically identified in the Services or accreditation system (such as UKAS ISO 17020:2012 clause 7.1.6):
 - a. The Services were based on information and/or analysis provided by independent testing and information services or laboratories upon which RSK was reasonably entitled to rely.
 - b. The Services were limited by the accuracy of the information, including documentation, reviewed by RSK and the observations possible at the time of the visual inspection.
 - c. The Services did not attempt to independently verify the accuracy or completeness of information, documentation or materials received from the client or third parties, including laboratories and information services, during the performance of the Services.

RSK is not liable for any inaccurate information or conclusions, the discovery of which inaccuracies required the doing of any act including the gathering of any information which was not reasonably available to RSK and including the doing of any independent investigation of the information provided to RSK save as otherwise provided in the terms of the contract between the Client and RSK.

8. The intrusive environmental site investigation aspects of the Services are a limited sampling of the site at pre-determined locations based on the known historic / operational configuration of the site. The conclusions given in this report are based on information gathered at the specific test locations and can only be extrapolated to an undefined limited area around those locations. The extent of the limited area depends on the properties of the materials adjacent and local conditions, together with the position of any current structures and underground utilities and facilities, and natural and other activities on site. In addition, chemical analysis was carried out for a limited number of parameters (as stipulated in the scope between the client and RSK, based on an understanding of the available operational and historical information) and it should not be inferred that other chemical species are not present.
9. Any site drawing(s) provided in this report is (are) not meant to be an accurate base plan but is (are) used to present the general relative locations of features on, and surrounding, the site. Features (intrusive and sample locations etc) annotated on site plans are not drawn to scale but are centred over the approximate location. Such features should not be used for setting out and should be considered indicative only.
10. The comments given in this report and the opinions expressed are based on the ground conditions encountered during the site work and on the results of tests made in the field and in the laboratory. However, there may be conditions pertaining to the site that have not been disclosed by the investigation and therefore could not be taken into account. In particular, it should be noted that there may be areas of made ground not detected due to the limited nature of the investigation or the thickness and quality of made ground across the site may be variable. In addition, groundwater levels and ground gas concentrations and flows, may vary from those reported due to seasonal, or other, effects and the limitations stated in the data should be recognised.
11. Asbestos is often observed to be present in soils in discrete areas. Whilst asbestos-containing materials may have been locally encountered during the fieldworks or supporting laboratory analysis, the history of brownfield and demolition sites indicates that asbestos fibres may be present more widely in soils and aggregates, which could be encountered during more extensive ground works.
12. Unless stated otherwise, only preliminary geotechnical recommendations are presented in this report and these should be verified in a Geotechnical Design Report, once proposed construction and structural design proposals are confirmed.



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APPENDIX B

Laboratory Certificates for Surface Water Analysis

28 September 2022

Test Report: COV/2369621/2022

Dear Ms Siberry

Analysis of your sample(s) submitted on 07 September 2022 is now complete and we have pleasure in enclosing the appropriate test report(s).

An invoice for the analysis carried out will be sent under separate cover.

Should you have any queries regarding this report(s) or any part of our service, please contact Customer Services on +44 (0)24 7642 1213 who will be happy to discuss your requirements.

If you would like to arrange any further analysis, please contact Customer Services. To arrange container delivery or sample collection, please call the Couriers Department directly on 024 7685 6562.

Thank you for using ALS Environmental Ltd and we look forward to receiving your next samples.

Yours Sincerely,

Signed: *Elizabeth Parker*

Name: E. Parker

Title: Potable Micro Team Leader



1314



EMS675527

OHS 542058



This communication has been sent to you by ALS Environmental Ltd. Registered in England and Wales. Registration No.02148934. Registered Office: ALS Environmental Limited, Torrington Avenue, Coventry, CV4 9GU.

Report Summary



Ms Claire Siberry
RSK Geosciences
18 Frogmore Road
Hertfordshire
Hemel Hempstead
HP3 9RT

Date of Issue: 28 September 2022

Report Number: COV/2369621/2022

Issue 1

This issue replaces
all previous issues

Number of Samples
included in this report 2

Job Received: 07 September 2022

Number of Test Results
included in this report 122

Analysis Commenced: 08 September 2022

Signed: Elizabeth Parker.

Name: E. Parker

Date: 28 September 2022

Title: Potable Micro Team Leader

ALS Environmental Ltd was not responsible for sampling unless otherwise stated.
Information on the methods of analysis and performance characteristics are available on request.
Opinions and interpretations expressed herein are outside the scope of UKAS accreditation. The results relate only to the items tested and where relevant sampled.
Tests marked 'Not UKAS Accredited' in this Report/Certificate are not included in the UKAS Accreditation Schedule for our laboratory.
This test report is not a statement of conformity to any specification or standard.

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Certificate of Analysis



Report Number: COV/2369621/2022
Laboratory Number 22007132

Issue 1
Sample 1 of 2

Sample Source: RSK Geosciences
Sample Point Description: Drinking Water
Sample Description: SW1
Sample Matrix: Drinking Water

Sample Date: 07 September 2022 Sample Received 07 September 2022 Analysis Complete: 28 September 2022

Test Description	Result	Units	Limit	Accreditation	Method
EPH >C10 - C44	<10	ug/l		Y Cov	GEO46
VPH/EPH >C5 - C44	<10	ug/l		N Cov	GEO45/GEO46
Aromatic VPH/EPH >C5 - C44	<10	ug/l		N Cov	GEO45/GEO46
Aliphatic VPH/EPH >C5 - C44	<10	ug/l		N Cov	GEO45/GEO46
Hydrogen ion (pH)	6.37	pH units	6.5 to 9.5	Y Wak	WPC8
Ammonium ammonia+ammonium ion	<0.010	mg/l	0.5	Y Wak	WPC64
Nitrite as NO2	<0.003	mg/l	0.5	Y Wak	WPC64
Nitrate as NO3	17.0	mg/l	50	Y Wak	WPC64
Chloride as Cl	17.1	mg/l	250	Y Wak	WPC64
Aluminium, Total as Al	<6.90	ug/l	200	Y Wak	WPC12/49
Calcium, Total as Ca	24.8	mg/l		Y Wak	WPC12/49
Iron, Total as Fe	145	ug/l	200	Y Wak	WPC12/49
Magnesium, Total as Mg	1.46	mg/l		Y Wak	WPC12/49
Manganese, Total as Mn	26.8	ug/l	50	Y Wak	WPC12/49
Sodium, Total as Na	13.2	mg/l	200	Y Wak	WPC12/49
Antimony, Total as Sb	0.16	ug/l	5	Y Wak	WPC15
Arsenic, Total as As	0.23	ug/l	10	Y Wak	WPC15
Cadmium, Total as Cd	0.04	ug/l	5	Y Wak	WPC15
Chromium, Total as Cr	<0.18	ug/l	50	Y Wak	WPC15
Nickel, Total as Ni	1.50	ug/l	20	Y Wak	WPC15
Selenium, Total as Se	0.86	ug/l	10	Y Wak	WPC15
Mercury, Total as Hg	<0.06	ug/l	1	Y Wak	WPC21
Solids, Dissolved 180 Deg C	104	mg/l	1500	Y Wak	WPC25
Benzo(b)fluoranthene	Analyst Comment	ng/l		Y Wak	WPC27
Benzo(k)fluoranthene	Analyst Comment	ng/l		Y Wak	WPC27
Benzo(ghi)perylene	Analyst Comment	ng/l		Y Wak	WPC27
Benzo(a)pyrene	Analyst Comment	ng/l	10	Y Wak	WPC27
Fluoranthene	Analyst Comment	ng/l		Y Wak	WPC27
Indeno(1,2,3cd)pyrene	Analyst Comment	ng/l		Y Wak	WPC27
Total PAHs 4 Constituents	Analyst Comment	ug/l	0.1	Y Wak	WPC27
Hardness, Total as CaCO3	67.9	mg/l		Y Wak	WPC12/49
PAH Total ug/l	Analyst Comment	ug/l		N Wak	WPC27
Total Oxidised Nitrogen as NO3	17.0	mg/l	50	Y Wak	WPC64
Boron, Total as B	<0.0320	mg/l	1.0	Y Wak	WPC12/49

Certificate of Analysis



Report Number: COV/2369621/2022

Laboratory Number 22007133

Sample Source: RSK Geosciences

Sample Point Description: Drinking Water

Sample Description: SW2

Sample Matrix: Drinking Water

Sample Date: 07 September 2022

Sample Received 07 September 2022

Analysis Complete: 28 September 2022

Test Description	Result	Units	Limit	Accreditation	Method
Copper, Total as Cu	8.6	ug/l	2000	Y Wak	WPC15
Lead, Total as Pb	33.5	ug/l	10	Y Wak	WPC15
Sulphur, Total as SO4	27.8	mg/l	250	Y Wak	WPC12/49
Aliphatic EPH >C35 - C44	<10	ug/l		N Cov	GEO46
Aliphatic EPH >C10 - C44	<10	ug/l		Y Cov	GEO46
Aromatic EPH >C10 - C12	<10	ug/l		Y Cov	GEO46
Aromatic EPH >C12 - C16	<10	ug/l		Y Cov	GEO46
Aromatic EPH >C16 - C21	<10	ug/l		Y Cov	GEO46
Aromatic EPH >C21 - C35	<10	ug/l		N Cov	GEO46
Aromatic EPH >C35 - C44	<10	ug/l		N Cov	GEO46
VPH >C5 - C10	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C5 - C6	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C6 - C8	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C5 - C10	<10	ug/l		N Cov	GEO45
Aromatic VPH >C5 - C7	<10	ug/l		N Cov	GEO45
Aromatic VPH >C7 - C8	<10	ug/l		N Cov	GEO45
Aromatic VPH >C8 - C10	<10	ug/l		N Cov	GEO45
Aromatic VPH >C5 - C10	<10	ug/l		N Cov	GEO45
Aliphatic EPH >C10 - C12	<10	ug/l		Y Cov	GEO46
Aliphatic EPH >C12 - C16	<10	ug/l		Y Cov	GEO46
Aromatic EPH >C10 - C44	<10	ug/l		Y Cov	GEO46
Aliphatic EPH >C16 - C35	<10	ug/l		N Cov	GEO46
Aliphatic VPH >C8 - 10	<10	ug/l		N Cov	GEO45
Total Coliforms confirmed	>100	cfu/100ml	0	Y Cov	W10
Escherichia coli confirmed	>100	cfu/100ml	0	Y Cov	W10
Total Coliform presumpt	>100	cfu/100ml	0	Y Cov	W10
E.coli presumptive	>100	cfu/100ml	0	Y Cov	W10

Analyst Comments for 22007133:

Confirmation process not been carried out for coliforms due to nature of the sample. Confirmation process not been carried out for coliforms due to nature of the sample. {(*)}Pb result confirmed by 33.6ug/l
Al result confirmed by 276ug/l
Fe result confirmed by 1340ug/l{(*)}

This issue replaces all previous issues

Accreditation Codes: Y = UKAS / ISO17025 Accredited, N = Not UKAS / ISO17025 Accredited, M = MCERTS.

Analysed at: CHE = Chester(CH5 3US), COV = Coventry(CV4 9GU), OTT = Otterbourne(SO21 2RU), S = Subcontracted, TRB = Subcontracted to Trowbridge(BA14 0XD), WAK = Wakefield(WF5 9TG),

F = Data supplied by customer.

For Microbiological determinands 0 or ND=Not Detected, For Legionella ND=Not Detected in volume of sample filtered.

I/S=insufficient sample For soil/sludge samples: AR=As received, DW=Dry weight.

Signed: Elizabeth Parker.

Name: E. Parker

Date: 28 September 2022

Title: Potable Micro Team Leader

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**Ms Siberry
RSK Geosciences
18 Frogmore Road
Hertfordshire
Hemel Hempstead HP3 9RT**



ALS Environmental Ltd
Torrington Avenue
Coventry
CV4 9GU

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F: +44 (0)24 7685 6575
www.alsenvironmental.co.uk

28 September 2022

Test Report: COV/2369621/2022

Dear Ms Siberry

Analysis of your sample(s) submitted on 07 September 2022 is now complete and we have pleasure in enclosing the appropriate test report(s).

An invoice for the analysis carried out will be sent under separate cover.

Should you have any queries regarding this report(s) or any part of our service, please contact Customer Services on +44 (0)24 7642 1213 who will be happy to discuss your requirements.

If you would like to arrange any further analysis, please contact Customer Services. To arrange container delivery or sample collection, please call the Couriers Department directly on 024 7685 6562.

Thank you for using ALS Environmental Ltd and we look forward to receiving your next samples.

Yours Sincerely,

Signed: *Elizabeth Parker.*

Name: E. Parker

Title: Potable Micro Team Leader



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Report Summary

**Ms Claire Siberry
RSK Geosciences
18 Frogmore Road
Hertfordshire
Hemel Hempstead
HP3 9RT**

ANALYSED BY



Date of Issue: **28 September 2022**

Report Number: **COV/2369621/2022**

Issue **1**

This issue replaces
all previous issues

Job Description: Drinking Waters Testing

Job Location: 18 FROGMORE RD. HP2 9RT

Number of Samples
included in this report **2**

Job Received: **07 September 2022**

Number of Test Results
included in this report **122**

Analysis Commenced: **08 September 2022**

Signed: *Elizabeth Parker.*

Name: **E. Parker**

Date: **28 September 2022**

Title: **Potable Micro Team Leader**

ALS Environmental Ltd was not responsible for sampling unless otherwise stated.

Information on the methods of analysis and performance characteristics are available on request.

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation. The results relate only to the items tested and where relevant sampled.

Tests marked 'Not UKAS Accredited' in this Report/Certificate are not included in the UKAS Accreditation Schedule for our laboratory. This test report is not a statement of conformity to any specification or standard.

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Page 1 of 12

Certificate of Analysis

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Report Number: COV/2369621/2022 Issue 1
Laboratory Number: 22007132 Sample 1 of 2
Sample Source: RSK Geosciences
Sample Point Description: Drinking Water
Sample Description: SW1
Sample Matrix: Drinking Water
Sample Date/Time: 07 September 2022 12:00
Sample Received: 07 September 2022
Analysis Complete: 28 September 2022

Test Description	Result	Units	Analysis Date	Accreditation	Method
Hardness, Total as CaCO3	67.9	mg/l	08/09/2022	Y Wak	WPC12/49
PAH Total ug/l	Analyst Com	ug/l	28/09/2022	N Wak	WPC27
Sulphur, Total as SO4	18.3	mg/l	08/09/2022	Y Wak	WPC12/49
Total Coliform presump	29	cfu/100ml	09/09/2022	Y Cov	W10
Total Coliforms confirmed	29	cfu/100ml	09/09/2022	Y Cov	W10
E.coli presumptive	0	cfu/100ml	09/09/2022	Y Cov	W10
Escherichia coli confirmed	0	cfu/100ml	09/09/2022	Y Cov	W10
Aluminium, Total as Al	<6.90	ug/l	08/09/2022	Y Wak	WPC12/49
Calcium, Total as Ca	24.8	mg/l	08/09/2022	Y Wak	WPC12/49
Iron, Total as Fe	145	ug/l	08/09/2022	Y Wak	WPC12/49
Magnesium, Total as Mg	1.46	mg/l	08/09/2022	Y Wak	WPC12/49
Antimony, Total as Sb	0.16	ug/l	08/09/2022	Y Wak	WPC15
Arsenic, Total as As	0.23	ug/l	08/09/2022	Y Wak	WPC15
Boron, Total as B	<0.0320	mg/l	08/09/2022	Y Wak	WPC12/49
Manganese, Total as Mn	26.8	ug/l	08/09/2022	Y Wak	WPC12/49
Cadmium, Total as Cd	0.04	ug/l	08/09/2022	Y Wak	WPC15
Chromium, Total as Cr	<0.18	ug/l	08/09/2022	Y Wak	WPC15
Sodium, Total as Na	13.2	mg/l	08/09/2022	Y Wak	WPC12/49
Copper, Total as Cu	0.69	ug/l	08/09/2022	Y Wak	WPC15
Lead, Total as Pb	0.16	ug/l	08/09/2022	Y Wak	WPC15
Nickel, Total as Ni	1.50	ug/l	08/09/2022	Y Wak	WPC15
Selenium, Total as Se	0.86	ug/l	08/09/2022	Y Wak	WPC15
Hydrogen ion (pH)	6.37	pH units	08/09/2022	Y Wak	WPC8

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Report Number: COV/2369621/2022 Issue 1
Laboratory Number: 22007132 Sample 1 of 2
Sample Source: RSK Geosciences
Sample Point Description: Drinking Water
Sample Description: SW1
Sample Matrix: Drinking Water
Sample Date/Time: 07 September 2022 12:00
Sample Received: 07 September 2022
Analysis Complete: 28 September 2022

Test Description	Result	Units	Analysis Date	Accreditation	Method
Solids, Dissolved 180 Deg C	104	mg/l	09/09/2022	Y Wak	WPC25
Ammonium ammonia+ammonium ion	<0.010	mg/l	08/09/2022	Y Wak	WPC64
Nitrite as NO2	<0.003	mg/l	08/09/2022	Y Wak	WPC64
Nitrate as NO3	17.0	mg/l	08/09/2022	Y Wak	WPC64
Chloride as Cl	17.1	mg/l	08/09/2022	Y Wak	WPC64
Total Oxidised Nitrogen as NO3	17.0	mg/l	08/09/2022	Y Wak	WPC64
Mercury, Total as Hg	<0.06	ug/l	08/09/2022	Y Wak	WPC21
Aliphatic VPH >C5 - C6	<10	ug/l	16/09/2022	N Cov	GEO45
Aliphatic VPH >C6 - C8	<10	ug/l	16/09/2022	N Cov	GEO45
Aliphatic VPH >C8 - 10	<10	ug/l	16/09/2022	N Cov	GEO45
Aliphatic VPH >C5 - C10	<10	ug/l	16/09/2022	N Cov	GEO45
Aromatic VPH >C5 - C7	<10	ug/l	16/09/2022	N Cov	GEO45
Aromatic VPH >C7 - C8	<10	ug/l	16/09/2022	N Cov	GEO45
Aromatic VPH >C8 - C10	<10	ug/l	16/09/2022	N Cov	GEO45
Aromatic VPH >C5 - C10	<10	ug/l	16/09/2022	N Cov	GEO45
VPH >C5 - C10	<10	ug/l	16/09/2022	N Cov	GEO45
Aliphatic EPH >C10 - C12	<10	ug/l	14/09/2022	Y Cov	GEO46
Aliphatic EPH >C12 - C16	<10	ug/l	14/09/2022	Y Cov	GEO46
Aliphatic EPH >C16 - C35	<10	ug/l	14/09/2022	N Cov	GEO46
Aliphatic EPH >C35 - C44	<10	ug/l	14/09/2022	N Cov	GEO46
Aliphatic EPH >C10 - C44	<10	ug/l	14/09/2022	Y Cov	GEO46
Aromatic EPH >C10 - C12	<10	ug/l	14/09/2022	Y Cov	GEO46
Aromatic EPH >C12 - C16	<10	ug/l	14/09/2022	Y Cov	GEO46
Aromatic EPH >C16 - C21	<10	ug/l	14/09/2022	Y Cov	GEO46

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ANALYSED BY

Issue 1 of 2

Sample 1 of 2

Test Description	Result	Units	Analysis Date	Accreditation	Method
Aromatic EPH >C21 - C35	<10	ug/l	14/09/2022	N Cov	GEO46
Aromatic EPH >C35 - C44	<10	ug/l	14/09/2022	N Cov	GEO46
Aromatic EPH >C10 - C44	<10	ug/l	14/09/2022	Y Cov	GEO46
EPH >C10 - C44	<10	ug/l	14/09/2022	Y Cov	GEO46
Aliphatic VPH/EPH >C5 - C44	<10	ug/l	16/09/2022	N Cov	GEO45/GEO46
Aromatic VPH/EPH >C5 - C44	<10	ug/l	16/09/2022	N Cov	GEO45/GEO46
VPH/EPH >C5 - C44	<10	ug/l	16/09/2022	N Cov	GEO45/GEO46
Benzo(b)fluoranthene	Analyst Com	ng/l	28/09/2022	Y Wak	WPC27
Benzo(k)fluoranthene	Analyst Com	ng/l	28/09/2022	Y Wak	WPC27
Benzo(ghi)perylene	Analyst Com	ng/l	28/09/2022	Y Wak	WPC27
Benzo(a)pyrene	Analyst Com	ng/l	28/09/2022	Y Wak	WPC27
Fluoranthene	Analyst Com	ng/l	28/09/2022	Y Wak	WPC27
Indeno(1,2,3cd)pyrene	Analyst Com	ng/l	28/09/2022	Y Wak	WPC27
Total PAHs 4 Constituents	Analyst Com	ug/l	28/09/2022	Y Wak	WPC27

Analyst Comments for 22007132: {*/}pH result confirmed with a repeat result of 6.35.
WPC27 - PAH - Sample sent back for repeat analysis due to Internal standard recovery failure. however no sample remaining for further analysis.{*/} Total coliforms identified as *Serratia fonticola*, *Buttiauxella* species, *Kluyvera* species, *Lelliottia amnigena*.

This issue replaces all previous issues
Accreditation Codes: Y = UKAS / ISO17025 Accredited, N = Not UKAS / ISO17025 Accredited, M = MCERTS.
Analysed at: CHE = Chester (CH5 3US), COV = Coventry (CV4 9GU), OTT = Otterbourne (SO21 2RU), S = Subcontracted, TRB = Subcontracted to Trowbridge (BA14 0XD), WAK = Wakefield (WF5 9TG).
F = Data supplied by customer.
For Microbiological determinands 0 or ND= Not Detected, For Legionella ND=Not Detected in volume of sample filtered.
I/S=Insufficient sample For soil/sludge samples: AR=As received, DW=Dry weight.

Signed: Elizabeth Parker.

Name: **E. Parker** Date: **28 September 2022**

Title: **Potable Micro Team Leader**

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Page 4 of 12

Page 5 of 12

Certificate of Analysis

ANALYSED BY

UKAS

TESTING

1314

ALS

Report Number:COV/2369621/2022

Laboratory Number:22007133

Sample Source:RSK Geosciences

Sample Point Description:Drinking Water

Sample Description:SW2

Sample Matrix:Drinking Water

Sample Date/Time:07 September 202211:30

Sample Received:07 September 2022

Analysis Complete:28 September 2022

Issue1

Sample2of2

Test Description	Result	Units	Analysis Date	Accreditation	Method
Aromatic EPH >C21 - C35	<10	ug/l	14/09/2022	N Cov	GEO46
Aromatic EPH >C35 - C44	<10	ug/l	14/09/2022	N Cov	GEO46
Aromatic EPH >C10 - C44	<10	ug/l	14/09/2022	Y Cov	GEO46
EPH >C10 - C44	<10	ug/l	14/09/2022	Y Cov	GEO46
Aliphatic VPH/EPH >C5 - C44	<10	ug/l	16/09/2022	N Cov	GEO45/GEO46
Aromatic VPH/EPH >C5 - C44	<10	ug/l	16/09/2022	N Cov	GEO45/GEO46
VPH/EPH >C5 - C44	<10	ug/l	16/09/2022	N Cov	GEO45/GEO46
Benzo(b)fluoranthene	1.02	ng/l	21/09/2022	Y Wak	WPC27
Benzo(k)fluoranthene	0.85	ng/l	21/09/2022	Y Wak	WPC27
Benzo(ghi)perylene	0.72	ng/l	21/09/2022	Y Wak	WPC27
Benzo(a)pyrene	1.33	ng/l	21/09/2022	Y Wak	WPC27
Fluoranthene	1.48	ng/l	21/09/2022	Y Wak	WPC27
Indeno(1,2,3cd)pyrene	1.19	ng/l	21/09/2022	Y Wak	WPC27
Total PAHs 4 Constituents	0.004	ug/l	21/09/2022	Y Wak	WPC27

Analyst Comments for 22007133:

Confirmation process not been carried out for coliforms due to nature of the sample. Confirmation process not been carried out for coliforms due to nature of the sample. {/**}Pb result confirmed by 33.6ug/l
Al result confirmed by 276ug/l
Fe result confirmed by 1340ug/l{/**}

This issue replaces all previous issues
Accreditation Codes: Y = UKAS / ISO17025 Accredited, N = Not UKAS / ISO17025 Accredited, M = MCERTS.
Analysed at: CHE = Chester(CH5 3US), COV = Coventry(CV4 9GU), OTT = Otterbourne(SO21 2RU), S = Subcontracted, TRB = Subcontracted to Trowbridge(BA14 0XD), WAK = Wakefield(WF5 9TG),
F = Data supplied by customer.
For Microbiological determinands 0 or ND=Not Detected, For Legionella ND=Not Detected in volume of sample filtered.
I/S=Insufficient sample For soil/sludge samples: AR=As received, DW=Dry weight.

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Page 8 of 12

Signed: Elizabeth Parker.

Name: E. Parker

Date: 28 September 2022

Title: Potable Micro Team Leader

Page 9 of 12

Donald Insall Associates

20



ANALYST COMMENTS FOR REPORT COV/2369621/2022

Issue 1

This issue replaces all previous issues

Date of Issue: 28 September 2022

Sample No	Analysis Comments
22007132	{/*}pH result confirmed with a repeat result of 6.35. WPC27 - PAH - Sample sent back for repeat analysis due to Internal standard recovery failure. however no sample remaining for further analysis.{*/} Total coliforms identified as Serratia fonticola, Buttiauxella species, Kluyvera species, Lelliottia amnigena.
22007133	Confirmation process not been carried out for coliforms due to nature of the sample. Confirmation process not been carried out for coliforms due to nature of the sample. {*/}Pb result confirmed by 33.6ug/l Al result confirmed by 276ug/l Fe result confirmed by 1340ug/l{*/}

Signed: Elizabeth Parker.

Name: E. Parker

Date: 28 September 2022

Title: Potable Micro Team Leader



DETERMINAND COMMENTS FOR REPORT COV/2369621/2022

ISSUE 1

This issue replaces all previous issues

Date of Issue: 28 September 2022

Sample No	Description	Determinand	Comments
22007132	SW1	Total PAHs 4 Constituents	{/*}WPC27 - PAH - Sample sent back for repeat analysis due to Internal standard recovery failure. however no sample remaining for further analysis.{*/}
22007132	SW1	PAH Total ug/l	{/*}WPC27 - PAH - Sample sent back for repeat analysis due to Internal standard recovery failure. however no sample remaining for further analysis.{*/}
22007132	SW1	Total Coliforms confirmed	Total coliforms identified as Serratia fonticola, Buttiauxella species, Kluyvera species, Lelliottia amnigena.
22007133	SW2	Total Coliforms confirmed	Confirmation process not been carried out for coliforms due to nature of the sample.
22007133	SW2	Total Coliform presumpt	Confirmation process not been carried out for coliforms due to nature of the sample.

Signed: Elizabeth Parker.

Name: E. Parker

Date: 28 September 2022

Title: Potable Micro Team Leader

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**Ms Siberry
RSK Geosciences
18 Frogmore Road
Hertfordshire
Hemel Hempstead HP3 9RT**



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20 December 2022

Test Report: COV/2371123/2022

Dear Ms Siberry

Analysis of your sample(s) submitted on 09 September 2022 is now complete and we have pleasure in enclosing the appropriate test report(s).

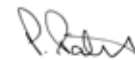
An invoice for the analysis carried out will be sent under separate cover.

Should you have any queries regarding this report(s) or any part of our service, please contact Customer Services on +44 (0)24 7642 1213 who will be happy to discuss your requirements.

If you would like to arrange any further analysis, please contact Customer Services. To arrange container delivery or sample collection, please call the Couriers Department directly on 024 7685 6562.

Thank you for using ALS Laboratories (UK) Limited and we look forward to receiving your next samples.

Yours Sincerely,

Signed: 

Name: P. Patel

Title: Inorganics Chemistry Manager



Report Summary



Ms Claire Siberry
RSK Geosciences
18 Frogmore Road
Hertfordshire
Hemel Hempstead
HP3 9RT

Date of Issue: **11 October 2022**

Report Number: **COV/2371123/2022**

Issue **1**

This issue replaces
all previous issues

Number of Samples
included in this report **2**

Job Received: **09 September 2022**

Number of Test Results
included in this report **25**

Analysis Commenced: **17 September 2022**

Signed:

Name: **P. Patel**

Date: **11 October 2022**

Title: **Inorganics Chemistry Manager**

ALS Laboratories (UK) Limited was not responsible for sampling unless otherwise stated.

Information on the methods of analysis and performance characteristics are available on request.

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation. The results relate only to the items tested and where relevant sampled.

Tests marked 'Not UKAS Accredited' in this Report/Certificate are not included in the UKAS Accreditation Schedule for our laboratory.

This test report is not a statement of conformity to any specification or standard.

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Page 1 of 5

Certificate of Analysis



Report Number: **COV/2371123/2022**

Issue **1**

Laboratory Number **22017647**

Sample **1** of **2**

Sample Source: **RSK Geosciences**

Sample Point Description: **Drinking Water**

Sample Description: **SW 1**

Sample Matrix: **Drinking Water**

Sample Date: **09 September 2022** Sample Received **09 September 2022** Analysis Complete: **29 September 2022**

Test Description	Result	Units	Limit	Accreditation	Method
Benzo(b)fluoranthene	2.74	ng/l		Y Wak	WPC27
Benzo(k)fluoranthene	1.57	ng/l		Y Wak	WPC27
Benzo(ghi)perylene	2.18	ng/l		Y Wak	WPC27
Benzo(a)pyrene	4.15	ng/l	10	Y Wak	WPC27
Fluoranthene	5.33	ng/l		Y Wak	WPC27
Indeno(1,2,3cd)pyrene	3.85	ng/l		Y Wak	WPC27
Total PAHs 4 Constituents	0.01	ug/l	0.1	Y Wak	WPC27
PAH Total ug/l	0.02	ug/l		N Wak	WPC27
VPH >C5 - C10	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C5 - C6	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C6 - C8	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C5 - C10	<10	ug/l		N Cov	GEO45
Aromatic VPH >C5 - C7	<10	ug/l		N Cov	GEO45
Aromatic VPH >C7 - C8	<10	ug/l		N Cov	GEO45
Aromatic VPH >C8 - C10	<10	ug/l		N Cov	GEO45
Aromatic VPH >C5 - C10	<10	ug/l		N Cov	GEO45
Aliphatic VPH >C8 - 10	<10	ug/l		N Cov	GEO45

Analyst Comments for 22017647:

No Analyst Comment

This issue replaces all previous issues

Accreditation Codes: Y = UKAS / ISO17025 Accredited, N = Not UKAS / ISO17025 Accredited, M = MCERTS.

Analysed at: CHE = Chester(CH5 3US), COV = Coventry(CV4 9GU), OTT = Otterbourne(SO21 2RU), S = Subcontracted, TRB = Subcontracted to Trowbridge(BA14 0XD), WAK = Wakefield(WF5 9TG).

F = Data supplied by customer.

For Microbiological determinands 0 or ND=Not Detected, For Legionella ND=Not Detected in volume of sample filtered.

I/S=Insufficient sample For soil/sludge samples: AR=As received, DW=Dry weight.

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Page 2 of 5

Certificate of Analysis



Report Number: COV/2371123/2022 Issue 1
Laboratory Number 22017648 Sample 2 of 2
Sample Source: RSK Geosciences
Sample Point Description: Drinking Water
Sample Description: SW 2
Sample Matrix: Drinking Water
Sample Date: 09 September 2022 Sample Received 09 September 2022 Analysis Complete: 29 September 2022

Test Description	Result	Units	Limit	Accreditation		Method
Benzo(b)fluoranthene	<0.39	ng/l	10	Y	Wak	WPC27
Benzo(k)fluoranthene	1.08	ng/l		Y	Wak	WPC27
Benzo(ghi)perylene	<0.43	ng/l		Y	Wak	WPC27
Benzo(a)pyrene	<0.57	ng/l		Y	Wak	WPC27
Fluoranthene	<0.88	ng/l		Y	Wak	WPC27
Indeno(1,2,3cd)pyrene	<0.79	ng/l	0.1	Y	Wak	WPC27
Total PAHs 4 Constituents	0.001	ug/l		Y	Wak	WPC27
PAH Total ug/l	0.001	ug/l		N	Wak	WPC27

Analyst Comments for 22017648: No Analyst Comment

This issue replaces all previous issues
Accreditation Codes: Y = UKAS / ISO17025 Accredited, N = Not UKAS / ISO17025 Accredited, M = MCERTS.
Analysed at: CHE = Chester(CH5 3US), COV = Coventry(CV4 9GU), OTT = Otterbourne(SO21 2RU), S = Subcontracted, TRB = Subcontracted to Trowbridge(BA14 0XD), WAK = Wakefield(WF5 9TG),
F = Data supplied by customer.
For Microbiological determinands 0 or ND=Not Detected, For Legionella ND=Not Detected in volume of sample filtered.
I/S=Insufficient sample For soil/sludge samples: AR=As received, DW=Dry weight.

Signed:

Name: P. Patel Date: 11 October 2022

Title: Inorganics Chemistry Manager

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APPENDIX C

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Generic Assessment Criteria for Controlled Waters



GENERIC ASSESSMENT CRITERIA FOR CONTROLLED WATERS

Protection of the water environment

The water environment in the United Kingdom is protected under a number of regulatory regimes. The relevant environmental regulator is consulted where there may be a risk that pollution of ‘controlled waters’ may occur or may have occurred in the past.

The term ‘controlled waters’ refers to coastal waters, inland freshwaters and groundwater. The EU Water Framework Directive (WFD) (2000/60/EC) is implemented via domestic regulations and guidance, covering aspects of groundwater and surface water protection as well as drinking water supply policy. Domestic legislation and guidance will vary across the United Kingdom. Therefore, the relevant legislation for England, Wales, Northern Ireland and Scotland should be reviewed, alongside guidance provided by the Environment Agency (EA), Natural Resource Wales (NRW), the Scottish Environmental Protection Agency (SEPA) or the Northern Ireland Environment Agency (NIEA), as appropriate.

The main objectives of the protection and remediation of groundwater under threat from land contamination are set out within “The Environment Agency’s approach to groundwater protection”, version 1.0 (March 2017)⁽¹⁾ and the associated guidance “Land contamination groundwater compliance points: quantitative risk assessments (March 2017)^(1a) that have replaced the previous guidance document “Groundwater Principles and Practice (GP3)”. When assessing risks to groundwater, the following need to be considered:

- Where pollutants have not yet entered groundwater, all necessary and reasonable measures must be taken to:
 - **prevent the input of hazardous substances into groundwater** (see description of hazardous substances below)
 - **limit the entry of other (non-hazardous) pollutants into groundwater to avoid pollution, deterioration in the status of groundwater bodies and to prevent sustained, upward trends in pollutant concentrations in groundwater.**
- Where pollutants have already entered groundwater, the priority is to take all necessary and reasonable measures to:
 - **minimise further entry of “contaminants” where there is a defined source**
 - **limit the pollution of groundwater or any effect on the status of the groundwater body from the future expansion of the ‘plume’, if necessary, by actively reducing its extent.**

Within the context of groundwater risk assessments on sites affected by land contamination, “reasonable” means feasible without involving disproportionate costs. What costs are “disproportionate” depends on site-specific circumstances, which may include:

- Considerations of technical feasibility such as identified by the remedial options appraisal, this may be due to the distribution or nature of the contamination and the available remedial methods to treat the identified contamination;
- Sustainability considerations.



DEFINITIONS AND SUBSTANCE CLASSIFICATIONS

Risks to surface waters:

When assessing risks to surface waters, the following list of definitions should be understood:

Priority substances (PS) are harmful substances originally identified under the Water Framework Directive (WFD) 2000/60/EC as substances ‘presenting a significant risk to or via the aquatic environment’ at a European level. Member States are required to incorporate the identified **PS** into their country-wide monitoring programmes. There are currently 33 **PS** defined within the Priority Substances Directive (2013/39/EU; Annex 1), with a further 12 additional substances due to come into force from 22 December 2018. Directive 2013/39/EU has been transposed into domestic legislation for England and Wales by The Water Framework Directive (Standards and Classification) Directions (England and Wales) 2015.

Under the umbrella of **PS**, there is a sub-set of substances identified as being “hazardous”, and these are referred to as **Priority hazardous substances (PHS)**. The list of **PHS** is defined at EU level within the Priority Substances Directive (2013/39/EU). The WFD defines hazardous substances as ‘substances (or groups of substances) that are toxic, persistent and liable to bio-accumulate, and other substances or groups of substances that give rise to an equivalent level of concern.’ There are currently **21 PHS** (previously 15 PHS, with a further 6 substances added in 22 December 2018).

There is also another group of substances defined at EU level and which are referred to as **other pollutants (OP)** in Directive 2013/39/EU. These are additional substances which although not **priority substances**, have EQS which are identical to those laid down in the legislation which applied prior to 13 January 2009 (Directive 2008/105/EU). The **OP** are listed along with the **priority substance (PS)** within the Priority Substances Directive (2013/39/EU), and their associated EQS are also listed therein. There are 6 **OP** defined within the Priority Substances Directive (2013/39/EU).

In addition to the EU level substances, there are also a group of pollutants defined at a Member State level, referred to as **Specific pollutants (SP)**. These substances are pollutants which are released in significant quantities into water bodies in each of the individual European Member States. Under the WFD, Member States are required to set their own EQS for these substances. An indicative list of **SP** is given in Annex VIII of the WFD. Many of the substances categorised as **SP** in the UK were formerly List 2 substances under the old Groundwater Directive (80/68/EEC). The **SP** are defined within Part 2 (Table 1) of The Water Framework Directive (Standards and Classification) Directions (England and Wales) 2015.

Risks to groundwater:

When assessing risks to groundwater, the following definitions should be understood:

Under the requirements of the Groundwater Daughter Directive (2006/118/EU), the UK has published a list of substances it considers to be **hazardous substances** with respect to groundwater. In their advisory capacity to the government, this list has been derived by the UK Joint Agencies Groundwater Directive Advisory Group (JAGDAG), of which the Environment Agency is a member. The latest JAGDAG list of **hazardous substances** was published in January 2019 and the Environment Agency will use the updated list of hazardous substances from this date for all new activities that may lead to the discharge of hazardous substances to groundwater. The list is extensive and can be found in full at:

<https://www.wfduk.org/stakeholders/jagdag>



Selecting the appropriate assessment criteria

When assessing the risks to controlled waters, various assessment criteria apply, depending on the nature of the assessment and the conceptual site model.

Where a surface water body is involved, then Environmental Quality Standards (EQS) are the relevant assessment criteria as they are designed to be protective of surface water ecology.

Where a public water supply or a Principal aquifer is involved, then the standards defined in The Water Supply (Water Quality) Regulations⁽²⁾ are the primary source of assessment criteria. The Private Water Supplies Regulations⁽³⁾ may also be applicable in some cases. For instances where there are no UK assessment criteria, then the World Health Organisation (WHO) drinking water guidelines⁽⁴⁾ may be used.

This appendix presents the generic assessment criteria (GAC) that RSK considers suitable for assessing risks to controlled waters for our most commonly encountered determinants. A full list of EQS for England and Wales are included in The Water Framework Directive (Standards and Classification) Directions (England and Wales) 2015.

The RSK GAC for controlled waters are presented in **Table 1** and **Table 2**. In line with the Environment Agency's Remedial Targets Methodology, the GAC for controlled waters are termed 'target concentrations'.

The appropriate target concentrations should be selected with consideration to:

- the site conceptual model (i.e. the receptor at potential risk);
- whether the substance is already present in groundwater at the site;
- whether or not the substance is classified as a priority hazardous substance under the Priority Substances Directive (2013/39/EC) (see above), or as a hazardous substance according to the current list of JAGDAG determinations⁽⁵⁾; and
- background concentrations in the aquifer (if applicable).

It is important to remember that the WFD and Environment Agency guidance^(1 & 1a) support a sustainable, risk-based approach be applied to groundwater contamination. Exceedance of any target concentration does not necessarily imply that an unacceptable risk exists or that remediation is inevitably required.



Target concentrations shaded in green are <u>statutory values</u>	Target concentrations shaded in orange are <u>non-statutory values</u>
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Note: Units µg/l throughout (unless otherwise stated)

Table 1: Target concentrations for controlled waters (excluding TPH CWG fractions)

Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Metals & other inorganics						
Hazardous substance	Specific pollutant	Arsenic	-	10 ⁽²⁾	50 ^(6a)	25 ^(6a)
Non-hazardous pollutant	Priority substance	Cadmium	0.1 ⁽⁷⁾	5 ⁽²⁾	≤0.08, 0.08, 0.09, 0.15, 0.25 ^(6b)	0.2 ^(6a)
(Not determined)	-	Chromium (total)	-	50 ⁽²⁾	8.1 Sum values for chromium III and VI	-
(Not determined)	Specific pollutant	Chromium (III)	-	Use value for total chromium	4.7 ^(6a)	-
Hazardous substance	Specific pollutant	Chromium (VI)			3.4 ^(6a)	0.6 ^(6a)



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
(Not determined)	Specific pollutant	Copper	-	2,000 ⁽²⁾	1 bioavailable ^(6a)	3.76 dissolved, where DOC ≤1mg/l ^(6a)
						3.76µg/l + (2.677µg/l x ((DOC/2) – 0.5µg/l)) dissolved, where DOC >1mg/l ^(6a)
Hazardous substance	Priority substance	Lead	-	10 ⁽²⁾	1.2 bioavailable ^(6a)	1.3 ^(6a)
Hazardous substance	Priority hazardous substance	Mercury	0.01 ⁽⁷⁾	1 ⁽²⁾	0.07 ^(6c)	0.07 ^(6c)
Non-hazardous pollutant	Priority substance	Nickel	-	20 ⁽²⁾	4.0 bioavailable ^(6a)	8.6 ^(6a)
Non-hazardous pollutant	-	Selenium	-	10 ⁽²⁾	-	-
Non-hazardous pollutant	Specific pollutant	Zinc	-	3,000 ⁽⁸⁾	10.9 bioavailable ^(6a)	6.8 dissolved ^(6a)
(Not determined)	Specific pollutant	Iron	-	200 ⁽²⁾	1000 ^{(6a)*1}	1000 ^(6a)) *1
(Not determined)	Specific pollutant	Manganese	-	50 ⁽²⁾ (0.05mg/l)	123 bioavailable ^(6a) (0.123mg/l)	-
(Not determined)	-	Aluminium	-	200 ⁽²⁾	-	-

Controlledwaters_GAC_Rev15 E&W



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Hazardous substance	Priority hazardous substance	Tributyltin compounds (Tributyltin-cation)	0.001 ⁽⁷⁾	-	0.0002 ^(6a)	0.0002 ^(6a)
(Not determined)	-	Sodium	-	200,000 ⁽²⁾ (200 mg/l)	-	-
Non-hazardous pollutant	Specific pollutant	Cyanide (Hydrogen cyanide)	-	50 ⁽²⁾ (0.05 mg/l)	1 ^(6a) (0.001 mg/l)	1 ^(6a) (0.001 mg/l)
Non-hazardous pollutant	-	Total ammoniacal nitrogen ^{\$}	-	500 ⁽²⁾ (0.5 mg/l) as NH ₄ (472 expressed as NH ₃ ; 389 expressed as N)	300 ^(6f) (0.3 mg/l) as N (364 expressed as NH ₃ ; 386 expressed as NH ₄)	-
Non-hazardous pollutant	Specific pollutant	Ammonia un-ionised (equilibrium ratio calculated) (NH ₃)	-	-	-	21 ^(6a) (0.021 mg/l)
Non-hazardous pollutant	Specific pollutant	Chlorine	-	-	2 ^(6a) (0.002 mg/l)	10 ^(6d) (0.01 mg/l)
(Not determined)	-	Chloride	-	250,000 ⁽²⁾ (250 mg/l)	-	-
(Not determined)	-	Sulphate	-	250,000 ⁽²⁾ (250 mg/l)	-	-
(Not determined)	-	Nitrate (as NO ₃)	-	50,000 ⁽²⁾ (50 mg/l)	-	-

Controlledwaters_GAC_Rev15 E&W



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
(Not determined)	-	Nitrite (as NO ₂)	-	500 ⁽²⁾ (0.5 mg/l)	10 ⁽⁹⁾ (0.01 mg/l)	-
Volatile organic compounds (VOC)						
Non-hazardous pollutant	Other pollutant	Tetrachloroethene (tetrachloroethylene; PCE)	0.1 ⁽⁷⁾	10 ⁽²⁾ sum of TCE and PCE	10 ^(6a)	10 ^(6a)
Hazardous substance	Other pollutant	Trichloroethene (trichloroethylene; TCE)	0.1 ⁽⁷⁾		10 ^(6a)	10 ^(6a)
Hazardous substance (1,1,2,2-tetra - chloroethane)	Specific pollutant	Tetrachloroethane	-	-	140 ^(6a)	-
Hazardous substance	Other pollutant	Carbon tetrachloride (tetrachloromethane)	0.1 ⁽⁷⁾	3.0 ⁽²⁾	12 ^(6a)	12 ^(6a)
Non-hazardous pollutant	Priority substance	1,2-Dichloroethane	1.0 ⁽⁷⁾	3.0 ⁽²⁾	10 ^(6a)	10 ^(6a)
Non-hazardous pollutant	-	1.2-Dichloroethene (DCE) sum of cis and trans	-	50.0 ⁽⁴⁾	-	-
Hazardous substance	-	Vinyl chloride (chloroethene, chloroethylene)	-	0.5 ⁽²⁾	-	-
Non-hazardous pollutant	Priority substance	Dichloromethane	-	20 ⁽⁴⁾	20 ^(6a)	20 ^(6a)

Controlledwaters_GAC_Rev15 E&W



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Non-hazardous pollutant	Priority substance	Trichlorobenzenes	0.01 ⁽⁷⁾	-	0.4 ^(6a)	0.4 ^(6a)
Hazardous substance	Priority substance	Trichloromethane (Chloroform)	0.1 ⁽⁷⁾	100 ^(2a) (sum of trihalomethanes – chloroform, bromform, dibromochloromethane, bromodichloromethane)	2.5 ^(6a)	2.5 ^(6a)
(Not determined)	-	Bromoform	-		-	-
(Not determined)	-	Dibromochloromethane	-		-	-
(Not determined)	-	Bromodichloromethane	-		-	-
Non-hazardous pollutant	Priority hazardous substance	Di(2-ethylhexyl) phthalate (bis(2-ethylhexyl) phthalate, DEHP)	-	8 ⁽⁴⁾	1.3 ^(6a)	1.3 ^(6a)
(Not determined)	Specific pollutant	Benzyl butyl phthalate	-	-	7.5 ^(6a)	0.75 ^(6e)
Hazardous substance	Priority hazardous substance	Hexachlorobutadiene (as a pesticide, but reported in a VOC suite)	0.005 ⁽⁷⁾	0.1 ⁽²⁾	0.6 ^(6c)	0.6 ^(6c)
Semi-volatile organic compounds (SVOC)						
(Not determined) Not to be confused with acenaphthene, which is a hazardous substance	-	Acenaphthylene (Aro EC12-EC16)	-	-	5.8 ⁽¹⁰⁾	
Hazardous substance	Priority hazardous substance	Anthracene (Aro EC16-EC21)	-	-	0.1 ^(6a)	0.1 ^(6a)

Controlledwaters_GAC_Rev15 E&W



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Non-hazardous pollutant	Priority substance	Naphthalene (Aro EC10-EC12)	-	-	2 ^(6a)	2 ^(6a)
Hazardous substance	Priority substance	Fluoranthene (Aro EC21-EC35) not used as an indicator for this EC band	-	-	0.0063 ^(6a)	0.0063 ^(6a)
Hazardous substance(s)	Priority hazardous substance(s)	Benzo(a)pyrene (Aro EC21-EC35)	-	0.01 ⁽²⁾	0.00017 ^(6a)	0.00017 ^(6a)
		Benzo(b)fluoranthene (Aro EC21-EC35)	-	0.1 ⁽²⁾ sum of the concentration of the four specified compounds	No EQS for these substances. B(a)P should be used as the indicator compound instead.	
		Benzo(k)fluoranthene (Aro EC21-EC35)	-			
		Benzo(g,h,i)perylene (Aro EC21-EC35)	-			
		Indeno(1,2,3-cd) pyrene (Aro EC21-EC35)	-			
Non-hazardous pollutant	Specific pollutant	Phenol	-	-	7.7 ^(6a)	7.7 ^(6a)
Hazardous substance	Specific pollutant	2,4-Dichlorophenol	0.1 ⁽⁷⁾	-	4.2 ^(6a)	0.42 ^(6a)



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Hazardous substance	Priority substance	Pentachloro-phenol (PCP) (as a pesticide, but reported in an SVOC suite)	0.1 ⁽⁷⁾	0.1 ⁽²⁾	0.4 ^(6a)	0.4 ^(6a)
Petroleum hydrocarbons						
Hazardous substance	-	Total petroleum hydrocarbons	-	See Table 2 for individual (non-statutory) TPH CWG fractions with respect to drinking water receptors	See individual risk driving compounds (i.e. BTEX and PAH) for specific EQS	
Hazardous substance	Priority substance	Benzene (Aro EC5-EC7)	1 ⁽⁷⁾	1 ⁽²⁾	10 ^(6a)	8 ^(6a)
Hazardous substance	Specific pollutant	Toluene (Aro EC7-EC8)	4 ⁽⁷⁾	700 ⁽⁴⁾	74 ^(6a)	74 ^(6a)
Hazardous substance	-	Ethylbenzene (Aro EC8-EC10)	-	300 ⁽⁴⁾	300 ⁽¹¹⁾	-
Hazardous substance	-	Xylenes (Aro EC8-EC10)	3 ⁽⁷⁾	500 ⁽⁴⁾	30 ⁽¹¹⁾	-
Non-hazardous pollutant	-	Methyl tertiary butyl ether (MTBE)	-	15 ⁽¹²⁾	-	

Controlledwaters_GAC_Rev15 E&W



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Pesticides, fungicides, insecticides and herbicides						
(Not determined) – assume to be Hazardous Substance as per below	-	Total pesticides	-	0.5 ⁽²⁾	-	-
(Not determined) - assume to be Hazardous Substance as per below	-	Other individual pesticides (unless otherwise detailed below)	-	0.1 ⁽²⁾	-	-
Hazardous substance(s)	Other pollutant (Cyclodiene pesticides)	Aldrin	0.003 ⁽⁷⁾	0.03 ⁽²⁾	0.01 ^(6a) (sum of all four)	0.005 ^(6a) (sum of all four)
		Dieldrin	0.003 ⁽⁷⁾	0.03 ⁽²⁾		
		Endrin	0.003 ⁽⁷⁾	0.1 ^(2b) ('other individual pesticide')		
		Isodrin*2	0.003 ⁽⁷⁾	0.1 ^(2b) ('other individual pesticide')		
Hazardous substance	Other pollutant	DDT (total)	0.002 ⁽⁷⁾	0.1 ⁽²⁾ ('other individual pesticide')	0.025 ^(6a)	0.025 ^(6a)
Hazardous substance	Specific pollutant	Carbendazim	-	0.1 ⁽²⁾ ('other individual pesticide')	0.15 ^(6a)	-



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Hazardous substance	Specific pollutant	Chlorothalonil	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.035 ^(6a)	-
Hazardous substance	Priority substance	Cypermethrin	-	0.1 ⁽²⁾ (‘other individual pesticide’)	8.0E ^{-5(6a)}	8.0E ^{-6(6a)}
Hazardous substance	Specific pollutant	Dimethoate	0.01 ⁽⁷⁾	0.1 ⁽²⁾ (‘other individual pesticide’)	0.48 ^(6a)	0.48 ^(6a)
(Not determined)	Specific pollutant	Glyphosate	-	0.1 ⁽²⁾ (‘other individual pesticide’)	196 ^(6a)	196 ^(6a)
Hazardous substance	Specific pollutant	Linuron	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.5 ^(6a)	0.5 ^(6a)
Non-hazardous pollutant	Specific pollutant	Mecoprop	0.04 ⁽⁷⁾	0.1 ⁽²⁾ (‘other individual pesticide’)	18 ^(6a)	18 ^(6a)
Non-hazardous pollutant	Specific pollutant	Methiocarb	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.01 ^(6a)	-
Non-hazardous pollutant	Specific pollutant	Pendimethalin	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.3 ^(6a)	-



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Hazardous substance	Specific pollutant	Permethrin	0.001 ⁽⁷⁾	0.1 ⁽²⁾ (‘other individual pesticide’)	0.001 ^(6a)	0.0002 ^(6a)
Hazardous substance	Priority substance	Alachlor	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.3 ^(6a)	0.3 ^(6a)
Hazardous substance	Priority substance	Atrazine	0.03 ⁽⁷⁾	100 ⁽⁴⁾ (‘other individual pesticide’)	0.6 ^(6a)	0.6 ^(6a)
Hazardous substance	Priority substance	Diuron	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.2 ^(6a)	0.2 ^(6a)
Hazardous substance	Priority hazardous substance	Endosulphan	0.005 ⁽⁷⁾	0.1 ⁽²⁾ (‘other individual pesticide’)	0.005 ^(6a)	0.0005 ^(6a)
Non-hazardous pollutant	Priority substance	Isoproturon	-	0.1 ⁽²⁾ (‘other individual pesticide’)	0.3 ^(6a)	0.3 ^(6a)
Hazardous substance	Priority substance	Simazine	0.03 ⁽⁷⁾	0.1 ⁽²⁾ (‘other individual pesticide’)	1 ^(6a)	1 ^(6a)
Hazardous substance	Priority hazardous substance	Trifluralin	0.01 ⁽⁷⁾	0.1 ⁽²⁾ (‘other individual pesticide’)	0.03 ^(6a)	0.03 ^(6a)



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Hazardous substance	Priority substance	Dichlorvos	-	0.1 ⁽²⁾ (‘other individual pesticide’)	6.0E ^{-4(6a)}	6.0E ^{-5(6a)}
Hazardous substance	Priority substance	Heptachlor and heptachlor epoxide	-	0.03 ⁽²⁾	2.0E ^{-7(6a)}	1.0E ^{-08(6a)}
Miscellaneous						
(Not determined)	Specific pollutant	Triclosan (antibacterial agent)	-	-	0.1 ^(6a)	0.1 ^(6a)
Hazardous substance	Priority hazardous substance	Perfluoro-octane sulfonic acid (and its derivatives) (PFOS)	-	-	6.5E ^{-4(6a)}	1.3E ^{-4(6a)}
Hazardous substance	Priority hazardous substance	Hexabromo cyclododecane (HBCDD)	-	-	0.0016 ^(6a)	0.0008 ^(6a)



Substance classification		Determinant	Target concentrations (µg/l)			
Groundwater receptors ⁽⁵⁾	Surface water receptors ⁽⁶⁾		Minimum reporting value	UK drinking water standard (or best equivalent)	EQS or best equivalent	
					Freshwater	Transitional (estuaries) and coastal waters
Notes: ‘-’ A target concentration is not available. ^{\$}Please note that total ammonia (NH₄⁺ and NH₃) is equivalent to ammoniacal nitrogen in laboratory reports ^{*1} Please note that although iron is listed in the 2015 Direction as 1.000 µg/l, the EQS remains at 1mg/l in Scotland and it is assumed this is an error and should read either 1,000 or 1000µg/l. ^{*2} Please note that although Isodrin is not listed in name within the group of “Cyclodiene pesticides” in Table 1 of Schedule 3 Part 3 of the 2015 Direction⁽⁶⁾, the CAS number for Isodrin (465-73-6) <u>is</u> listed and therefore it is assumed that it has been missed off the named list of substances. ^{*3} Total petroleum hydrocarbons is used for consistency, but is an analytical method-defined measurement for a mixture of hydrocarbons subject to environmental analysis¹¹. “Bioavailable” in relation to copper, zinc, nickel and manganese (but not lead) is the generic EQSbioavailable^(6a) derived from the Metal Bioavailability Assessment Tool (M-BAT) developed by the Water Framework Directive UK Technical Advisory Group (WFDTAG). Exceedance of this value should prompt a site-specific assessment using the M-BAT with pH, DOC and Ca to derive a site-specific EQS termed the PNEC_{dissolved}. http://www.wfduk.org/resources/rivers-lakes-metal-bioavailability-assessment-tool-m-bat. For zinc, if there is an exceedance of the EQSbioavailable in an initial GQRA, Tier 2 required that the EQS for zinc should also have the ambient background concentration of zinc added as well						



Table 2: World Health Organization (WHO) guide values for TPH CWG fractions in drinking water⁽¹³⁾ (as referenced in CL:AIRE, 2017⁽¹¹⁾)

TPH CWG fraction	WHO guide value for drinking water ⁽¹³⁾ (µg/l)
Aliphatic fractions:	
Aliphatic EC5-EC6	15,000
Aliphatic >EC6-EC8	15,000
Aliphatic >EC8-EC10	300
Aliphatic >EC10-EC12	300
Aliphatic >EC12-EC16	300
Aliphatic >EC16-EC21	-
Aliphatic >EC21-EC35	-
Aromatic fractions:	
Aromatic EC5-EC6	10 (benzene)
Aromatic >EC6-EC8	700 (toluene)
Aromatic >EC8-EC10	300 (ethyl benzene) 500 (xylenes)
Aromatic >EC10-EC12	90
Aromatic >EC12-EC16	90
Aromatic >EC16-EC21	90
Aromatic >EC21-EC35	90
Reference: World Health Organisation (WHO), 2008. Petroleum products in drinking-water. Background document for development of WHO guidelines for drinking water quality. WHO/SDE/WSH/05.08/123. World Health Organisation, Geneva ⁽¹³⁾ .	



References

1. Environment Agency (2018), ‘The Environment Agency’s approach to groundwater protection’, version 1.2, February 2018 (formerly contained within GP3).
<https://www.gov.uk/government/collections/groundwater-protection>

1a. Environment Agency (2017), ‘Land contamination groundwater compliance points: quantitative risk assessments’, March 2017 (formerly contained within GP3) [accessed 29 March 2017].
<https://www.gov.uk/government/collections/groundwater-protection>

2. The Water Supply (Water Quality) Regulations 2016 (SI 2016/619)
2a. Sum of chloroform, bromoform, dibromochloromethane and bromodichloromethane
2b. Standard applies to individual pesticides except aldrin, dieldrin, heptachlor and heptachlor epoxide, for which a separate standard is defined.

3. The Private Water Supplies (England) Regulations 2016. SI 2016 / 618

4. WHO (2011), *Guidelines for drinking-water quality*, 4th edn

5. JAGDAG hazard substance determinations (January 2019): This list contains substances that are determined to be hazardous substances or non-hazardous pollutants for the purposes of the groundwater directive 2006/118/EC. <https://www.wfduk.org/stakeholders/jagdag> [accessed 1 February 2021]
The absence of an assessment or substance from the list means an assessment has not been completed and is therefore presented as ‘Not determined’. For further details on how substances are assessed, see the Joint Agencies Groundwater Directive Advisory Group (JAGDAG) ‘Methodology for the determination of hazardous substances in groundwater for the purposes of the groundwater directive 2006/118/EC’ which is available from the JAGDAG website. The methodology is a UK –wide framework that sets criteria for how to assess whether a substance is a hazardous substances in groundwater.

6. The Water Framework Directive (Standards and Classification) Directions (England and Wales) 2015.
6a. The EQS for these substances are based on a “long term mean” or an “annual average (AA)” EQS.
6b. For cadmium and its compounds the EQS values vary depending on the hardness of the water as specified in five class categories (Class 1: < 40 mg CaCO3/l, Class 2: 40 to < 50 mg CaCO3/l, Class 3: 50 to < 100 mg CaCO3/l, Class 4: 100 to < 200 mg CaCO3/l and Class 5: ≥ 200 mg CaCO3/l).
6c. The EQS for Mercury and hexachlorobutadiene are based on a “maximum acceptable concentration (MAC)” EQS in absence of an “annual average (AA)” EQS.
6d. The EQS for chlorine in saltwater is based on the 95th percentile concentration of total residual oxidant, which refers to the sum of all oxidising agents existing in water, expressed as available chlorine.
6e. The recommended saltwater standard is derived using a safety factor of 100. Where the standard is failed, it is recommended that supporting evidence of ecological damage should be obtained before committing to expensive action.
6f. EQS for total ammonia is as per Schedule 3, Part 1, Table 7 of of the above directions. EQS applies to river types 1, 2 and 4 and 6 (namely upland and low alkalinity). The EQS for a lowland and high alkalinity rivers (types 3, 5 and 7) is 600µg/l (0.6mg/l).

Additional information on the Metal Bioavailability Assessment Tool (M-BAT) is available at <http://www.wfduk.org/resources/rivers-lakes-metal-bioavailability-assessment-tool-m-bat>

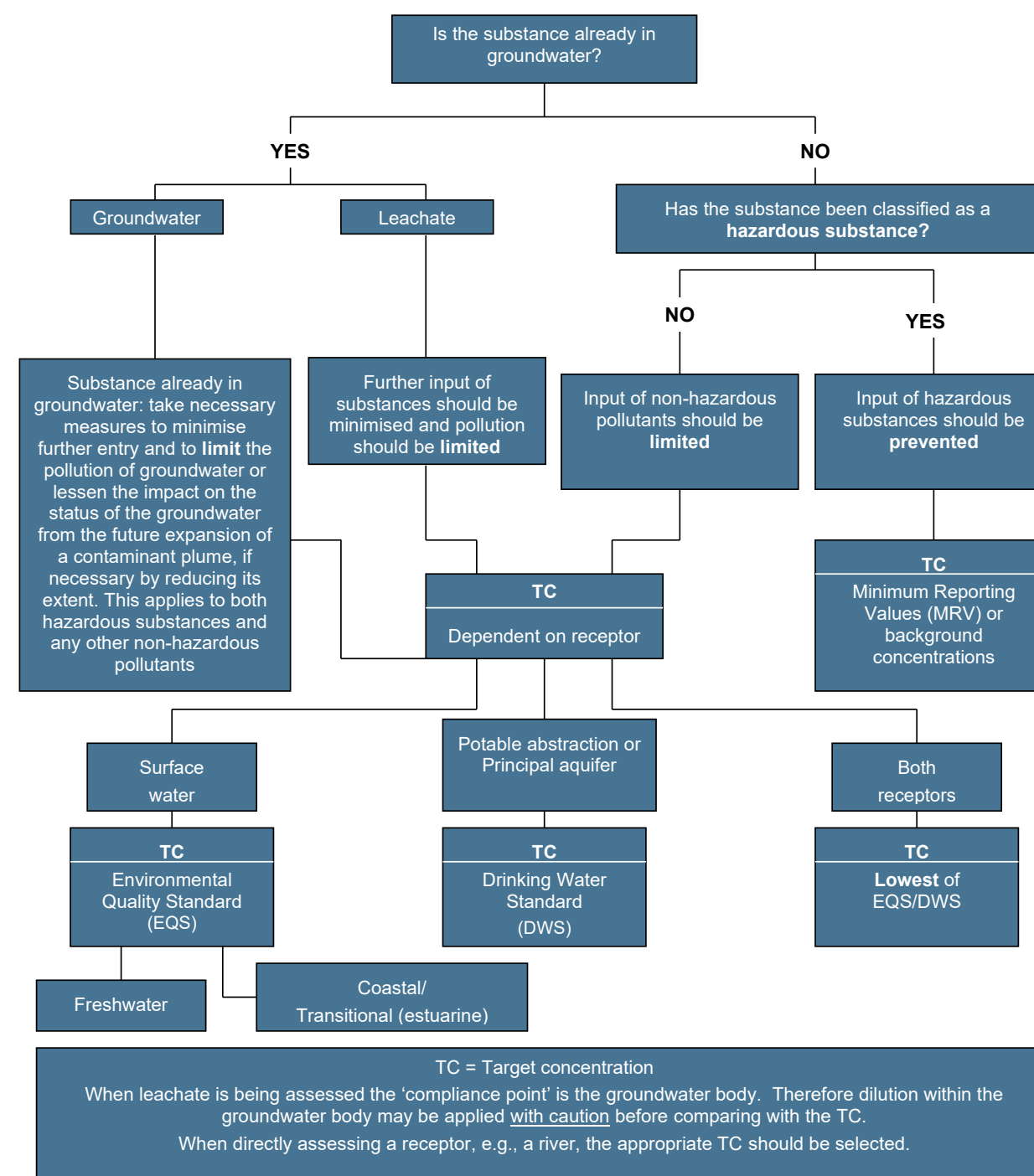


7. Minimum reporting values listed at <https://www.gov.uk/government/publications/values-for-groundwater-risk-assessments/hazardous-substances-to-groundwater-minimum-reporting-values> (updated 13 January 2017; accessed 29 March 2017). Note target concentration for xylenes is 3 µg/l each for o-xylene and m/p xylene as it may not be possible to separate m- and p-xylene; 135 tcb, 124 tcb, 123 tcb each to 0.01 µg/l)
8. The Surface Waters (Abstraction for Drinking Water) (Classification) Regulations 1996 (as amended). SI 1996 / 3001
9. Council Directive on the Quality of Fresh Waters Needing Protection or Improvement in Order to Support Fish Life (Freshwater Fish Directive) (78/659/EEC)
10. WRc plc (2002), R&D Technical Report P45.
11. CL:AIRE, 2017. Petroleum Hydrocarbons in Groundwater: Guidance on assessing petroleum hydrocarbons using existing hydrogeological risk assessment methodologies. V1.1.
12. Drinking Water Inspectorate (London, UK). Environmental Information Request on MTBE in drinking water. Ref. DWI 1/10/18; dated 28 November 2006. Value is based on the odour threshold for MTBE, which is lower than a health-based guideline value
13. World Health Organisation (WHO), 2008. Petroleum products in drinking-water. Background document for development of WHO guidelines for drinking water quality. WHO/SDE/WSH/05.08/123. World Health Organisation, Geneva. [accessed 29 March 2017] http://www.who.int/water_sanitation_health/dwg/chemicals/petroleumproducts_2add_june2008.pdf

Controlledwaters_GAC_Rev15 E&W



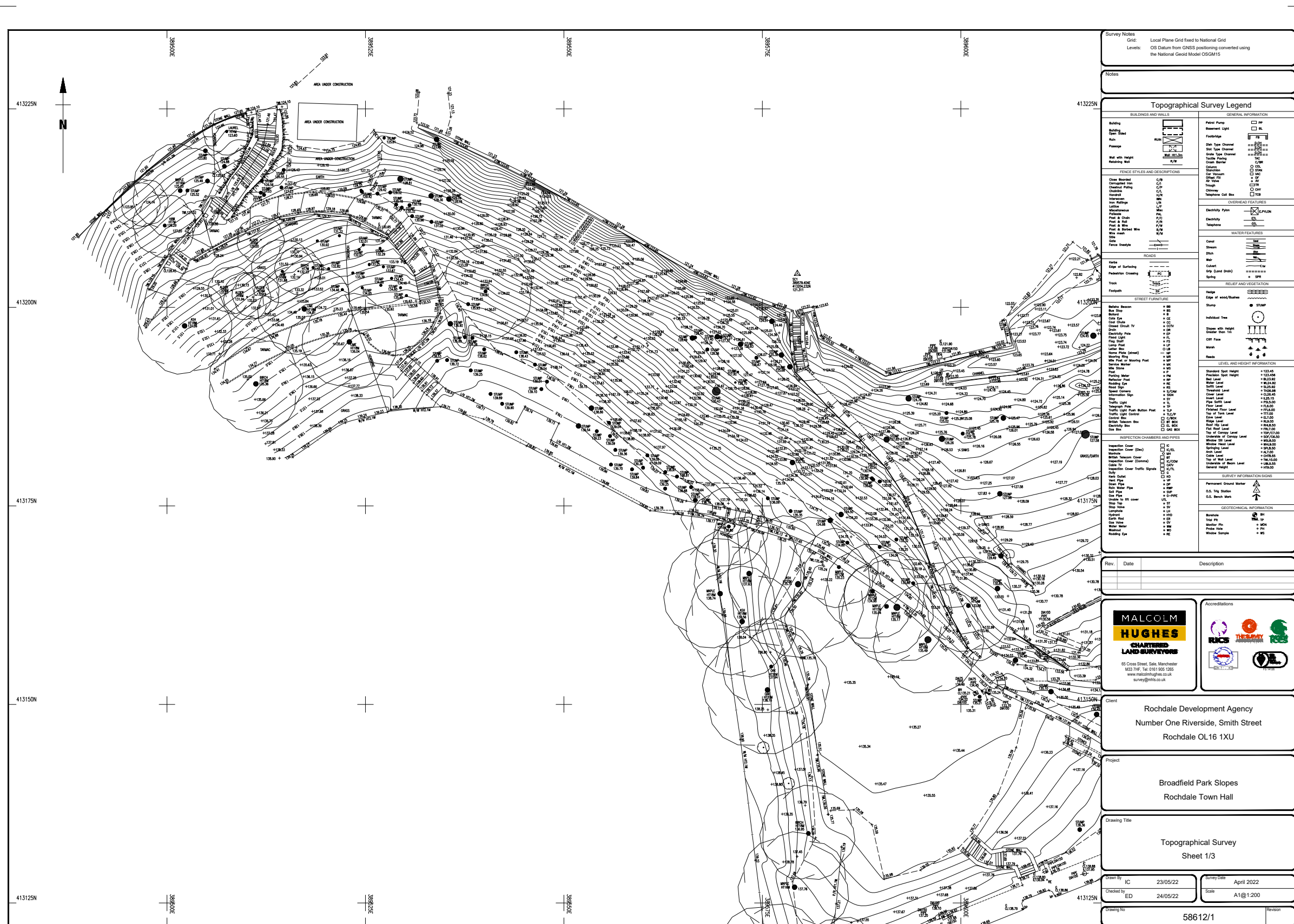
FLOW CHART TO ASSIST WITH SELECTION OF TARGET CONCENTRATIONS

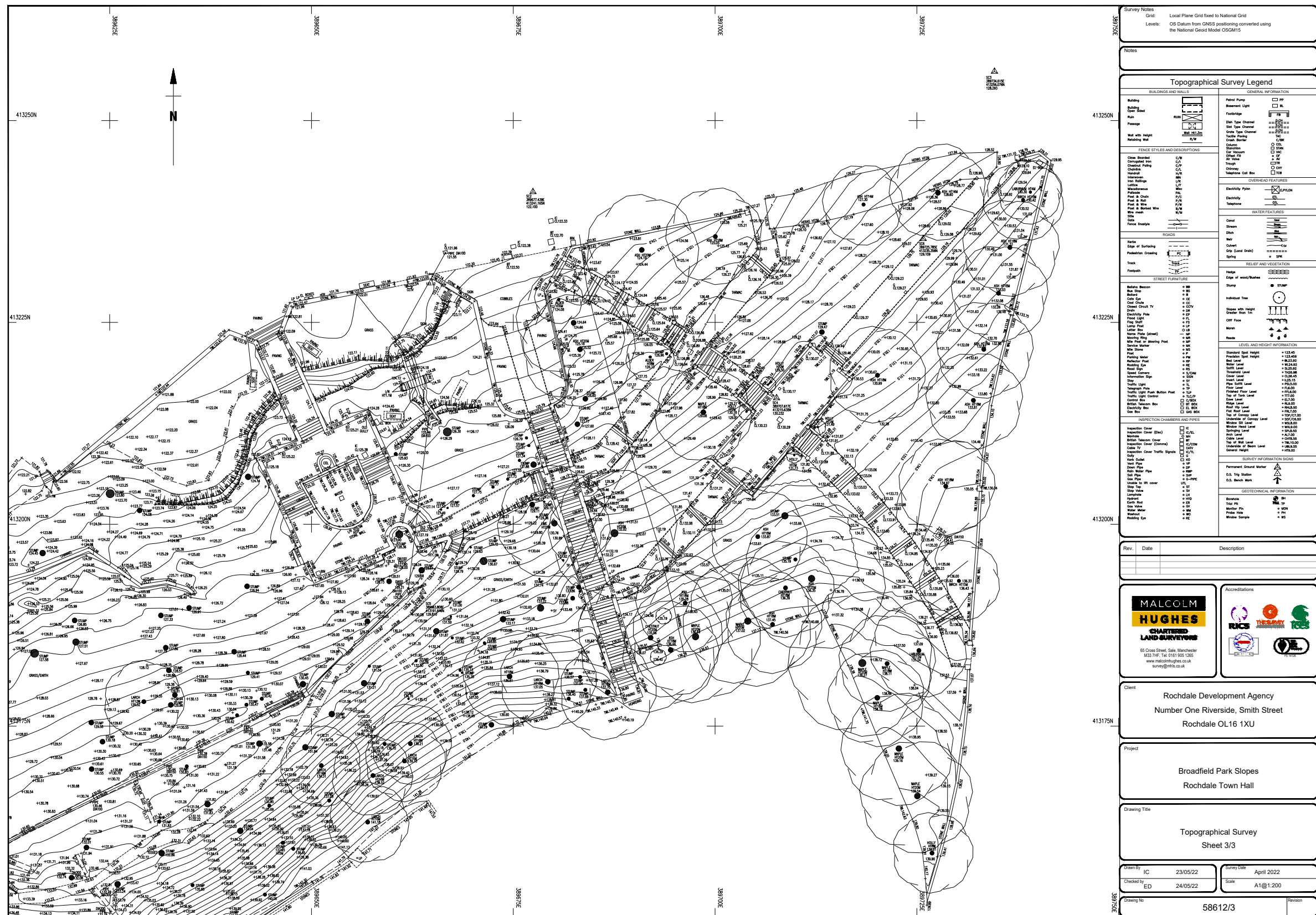


Controlledwaters_GAC_Rev15 E&W



Appendix B Topo Survey







Appendix C

CCTV Drainage Survey

LAND SURVEYING
BUILDING SURVEYING
UNDERGROUND SURVEYING
SITE ENGINEERING
MONITORING



IPSWICH BRENTWOOD COVENTRY GLASGOW NORWICH NOTTINGHAM YEOVIL

CCTV Report



Project

Broadfield Slopes, The Esplanade,
Rochdale,

Survey Date

03/07/2023

Client

Price & Myers

Surveyor

OTM

Job Number

49962CCTV-02

Issue Date

10/07/2023

Checked by

JAD

Revision - Issue Date



Price & Myers



Survey Solutions
Nottingham
Nottinghamshire
Tel: 01773 714733, Fax:

Project-information

Project name:
Broadfield Slopes

Contract number:
49962

Contact:
Steffan Rees

Date:
10/07/2023

Client **Price & Myers**
Contact: **Steffan Rees**
Position: **Civil Engineer**
Road **37 Alfred Place**
Town **London**
County
Telephone: **02045246770**
Fax:
Mobile:
E-Mail: **srees@pricemyers.com**

Site **Broadfield Slopes**
Contact: **Steffan Rees**
Position: **Civil Engineer**
Road **The Esplanade**
Town **Rochdale**
County **Greater Manchester**
Telephone:
Fax:
Mobile:
E-Mail:

Contractor **Survey Solutions**
Contact: **Owain McDonald**
Position: **Surveyor**
Road **Farrington Way**
Town **Nottingham**
County **Nottinghamshire**
Telephone: **01773 714733**
Fax:
Mobile:
E-Mail: **Nottingham.office@survey-solutions.co.uk**

 SURVEY SOLUTIONS		Survey Solutions Nottingham Nottinghamshire Tel: 01773 714733, Fax:	
Defect Grade Description			
Project name: Broadfield Slopes	Contract number: 49962	Contact: Steffan Rees	Date: 10/07/2023
1: Brick: No Structural Defects Pipe: No Structural Defects Acceptable Structural Condition			
2: Brick: Minor cracking, Surface mortar loss, Spalling slight, wear slight Pipe: Circumfrential crack, Moderate joint defects, Spalling slight, Wear slight Minor collapse risk in short term but potential for further deterioration			
3: Brick: Total mortarloss without other defects, single brick displaced, Deformation up to 5%, Spalling medium, Wear medium Pipe: Fractures with deformation up tp 5%, Longitudinal cracking or mulitlpe cracking, Minor loss of level, More severe joint defects, Spalling medium, Wear medium ! Collapse unlikely in near future but future deterioration likely !			
4: Brick: Total mortarloss with deformation greater than 10%, Deformation up to 10% and fractured, Displaced/hanging brickwork, Small number of missing bricks Pipe: Broken, Deformation up to 10% and broken,, Fractured with deformation 5 - 10%, Multiple fractures, Serious loss of level, spalling large, wear large !! Collapse likely in foreseeable future !!			
5: Brick: Already Collapsed, Missing invert, Deformation over 10% and fractured, Displaced/hanging brickwork and deformation over 10%, Extensive missing bricks Pipe: Already collapsed, Deformation over 10% and broken, Extensive areas of fabric missing, Fractured with deformation over 10% !!! Collapsed or collapse imminent !!!			

		Survey Solutions Nottingham Nottinghamshire Tel: 01773 714733, Fax:	
Inspection report			
Date: 10/07/2023	Job N°: 49962	Weather: Heavy rain	Operator:
Present:	Vehicle:	Camera:	Preset:
		section number: 1	PLR: MH6 A X
		Cleaned: No	Grade:

Road: The Esplanade	Division:	start MH: MH6 A
Place: Broadfield Slopes	District:	end MH:
Location:	Tape No.:	Total length: 1 m
Purpose: Asset condition	Shape/Size: Circular dia 100 mm	
Use: Surface water	Material: Vitrified clay	Pipe length:
Catchment:	Lining:	
Category:		
Comment:		
Location details:		

1:25	position	code	observation	counter	photo	grade
		MH	Manhole Remark: MH6 A	00:00:00		(Constr) 0
	0.00	ST	Start of Survey	00:00:00		(Misc) 0
	0.00	WL	Water level, 0 % height/diameter	00:00:00		(Serv) 0
	0.20	JDL	Joint displaced. Large	00:00:00	4_4a	(Struct) 2
	0.60	LL	Line of Sewer deviates left	00:00:00		(Serv) 0
	1.00	OB	Obstruction, 95 % height/diameter loss	00:00:00		(Serv) 2
	1.00	SA	Survey abandoned, Remark: Unable to survey beyond obstruction.	00:00:00	7_7a	(Misc) 0

Structural Defects	Constructional Features
Service Defects	Miscellaneous Features

Price & Myers

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Inspection photos

Place:
Broadfield Slopes

Road:
The Esplanade

Date:
10/07/2023

section number:
1

PLR:
MH6 A X

A close-up inspection photo of a pipe joint. The joint is displaced, and there is a large amount of debris and silt around it. The photo is labeled 'MH6 A' in the top left corner and '10:46:30.3-JUL-2023 0.07m' in the bottom left corner.

Photo: 4_4a, 00:00:00
0.2m, Joint displaced. Large

A close-up inspection photo of a pipe obstruction. The obstruction is a large, irregular mass of debris and silt that completely blocks the view of the pipe beyond it. The photo is labeled 'MH6 A' in the top left corner and '10:46:30.3-JUL-2023 0.99m' in the bottom left corner.

Photo: 7_7a, 00:00:00
1m, Survey abandoned, Remark: Unable to survey beyond obstruction.

49962CCTV-02.mdb // page: 4

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Inspection report

Date:
10/07/2023

Job N°:
49962

Weather:
Heavy rain

Operator:

section number:
2

PLR:
MH6 X X

Present:

Vehicle:

Camera:

Preset:

Cleaned:
No

Grade:

Road:
The Esplanade

Place:
Broadfield Slopes

Location:

Division:

District:

Tape No.:

start MH:
MH6 X

end MH:

Total length:
0.5 m

Purpose:
Asset condition

Use:
Surface water

Catchment:

Shape/Size:
Circular dia 100 mm

Material:
Vitrified clay

Lining:

Category:

Comment:

Location details:

1:25	position	code	observation	counter	photo	grade
		MH	Manhole Remark: MH6 X	00:00:00		(Constr) 0
		ST	Start of Survey	00:00:00		(Misc) 0
		WL	Water level, 0 % height/diameter	00:00:00		(Serv) 0
		DES	Debris silt, 10 % cross-sectional area loss	00:00:00	11_4a	(Serv) 1
		LR	Line of Sewer deviates right	00:00:00		(Serv) 0
		OB	Obstruction, 95 % height/diameter loss	00:00:00	13_6a	(Serv) 2
		SA	Survey abandoned, Remark: Unable to survey beyond obstruction.	00:00:00		(Misc) 0

Structural Defects	Constructional Features
Service Defects	Miscellaneous Features

49962CCTV-02.mdb // page: 5

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49

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Inspection photos

Place:
Broadfield Slopes

Road:
The Esplanade

Date:
10/07/2023

section number:
2

PLR:
MH6 X X

10:46:52 10-JUL-2023

0.06m

Photo: 11_4a, 00:00:00

0m, Debris silt, 10 % cross-sectional area loss

10:47:00 3-JUL-2023

0.46m

Photo: 13_6a, 00:00:00

0.5m, Obstruction, 95 % height/diameter loss

49962CCTV-02.mdb // page: 6

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Inspection report

Date:
10/07/2023

Job N°:
49962

Weather:
Heavy rain

Operator:

section number:
3

PLR:
MH7 A X

Present:

Vehicle:

Camera:

Preset:

Cleaned:
No

Grade:

Road:
The Esplanade

Division:

start MH:
MH7 A

Place:
Broadfield Slopes

District:

end MH:

Location:

Tape No.:

Total length:
9.1 m

Purpose:
Asset condition

Shape/Size:
Circular dia 100 mm

Use:
Surface water

Material:
Vitrified clay

Catchment:

Pipe length:

Comment:

Location details:

1:75

position

code

observation

counter

photo

grade

MH7 A

0.00

MH

Manhole Remark: MH7 A

00:00:00

(Constr) 0

0.00

ST

Start of Survey

00:00:00

(Misc) 0

0.00

WL

Water level, 30 % height/diameter

00:00:00

(Serv) 0

2.30

OJM

Open joint medium

00:00:00

3_4a

(Struct) 1

7.40

CC

Circumferential Crack, from 05 to 02 o'clock

00:00:00

3_5a

(Struct) 2

8.40

S1

RF

Fine Roots, Start

00:00:00

21_6a

(Serv) 1

8.90

F1

RF

Fine Roots, Finish

00:00:00

(Serv) 1

9.10

OB

Obstruction, 95 % height/diameter loss

00:00:00

23_8a

(Serv) 2

9.10

SA

Survey abandoned, Remark: Unable to survey beyond obstruction.

00:00:00

(Misc) 0

Structural Defects

Constructional Features

Service Defects

Miscellaneous Features

49962CCTV-02.mdb // page: 7

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Inspection photos

Place:
Broadfield Slopes

Road:
The Esplanade

Date:
10/07/2023

section number:
3

PLR:
MH7 A X



Photo: 3_4a, 00:00:00
2.3m, Open joint medium



Photo: 3_5a, 00:00:00
7.4m, Circumferential Crack, from 05 to 02 o'clock



Photo: 21_6a, 00:00:00
8.4m, Fine Roots, Start



Photo: 23_8a, 00:00:00
9.1m, Obstruction, 95 % height/diameter loss

49962CCTV-02.mdb // page: 8

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Inspection report

Date:
10/07/2023

Job N°:
49962

Weather:
Heavy rain

Operator:

section number:
4

PLR:
MH9 A X

Present:

Vehicle:

Camera:

Preset:

Cleaned:
No

Grade:

Road:
The Esplanade

Division:

start MH:
MH9 A

Place:
Broadfield Slopes

District:

end MH:

Location:

Tape No.:

Total length:
7.2 m

Purpose:
Asset condition

Shape/Size:
Circular dia 100 mm

Use:
Surface water

Material:
Vitrified clay

Catchment:

Pipe length:

Comment:

Location details:

1:75

position

code

observation

counter

photo

grade

MH9 A

0.00

MH

Manhole Remark: MH9 A

00:00:00

(Constr) 0

0.00

ST

Start of Survey

00:00:00

(Misc) 0

0.00

WL

Water level, 30 % height/diameter

00:00:00

(Serv) 0

0.20

JDL

Joint displaced. Large

00:00:00

30_4a

(Struct) 2

3.00

LR

Line of Sewer deviates right, Remark: Slight deviation.

00:00:00

(Serv) 0

7.20

B

Sewer Broken, from 09 to 03 o'clock

00:00:00

32_6a

(Struct) 4

7.20

SA

Survey abandoned, Remark: Unable to survey beyond broken sewer.

00:00:00

(Misc) 0

Structural Defects

Constructional Features

Service Defects

Miscellaneous Features

49962CCTV-02.mdb // page: 9

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51

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Inspection photos

Place: Broadfield Slopes	Road: The Esplanade	Date: 10/07/2023	section number: 4	PLR: MH9 A X
-----------------------------	------------------------	---------------------	----------------------	-----------------

Photo: 30_4a, 00:00:00
0.2m, Joint displaced. Large

Photo: 32_6a, 00:00:00
7.2m, Sewer Broken, from 09 to 03 o'clock

49962CCTV-02.mdb // page: 10

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Inspection report

Date: 10/07/2023	Job N°: 49962	Weather: Heavy rain	Operator:	section number: 5	PLR: DR1 A X
Present:	Vehicle:	Camera:	Preset:	Cleaned: No	Grade:

Road: The Esplanade	Division:	start MH: DR1 A
Place: Broadfield Slopes	District:	end MH: MH9
Location:	Tape No.:	Total length: 4.2 m
Purpose: Asset condition	Shape/Size: Circular dia 100 mm	
Use: Surface water	Material: Vitrified clay	Pipe length:
Catchment:	Lining:	
	Category:	
Comment:		
Location details:		

1:50	position	code	observation	counter	photo	grade
	0.00	MH	Manhole Remark: DR1 A	00:00:00		(Constr) 0
	0.00	ST	Start of Survey	00:00:00		(Misc) 0
	0.00	WL	Water level, 0 % height/diameter	00:00:00		(Serv) 0
	0.60	LL	Line of Sewer deviates left, Remark: Slight deviation.	00:00:00		(Serv) 0
	0.80	LU	Line of Sewer deviates up	00:00:00		(Serv) 0
	0.80	LL	Line of Sewer deviates left, Remark: Slight deviation.	00:00:00		(Serv) 0
	1.50	JDL	Joint displaced. Large	00:00:00	5_7a	(Struct) 2
	1.50	LD	Line of Sewer deviates down	00:00:00		(Serv) 0
	4.10	JN	Junction at 09 o'clock, dia 100 mm	00:00:00	5_9a	(Constr) 0
	4.20	FH	Finish Survey, Remark: MH9 reached.	00:00:00		(Misc) 0

Structural Defects	Constructional Features
Service Defects	Miscellaneous Features

49962CCTV-02.mdb // page: 11

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52

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Inspection photos

Place: Broadfield Slopes	Road: The Esplanade	Date: 10/07/2023	section number: 5	PLR: DR1 A X
------------------------------------	-------------------------------	----------------------------	-----------------------------	------------------------



Photo: 5_7a, 00:00:00
1.5m, Joint displaced. Large



Photo: 5_9a, 00:00:00
4.1m, Junction at 09 o'clock, dia 100 mm

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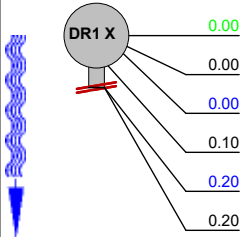
Inspection report

Date: 10/07/2023	Job N°: 49962	Weather: Heavy rain	Operator:	section number: 6	PLR: DR1 X X
Present:	Vehicle:	Camera:	Preset:	Cleaned: No	Grade:

Road: The Esplanade	Division:	start MH: DR1 X
Place: Broadfield Slopes	District:	end MH:
Location:	Tape No.:	Total length: 0.2 m
Purpose: Asset condition	Shape/Size: Circular dia 100 mm	
Use: Surface water	Material: Vitrified clay	Pipe length:
Catchment:	Lining:	
	Category:	

Comment:

Location details:

1:25	position	code	observation	counter	photo	grade
	0.00	MH	Manhole Remark: DR1 X	00:00:00		(Constr) 0
	0.00	ST	Start of Survey	00:00:00		(Misc) 0
	0.00	WL	Water level, 0 % height/diameter	00:00:00		(Serv) 0
	0.10	GP	General Condition photograph	00:00:00	54_4a	(Misc) 0
	0.20	LD	Line of Sewer deviates down	00:00:00		(Serv) 0
	0.20	SA	Survey abandoned, Remark: Unable to survey beyond deviation.	00:00:00		(Misc) 0

Structural Defects	Constructional Features
Service Defects	Miscellaneous Features



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Inspection photos

Place: Broadfield Slopes	Road: The Esplanade	Date: 10/07/2023	section number: 6	PLR: DR1 X X
-----------------------------	------------------------	---------------------	----------------------	-----------------



Photo: 54_4a, 00:00:00
0.1m, General Condition photograph

Project Details

Site Address	Broadfield Slopes
Town / City	Rochdale
Post Code	
Date	16-06-2023
Customer / Client	Price & Myers LLP

Manhole Report

MH (Node) Reference: MH1	Date: 16-06-2023
--------------------------	------------------

Location: MH1		Location Type: Difficult Access	
Function: Combined		Node Type: MH - Manhole	Status: PR - Private
Latitude:	Longitude:	Year Installed: Z - Others	Area Code:

LEVELS	Depth (m):	Cover Level (m):	Invert Level (m):
COVER	Shape: C - Circular	Hinged? ☐	Dimension 1:
	Duty: Light	Locked? ☐	Dimension 2:

CHAMBER				
Construction: Brick	Access:	Access Shaft? ☐	Reducing Slab? ☐	Landing Slab? ☐
Chamber Dimensions	Dimension 1 (Dia.):	Shaft Dimensions(if present)		Dimension 1 (Dia.):
	Dimension 2:			Dimension 2:
Flow Depth (mm):	Sluice Gate? ☐	Grating? ☐	Penstock? ☐	
Silt Depth (mm):	Flap Valve? ☐	Headwall? ☐	Screens? ☐	
Surcharge Level (mm):	Orifice? ☐	Flume? ☐	Weir? ☐	

Condition	Cover	Shaft	Step Irons/Ladders	Chamber	Benching	Half Channel
Good	☐	☐	☐	☐	☐	☐
Fair	☒	☒	☒	☒	☐	☐
Poor	☐	☐	☐	☐	☐	☐
Not Present	☐	☐	☐	☐	☐	☐

INCOMING PIPES

Letter	Upstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
A		Circular	100	x		VC - Vitrified Clay	Not present	1.62
B		Circular	150	x		VC - Vitrified Clay	Not present	
C		Circular	100	x		VC - Vitrified Clay	Not present	0.79

OUTGOING PIPES

Letter	Downstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
X		Circular	150	x		VC - Vitrified Clay	Not present	1.81

Evidence of misconnection? ☐

Manhole Images



Additional Notes and/or Recommendations

Manhole Report

MH (Node) Reference: MH2

Date: 16-06-2023

Location: MH2		Location Type: Difficult Access	
Function: Combined		Node Type: MH - Manhole	Status: PR - Private
Latitude:	Longitude:	Year Installed:	Area Code:

LEVELS	Depth (m):	Cover Level (m):	Invert Level (m):
--------	------------	------------------	-------------------

COVER	Shape: C - Circular	Hinged? ☐	Dimension 1:
	Duty: Light	Locked? ☐	Dimension 2:

CHAMBER					
Construction: Brick		Access:	Access Shaft? ☐	Reducing Slab? ☐	Landing Slab? ☐
Chamber Dimensions	Dimension 1 (Dia.):		Shaft Dimensions(if present)	Dimension 1 (Dia.):	
	Dimension 2:			Dimension 2:	
Flow Depth (mm):		Sluice Gate? ☐		Grating? ☐	Penstock? ☐
Silt Depth (mm):0.8		Flap Valve? ☐		Headwall? ☐	Screens? ☐
Surcharge Level (mm):		Orifice? ☐		Flume? ☐	Weir? ☐

Condition	Cover	Shaft	Step Irons/Ladders	Chamber	Benching	Half Channel
Good	☐	☐	☐	☐	☐	☐
Fair	☐	☒	☐	☒	☐	☐
Poor	☒	☐	☐	☐	☐	☐
Not Present	☐	☐	☒	☐	☒	☒

INCOMING PIPES

Letter	Upstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
A		Circular	100	x		VC - Vitrified Clay	Not present	0.71
		Circular		x		VC - Vitrified Clay	Not present	

OUTGOING PIPES

Letter	Downstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
X		Circular	75	x		VC - Vitrified Clay	Not present	0.73

Evidence of misconnection? ☐

Manhole Images



Additional Notes and/or Recommendations

Manhole Report

MH (Node) Reference: MH3	Date: 16-06-2023
--------------------------	------------------

Location: MH3		Location Type: Difficult Access	
Function: Combined		Node Type: MH - Manhole	Status: PR - Private
Latitude:	Longitude:	Year Installed:	Area Code:

LEVELS	Depth (m):	Cover Level (m):	Invert Level (m):
COVER	Shape: C - Circular	Hinged? ☒	Dimension 1:
	Duty: Heavy	Locked? ☐	Dimension 2:

CHAMBER				
Construction:	Access:	Access Shaft? ☐	Reducing Slab? ☐	Landing Slab? ☐
Chamber Dimensions	Dimension 1 (Dia.):	Shaft Dimensions(if present)		Dimension 1 (Dia.):
	Dimension 2:			Dimension 2:
Flow Depth (mm):	Sluice Gate? ☐	Grating? ☐	Penstock? ☐	
Silt Depth (mm):	Flap Valve? ☐	Headwall? ☐	Screens? ☐	
Surcharge Level (mm):	Orifice? ☐	Flume? ☐	Weir? ☐	

Condition	Cover	Shaft	Step Irons/Ladders	Chamber	Benching	Half Channel
Good	☐	☐	☐	☐	☐	☐
Fair	☒	☒	☐	☒	☐	☐
Poor	☐	☐	☐	☐	☐	☐
Not Present	☐	☐	☒	☐	☐	☐

INCOMING PIPES

Letter	Upstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
A		Circular	100	x		PVC - Polyvinyl Chloride	Not present	0.71
B		Circular	225	x		VC - Vitrified Clay	Not present	1.36

OUTGOING PIPES

Letter	Downstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
X		Circular	225	x		VC - Vitrified Clay	Not present	2.65

Evidence of misconnection? ☐

Manhole Images



Additional Notes and/or Recommendations

Manhole Report

MH (Node) Reference: MH5	Date: 16-06-2023
--------------------------	------------------

Location: MH5		Location Type: Road		
Function: Combined		Node Type: MH - Manhole		Status: PU - Public
Latitude:	Longitude:	Year Installed:	Area Code:	

LEVELS	Depth (m):	Cover Level (m):	Invert Level (m):
COVER	Shape: C - Circular	Hinged? ☐	Dimension 1:
	Duty: Medium	Locked? ☐	Dimension 2:

CHAMBER				
Construction: Others	Access:	Access Shaft? ☐	Reducing Slab? ☐	Landing Slab? ☐
Chamber Dimensions	Dimension 1 (Dia.):		Shaft Dimensions(if present)	Dimension 1 (Dia.):
	Dimension 2:			Dimension 2:
Flow Depth (mm):		Sluice Gate? ☐	Grating? ☐	Penstock? ☐
Silt Depth (mm):		Flap Valve? ☐	Headwall? ☐	Screens? ☐
Surcharge Level (mm):		Orifice? ☐	Flume? ☐	Weir? ☐

Condition	Cover	Shaft	Step Irons/Ladders	Chamber	Benching	Half Channel
Good	☒	☒	☒	☒	☐	☐
Fair	☐	☐	☐	☐	☐	☐
Poor	☐	☐	☐	☐	☐	☐
Not Present	☐	☐	☐	☐	☐	☐

INCOMING PIPES

Letter	Upstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
A		Circular	300	x		VC - Vitrified Clay	Not present	

OUTGOING PIPES

Letter	Downstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
X		Circular	300	x		VC - Vitrified Clay	Not present	1.81

Evidence of misconnection? ☐

Manhole Images



Additional Notes and/or Recommendations

Manhole Report

MH (Node) Reference: MH4	Date: 16-06-2023
--------------------------	------------------

Location: MH4		Location Type: Road		
Function: Combined		Node Type: MH - Manhole		Status: PU - Public
Latitude:	Longitude:		Year Installed:	Area Code:

LEVELS	Depth (m):	Cover Level (m):	Invert Level (m):
COVER	Shape: C - Circular	Hinged? ☐	Dimension 1:
	Duty:	Locked? ☐	Dimension 2:

CHAMBER				
Construction:	Access:	Access Shaft? ☐	Reducing Slab? ☐	Landing Slab? ☐
Chamber Dimensions	Dimension 1 (Dia.):		Shaft Dimensions(if present)	Dimension 1 (Dia.):
	Dimension 2:			Dimension 2:
Flow Depth (mm):		Sluice Gate? ☐	Grating? ☐	Penstock? ☐
Silt Depth (mm):		Flap Valve? ☐	Headwall? ☐	Screens? ☐
Surcharge Level (mm):		Orifice? ☐	Flume? ☐	Weir? ☐

Condition	Cover	Shaft	Step Irons/Ladders	Chamber	Benching	Half Channel
Good	☐	☐	☐	☐	☐	☐
Fair	☐	☐	☐	☐	☐	☐
Poor	☐	☐	☐	☐	☐	☐
Not Present	☐	☐	☐	☐	☐	☐

INCOMING PIPES

Letter	Upstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
		Circular		x		VC - Vitrified Clay	Not present	

OUTGOING PIPES

Letter	Downstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
		Circular		x		VC - Vitrified Clay	Not present	

Evidence of misconnection? ☐

Manhole Images



Additional Notes and/or Recommendations

Manhole Report

MH (Node) Reference: MH6	Date: 16-06-2023
--------------------------	------------------

Location: MH6		Location Type: Footway	
Function: Combined		Node Type: MH - Manhole	Status: PU - Public
Latitude:	Longitude:	Year Installed:	Area Code:

LEVELS	Depth (m):	Cover Level (m):	Invert Level (m):
--------	------------	------------------	-------------------

COVER	Shape: C - Circular	Hinged? ☐	Dimension 1:
	Duty: Heavy	Locked? ☐	Dimension 2:

CHAMBER				
Construction: Plastic	Access:	Access Shaft? ☐	Reducing Slab? ☐	Landing Slab? ☐
Chamber Dimensions	Dimension 1 (Dia.):	Shaft Dimensions(if present)		Dimension 1 (Dia.):
	Dimension 2:			Dimension 2:
Flow Depth (mm):	Sluice Gate? ☐	Grating? ☐	Penstock? ☐	
Silt Depth (mm):	Flap Valve? ☐	Headwall? ☐	Screens? ☐	
Surcharge Level (mm):	Orifice? ☐	Flume? ☐	Weir? ☐	

Condition	Cover	Shaft	Step Irons/Ladders	Chamber	Benching	Half Channel
Good	☐	☐	☐	☐	☐	☐
Fair	☒	☒	☐	☒	☐	☐
Poor	☐	☐	☐	☐	☐	☐
Not Present	☐	☐	☒	☐	☐	☐

INCOMING PIPES

Letter	Upstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
A		Circular	100	x		VC - Vitrified Clay	Not present	
B		Circular	150	x		VC - Vitrified Clay	Not present	
C		Circular	100	x		VC - Vitrified Clay	Not present	

OUTGOING PIPES

Letter	Downstream Ref.	Pipe Shape	Height (Dia.)		Width	Pipe Material	Lining Material	Depth (m)
X		Circular	150	x		VC - Vitrified Clay	Not present	1.31

Evidence of misconnection? ☐

Manhole Images



Additional Notes and/or Recommendations



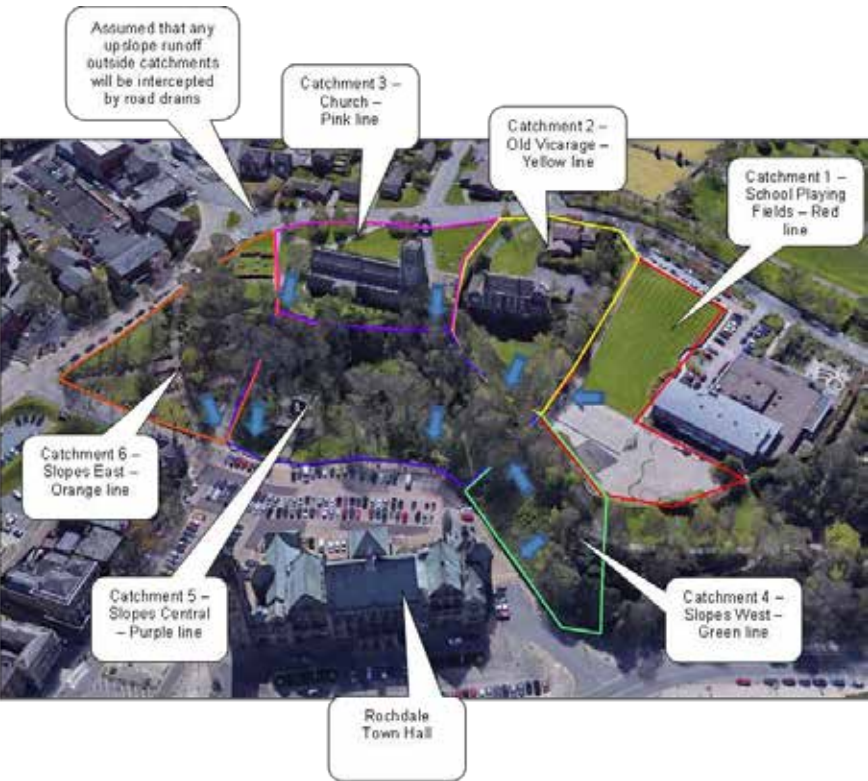
Appendix D

Hydrology

Rochdale Town Hall (RTH) Slopes
Hydrology Studies

September 2021 to August 2022

Rochdale Town Hall
The Esplanade
Rochdale
OL16 1AB



Executive Summary

This report presents the results of the investigations (between September 2021 & August 2022) of the hydrology and hydrogeology of the slopes to the south of Rochdale Town Hall (RTH).

In parallel to the on-going restoration of the RTH grade 1 listed building, the slopes immediately to the south are to be restored and renovated to provide a multipurpose outdoor public space.

The main objectives of the hydrology and hydrogeology studies of the slopes were to

- i) Investigate the evidence of the sources and evolution of the two historic water supply systems issuing from the slopes (Packer Spout water reservoir and Fulling Mill water supply) from their 18th Century origins to the present day
- ii) Provide information to help to restore the two water supply systems and as a result improve the drainage of the slopes
- iii) estimate the rates and volumes of runoff from 100 year return period storm events which may flow into the RTH drainage systems (which are currently being revised during the RTH restoration) as an aid to determining whether any additional flow attenuation structures may be required to avoid future flooding of RTH

It should be noted that as the slopes hydrology and hydrogeology studies have been based on a series of site visits and archive investigations, and that the observation, conclusions and recommendations in this final report supersede those in any previous Ambellia Consultancy reports relating to RTH.

This section presents the outline plan for restoring the two historic water supply systems on the slopes (Packer Spout and Fulling Mill) and for generally improving the slopes drainage and helping to avoid the overwhelming of the new RTH drainage during storm runoff conditions

The 6 slope runoff catchments defined for this report are shown in Fig. ExecSum.1.

Recommendations for the restoration of the Packer Spout Water Supply are listed in Fig. ExecSum.2.

Recommendations for the restoration of the Fulling Mill Water Supply are listed in Fig. ExecSum.3.

A Summary of RTH Slopes Catchments drainage problems and recommendations for remedies are listed in Fig. ExecSum.4.

Fig. ExecSum.1
'3D' Aerial Photo Showing 6 Main Water Runoff Catchments for the RTH Slopes
(Blue arrows indicate main exit routes for runoff from catchments)

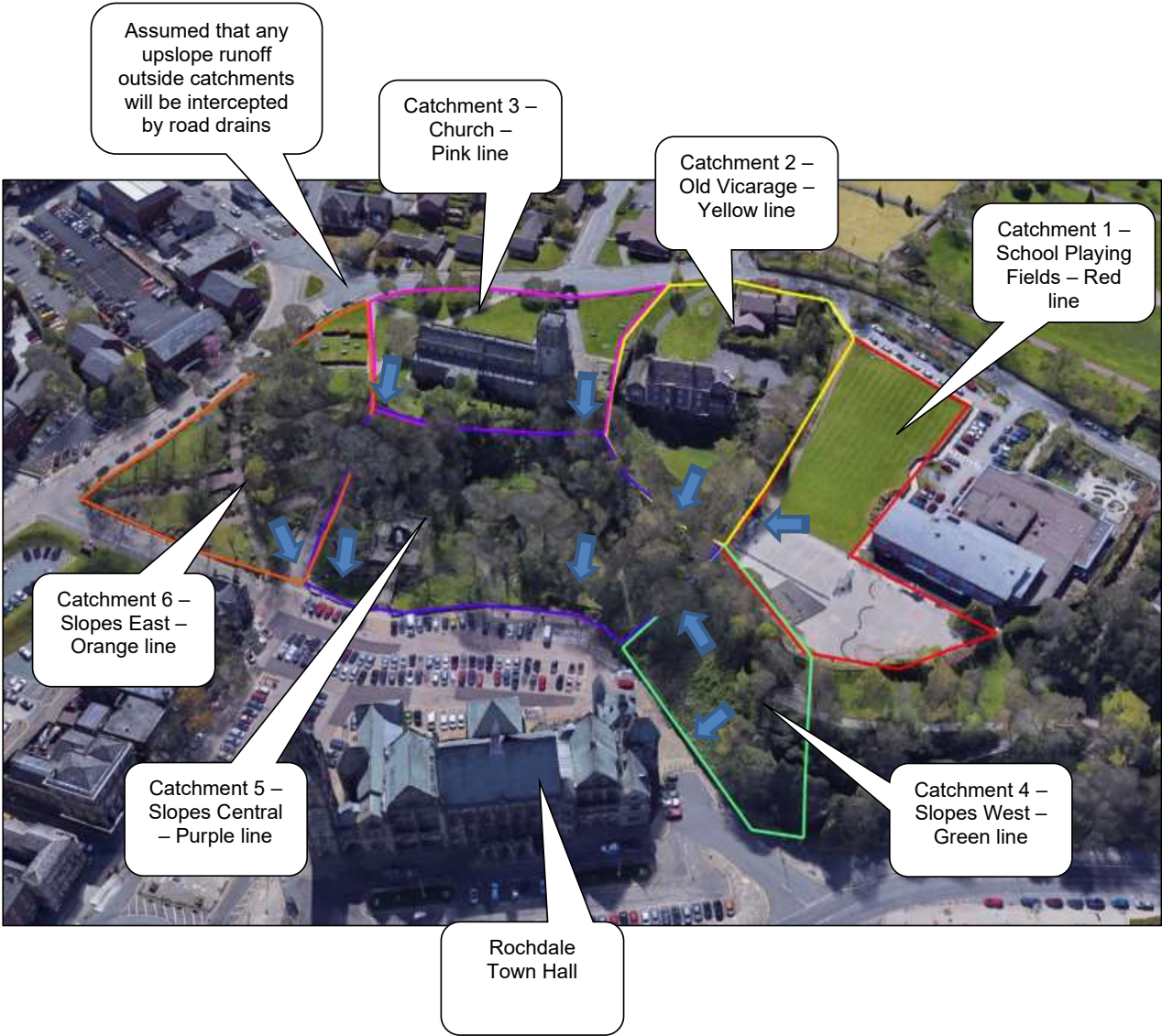


Fig. ExecSum.2
Recommendations for Restoration of the Packer Spout Water Supply

Item No.	Description
1	Restoring the catchpit for the main spring source so that sand is trapped in the catchpit and the spring water flows directly into half-round culvert and so removing the pond
2	Restoring the other 5 spring catchpits which can also act as sand/silt traps
3	Removing the back plastic pipes and other debris from the half round pipes
4	Refilling the round holes in the main west-east half-round culvert with lime mortar to stop water leaking to the slopes
5	Recovering the half-round culverts with the 90 cm natural stone slabs
6	Restoring the round brick sand/silt trap at the eastern end of the main half-round culvert
7	Restoring the stone culvert from the round brick chamber to the Packer Spout reservoir
8	Cleaning out the Packer Spout reservoir
9	Restoring the inflow and outflow controls for the Packer Spout reservoir
10	Laying new pipe from the Packer Spout Reservoir to the new RTH drainage to avoid current leaking drainage pipes
11	Determine if flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
12	Prepare maintenance plan including prevention of build-up of sand, silt and algae.

RTH Initial Hydrology Studies – Final Report

Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AB

Rev 00. August 2022.

Page 5 of 118

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Fig. ExecSum.3
Recommendations for Restoration of the Fulling Mill Water Supply

Item No.	Description
1	Restoring the complete length of the stone culvert from the western side of St Chad's church to the underground brick chamber (which is 11 m from the base of the slope)
2	Restoring the system catchpits and culverts which is draining the lower spring line and channelling water to the underground brick chamber.
3	Determine if flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
4	Prepare maintenance plan

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RTH Initial Hydrology Studies – Final Report

Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AB

Rev 00. August 2022.

Page 6 of 118

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Fig. ExecSum.4
Summary of RTH Slopes Catchments Drainage Problems and Remedy Recommendations

Catchment No.	Catchment Name	Problems	Remedy
1	School Playing Fields	Possible uncontrolled runoff from the school playing fields is possibly reaching the RTH slopes via the vicarage garden	If confirmed, divert runoff away from RTH slopes or if required install flow attenuation prior to water reaching RTH slopes
2	Old Vicarage (Partnership House)	Foul water from vicarage probably reaching RTH slopes combined with runoff	If confirmed, connect vicarage to mains drains for install PWTP
		Possible uncontrolled runoff from the vicarage is reaching the RTH slopes via the vicarage garden	If confirmed, install flow attenuation if required prior to water reaching RTH slopes
3	Church	Foul water from church probably reaching RTH slopes combined with runoff	If confirmed, connect church to mains drains for install PWTP
		Uncontrolled runoff from the church is reaching the RTH slopes	Install flow attenuation if required at the base of the slopes
4	Slopes West	Unknown extent and condition of drains on these very steep slopes	Monitor drainage of catchment. Avoid damage to any drains during new planting
5	Slopes central	Blocked lateral field drains at base of slope	The lateral land drains at the base of the slope need to be replaced, with the land drain gravel covered by a permeable geotextile membrane to prevent silt blocking the land drain gravel.
			Runoff attenuation should be installed if required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
6	Slopes East	Unknown extent and condition of drains on these very steep slopes	Investigate condition of significant drain for the surface runoff next to the base of the church steps
			Determine if storm runoff flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.

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Contents

Executive Summary
Contents
1.0 Introduction
2.0 1754 Initial Development of the Packer Spout Spring Water Supply
3.0 1760 Construction of Larger Packer Spout Reservoir
4.0 1764 Construction of Water Supply for Fulling Mill & Dyehouse for the Processing of Woven Wool Fabrics
5.0 Changes to the Two Water Supply Systems Following Construction of the Rochdale Town Hall and the Central Corn Mill
6.0 Changes to the Two Water Supply Systems Following the Demolition of the Central Corn Mill and Prior to Current RTH Restoration
7.0 Impact of March/April 2022 Tree Clearance on Slope Drainage
8.0 Remedial Action on Slope Drainage to Prevent Surface Runoff Reaching RTH
9.0 Hydrogeological Model for the Springs Water Flows for the Packer Spout and Fulling Mill Water Systems Flows
10.0 Summary of Slopes Catchments & 100 Year Return Period Storm Runoff Rates and Volumes
11.0 Slopes Catchments 1 – School Playing Fields - RTH Drainage Considerations
12.0 Catchment 2 – Old Vicarage (now Partnership House) - RTH Drainage Considerations
13.0 Catchment 3 – St Chad's Church - RTH Drainage Considerations
14.0 Catchment 4 – Slopes West - RTH Drainage Considerations
15.0 Catchment 5 – Slopes Central - RTH Drainage Considerations
16.0 Catchment 6 – Slopes East - RTH Drainage Considerations
17.0 Summary of Conclusions & Recommendations

This study can only be used in relation to Rochdale Town Hall, The Esplanade, Rochdale, OL16 1AB. Dr. Robin Evans trading as Ambellia Consultancy retains the copyright of the report. This report was prepared with every possible effort to ensure that the work conforms to the accepted scientific practice. However, Dr. Robin Evans, trading as Ambellia Consultancy, makes no warranty, expressed or implied, and assumes no legal liability or responsibility for the accuracy, completeness, or usefulness of any information, apparatus, product or process disclosed. Dr. Robin Evans, trading as Ambellia Consultancy shall not be responsible for any consequential loss, howsoever caused, associated with the use of this study.

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Rochdale Town Hall (RTH) Slopes Hydrology Studies

September 2021 to August 2022

1.0 Introduction

This report presents the results of the investigations (between September 2021 & August 2022) of the hydrology and hydrogeology of the slopes to the south of Rochdale Town Hall (RTH).

In parallel to the on going restoration of the RTH grade 1 listed building, the slopes immediately to the south are to be restored and renovated to provide a multipurpose outdoor public space.

The main objectives of the hydrology and hydrogeology studies of the slopes were to

- Investigate the evidence of the sources and evolution of the two historic water supply systems issuing from the slopes (Packer Spout water reservoir and Fulling Mill water supply) from their 18th Century origins to the present day
- Provide information to help to restore the two water supply systems and as a result improve the drainage of the slopes
- estimate the rates and volumes of runoff from 100 year return period storm events which may flow into the RTH drainage systems (which are currently being revised during the RTH restoration) as an aid to determining whether any additional flow attenuation structures may be required to avoid future flooding of RTH

It should be noted that as the slopes hydrology and hydrogeology studies have been based on a series of site visits and archive investigations, and that the observation, conclusions and recommendations in this final report supersede those in any previous Ambellia Consultancy reports relating to RTH.

A time line for the development to the 2 historical water supply systems on the slopes is presented in Fig. 1.1, along with a list of relevant sections in the report.

The Hydrogeological model for the 2 spring water sources on the slopes is shown in Fig. 1.2.

A summary of the area of the main storm water runoff catchments on the slopes is illustrated in Fig. 1.3.

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Fig. 1.1.
Summary of History of Water Use & Drainage on Broadfield Slopes to South of RTH
and Main Relevant Report Sections

Year	Activity	Main relevant report sections
1754	Initial development of Packer Spout water supply with small reservoir/trough in front of Leyland House next to church steps	2
1760	Stone Reservoir built for Packer Spout spring on east side of slopes to supply water to Packer Street, Fleece Street, Church Street area	3
1764	Springs and runoff from St Chad's Church, vicarage and west side of slopes used to power fulling mill on glebe land below the slopes	4
1864	Glebe land is purchased to build Rochdale Town Hall & fulling mill is demolished	5
1871	Rochdale Town Hall is completed	5
Early 1870's	Water powered corn mill is built adjacent to Packer Spout reservoir using it as water source to power water turbine. Fulling mill water supply also possibly used for a second smaller mill	5
1934	Corn mill is demolished and landscaped gardens created. Spring and storm runoff flow diverted around Packer Spout reservoir to create small waterfall features prior to entering storm drains under Packer Street. Water has to be pumped into Packer Spout to fill it. Fulling mill water drainage system is significantly damaged	6
1960's	Primary School built in western side of vicarage garden	11

Figure 1.2
RTH SE-NW Geological Cross-Section

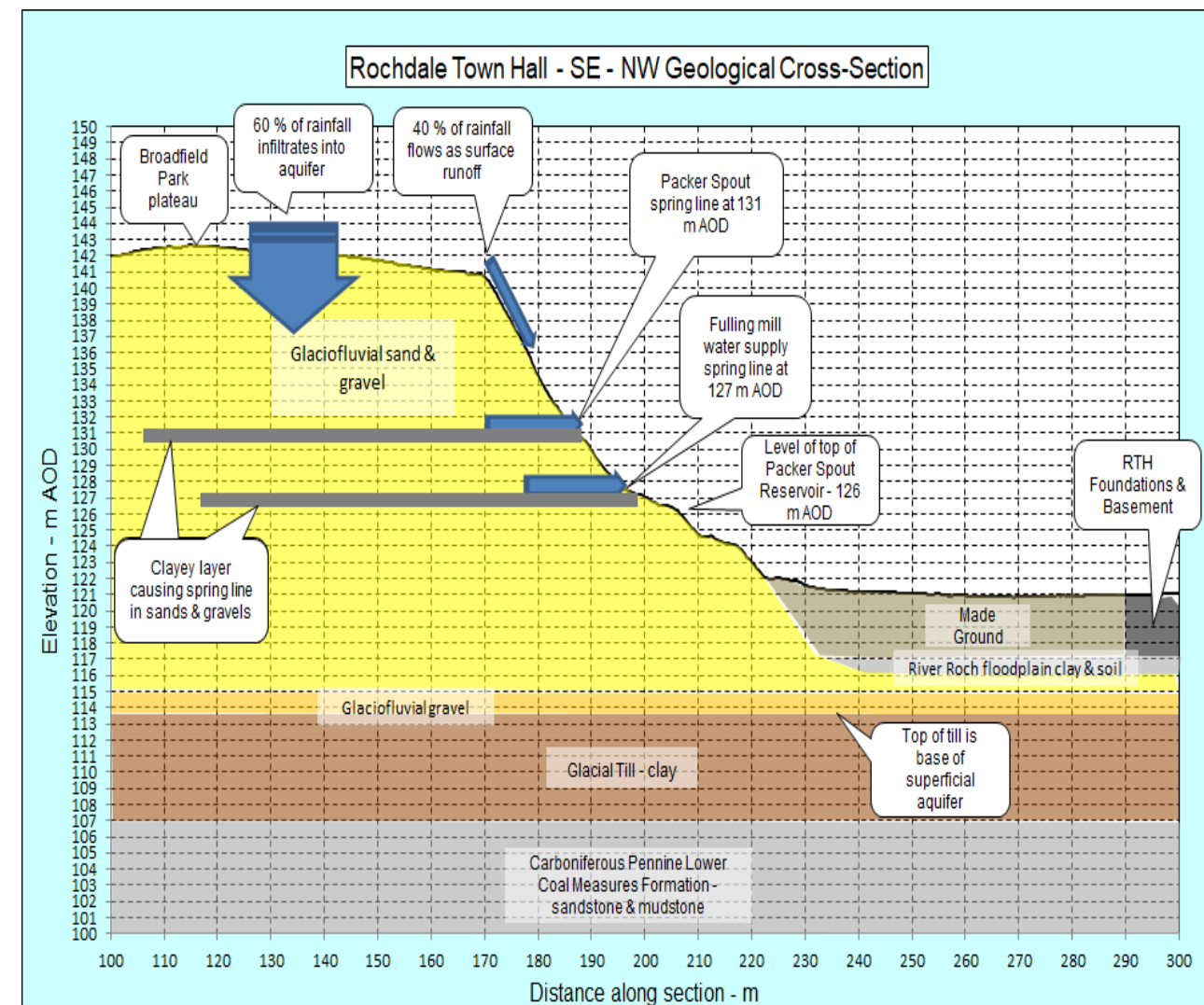
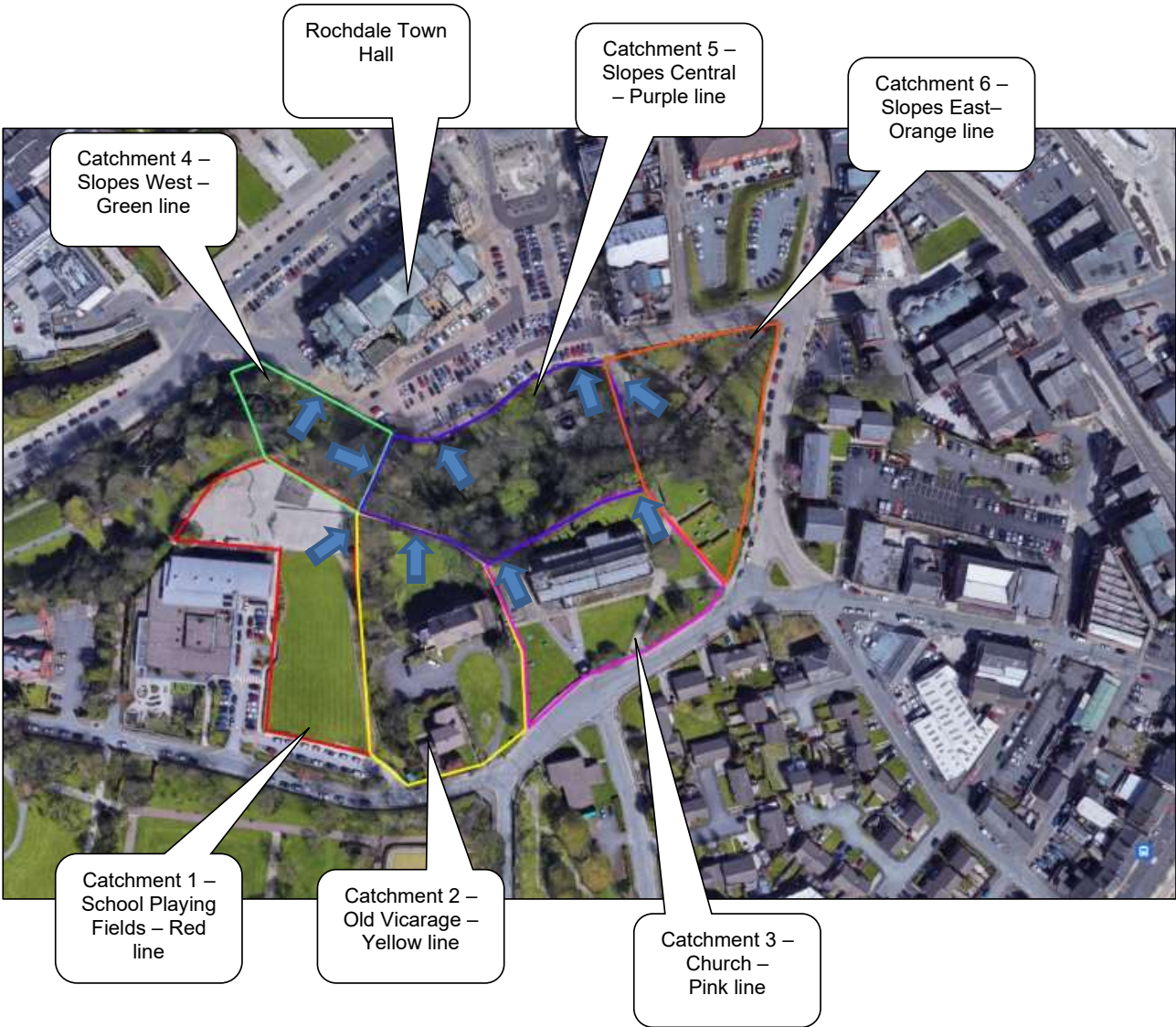


Figure 1.3
Aerial Photo Showing 6 Main Water Runoff Catchments for the RTH Slopes
(Blue arrows indicate main exit routes for runoff from catchments)



2.0 1754 Initial Development of the Packer Spout Spring Water Supply

Fig. 2.1 illustrates the original layout of the system of springs, spring catchpits and stone slab covered culverts which supplied water to the original Packer Spout trough in front of Leyland House which is believed to have been completed in 1754.

The springs feeding the original 1754 Packer Spout water trough were between 40 m and 75 m to the SW of the trough on the slopes area known as Leland Bank or Leylands Brow (Fig. 2.2).

Packer Spout, with its original publicly accessible water trough in front of Leyland House, was one of the first engineered water supplies in Rochdale (Fig. 2.3).

The original water trough was removed in 1927 when Leyland House was demolished, and the site is next to the Church Steps is now grassed over (Fig. 2.4).

Originally the main spring source, 75 m to the SW of the Leylands House trough, would have had catchpit similar to those at other spring sources supplying Packer Spout. The visible remnants of those catchpits indicate the presence of circular chambers which would have acted as sand/silt traps (Fig. 2.5).

To protect the spring water quality (and to prevent culvert blockages) there would have been no pond (Fig. 2.5) and the spring water would have flowed directly into the half-round culverts covered by stone slabs, as recreated in Fig. 2.6.

At the end of the half-round culvert section was a round brick chamber, which would have provided another sand/silt trap before the water flowed down a rectangular stone culvert to the water trough at Leylands House (Fig. 2.6).

Fig. 2.1.
Packer Spout Arrangement Pre-1760

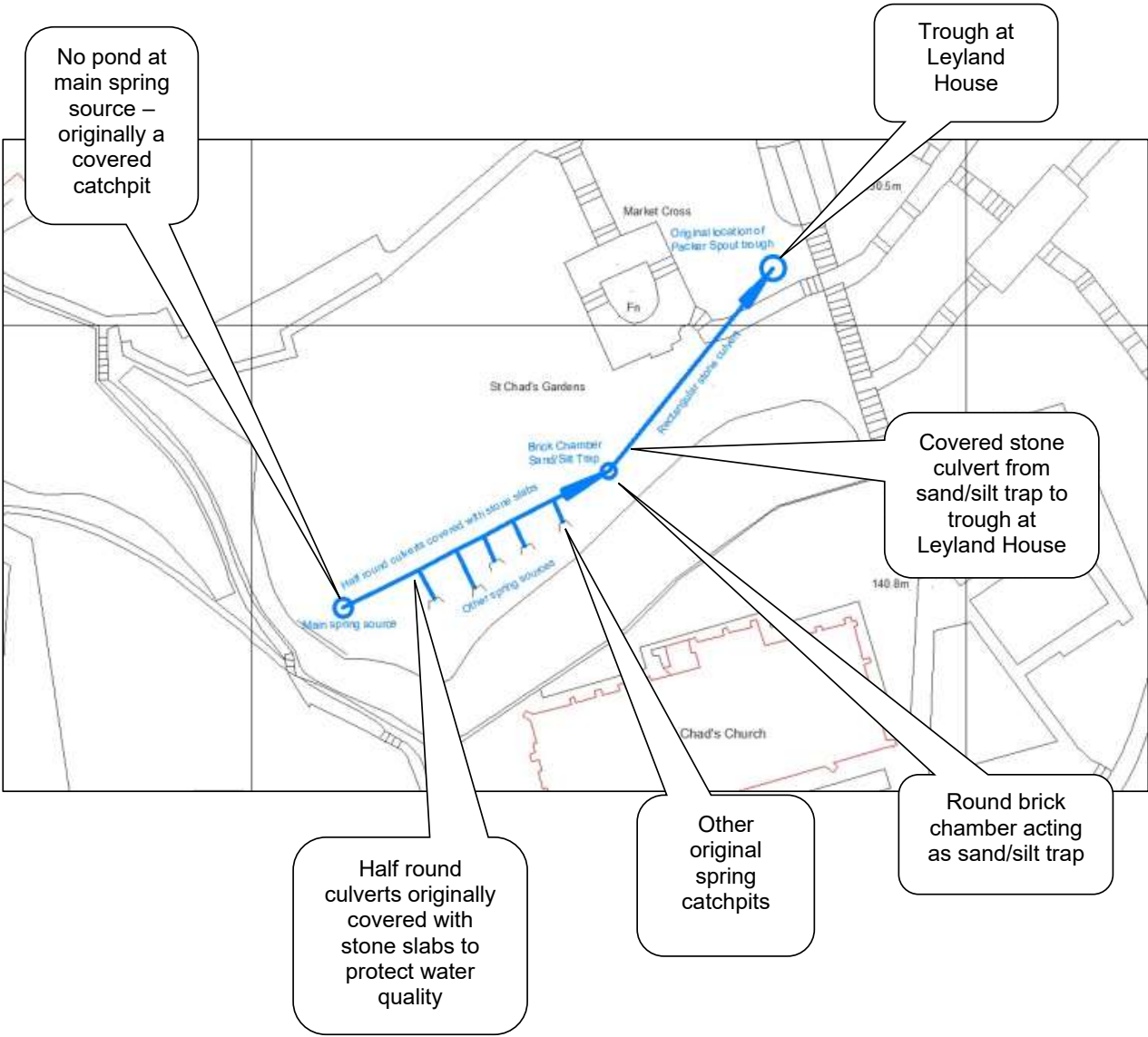


Fig. 2.2.
1754-1757 Rochdale Map Showing Leland Bank – Source of Packer Spout Springs

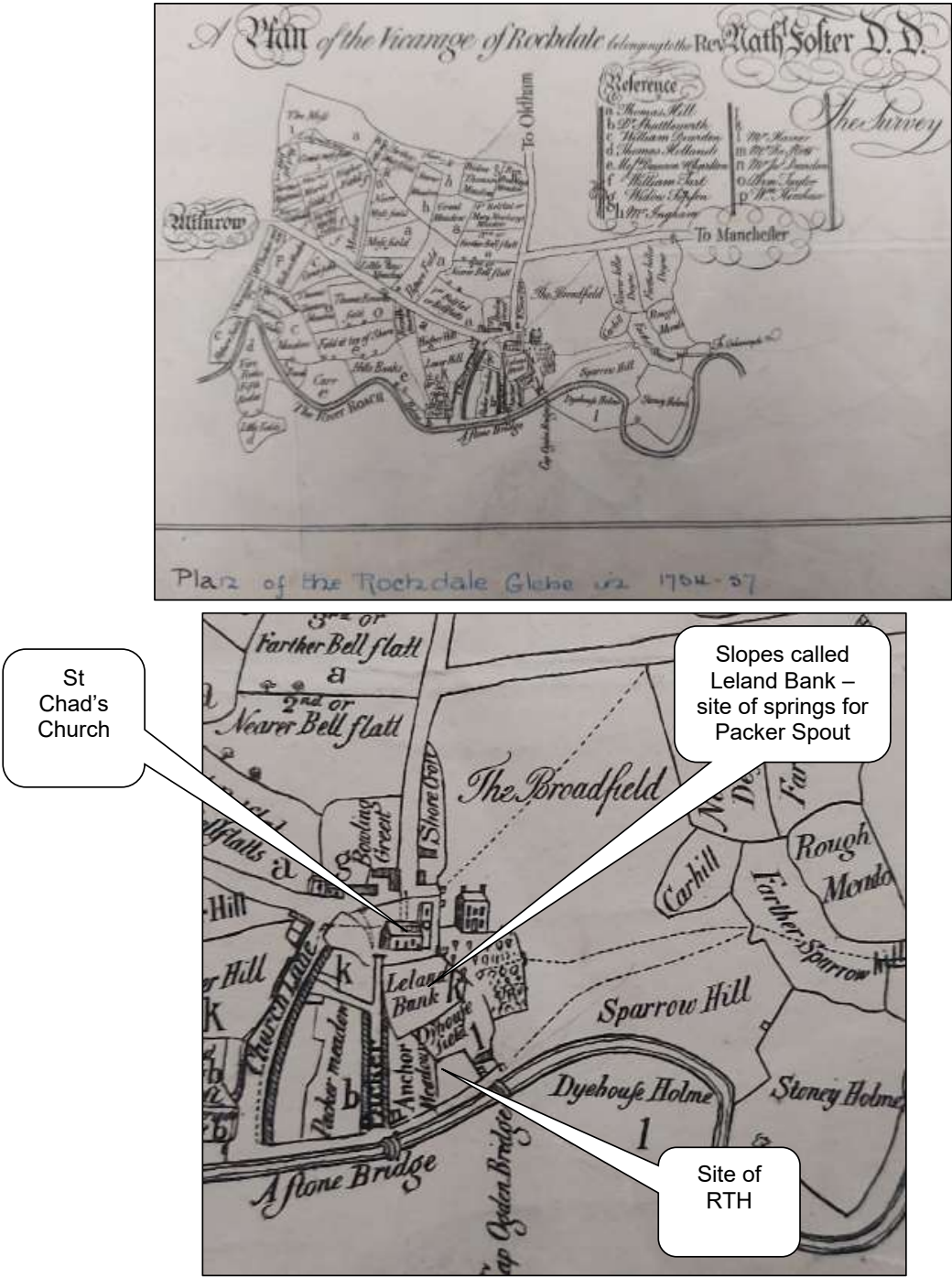


Fig. 2.3.
References to Packer Spout Prior to 1760

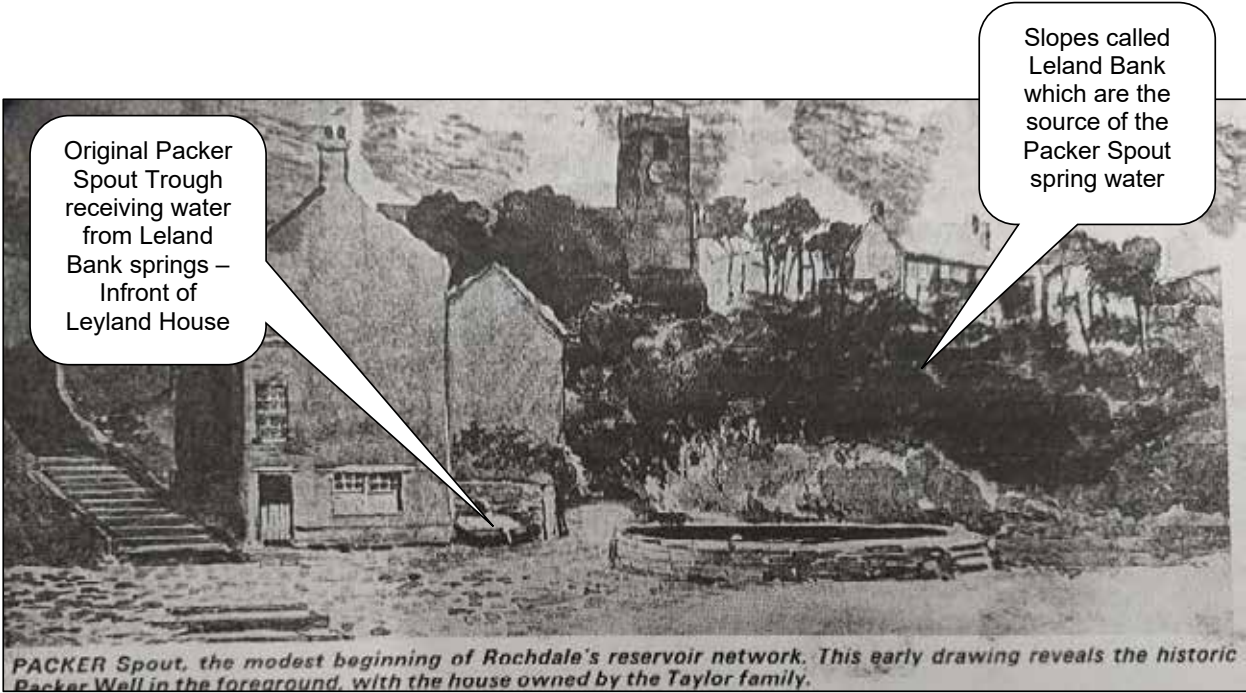
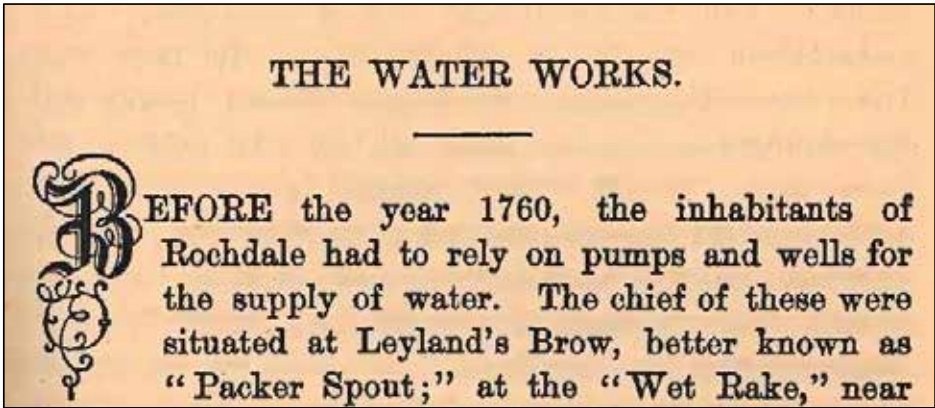


Fig. 2.4.
Original Packer Spout Trough Removed in 1927 when Leyland House was Demolished

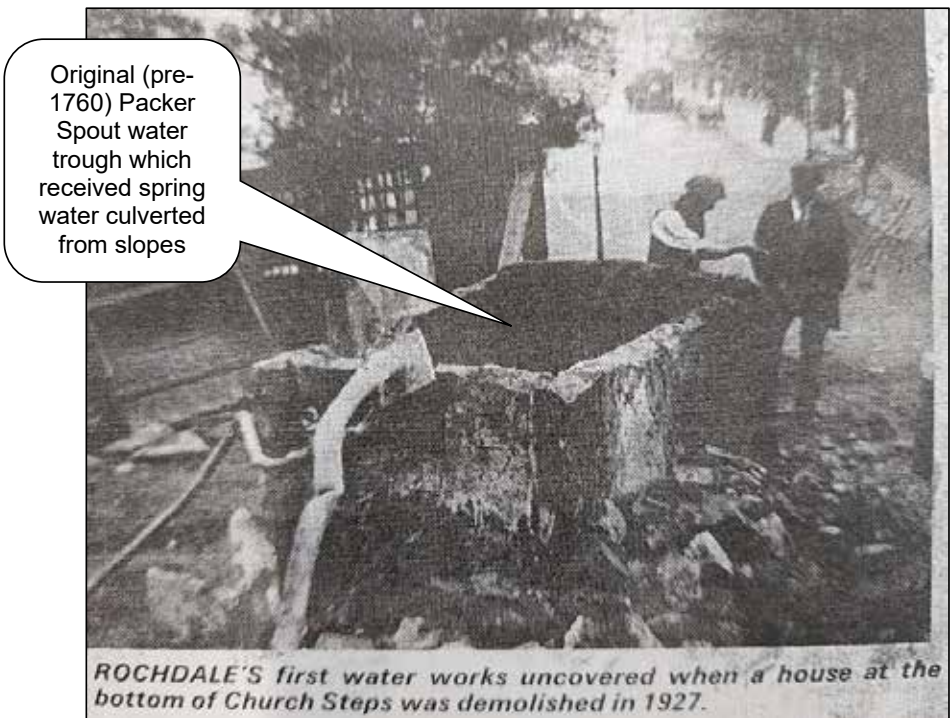


Fig. 2.5.
Main Spring Source Originally would Have had Covered Catchpit (i.e. No Pond)

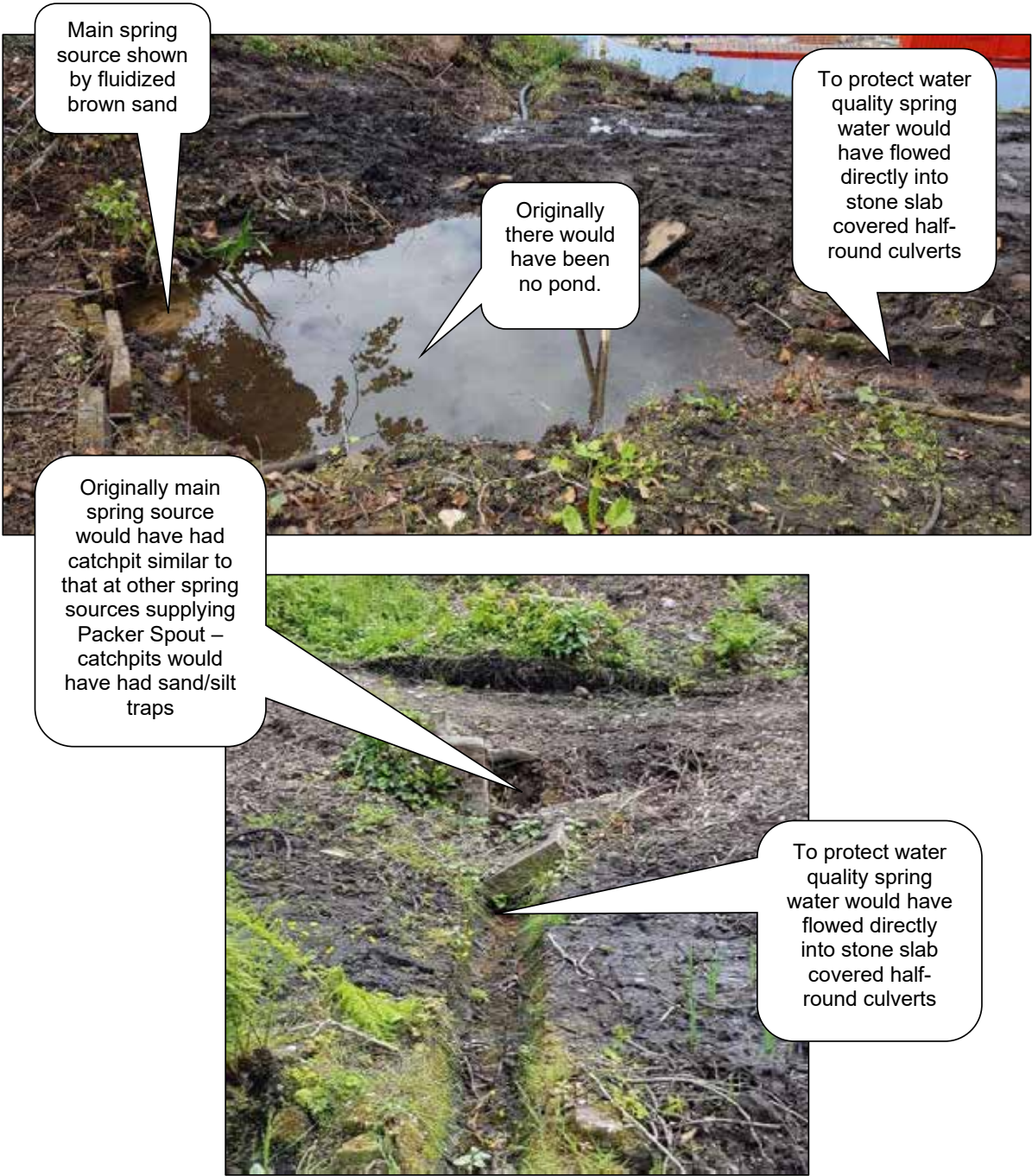
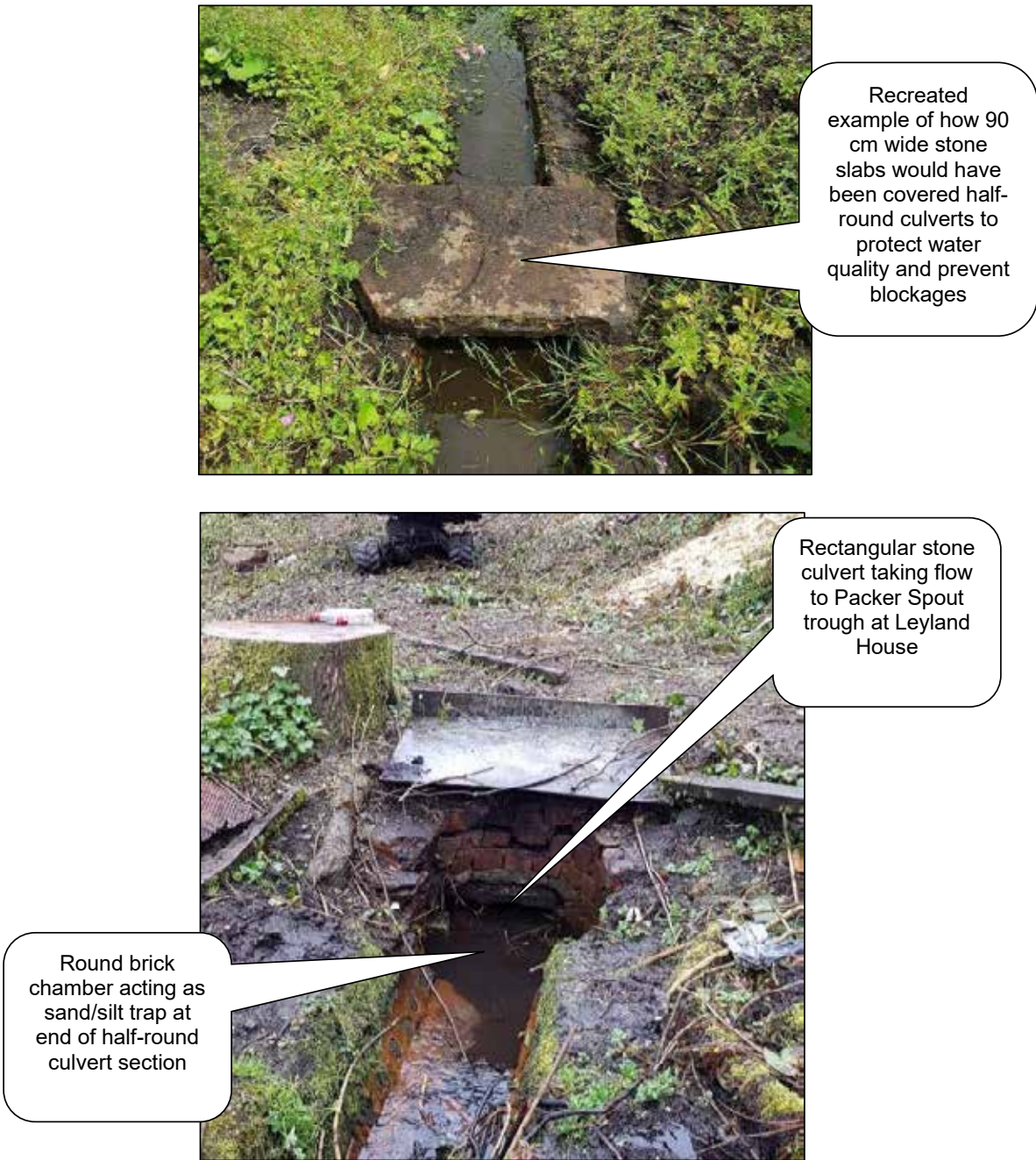


Fig. 2.6.
Example of How Half Round Culvert Section Would Have Been Covered by Stone Slabs & Round Brick Chamber Sand/Silt Trap Prior to Rectangular Stone Culvert Leading to Trough at Leyland House



3.0 1760 Construction of Larger Packer Spout Reservoir

In 1760 a larger stone reservoir was built about 20 m to the west of Leyland House with a capacity of about 70 m³ (Fig. 3.1 & 3.2).

This required the re-routing of the stone culvert from the brick chamber sand/silt trap, however the rest of the water collection system remained the same.

As shown in Fig. 3.3, the original stone culvert is still in place at the inlet of the reservoir.

The draining of the Packer Spout reservoir in June & July 2022 did not reveal any water outlet pipes near the base of the reservoir.

This raises the possibility that the water outlet for the reservoir could have consisted of a cast iron pipe siphon system in the NE corner of the reservoir (Fig. 3.4).

Most of the siphon pipe system has now been lost due to corrosion. The inlet to the syphon pipe in the reservoir would have had a check valve, and stop valves at the highest point of the siphon pipe and at the outlet would have enabled the priming of the siphon.

The outflow from the siphon pipe (which would have been lower than the level of the inlet pipe in the reservoir) entered a distribution chamber accessible via a manhole cover in the paving at the NE corner of the reservoir (Fig. 3.5). Outlet pipes from the distribution chamber sent water to the streets below in Rochdale.

Once the siphon outlet was primed the whole water distribution system would have operated using gravity with a maximum head of about 7 m of water (10 psi).

Fig. 3.1.
Packer Spout Arrangement With Larger 1760 Constructed Reservoir

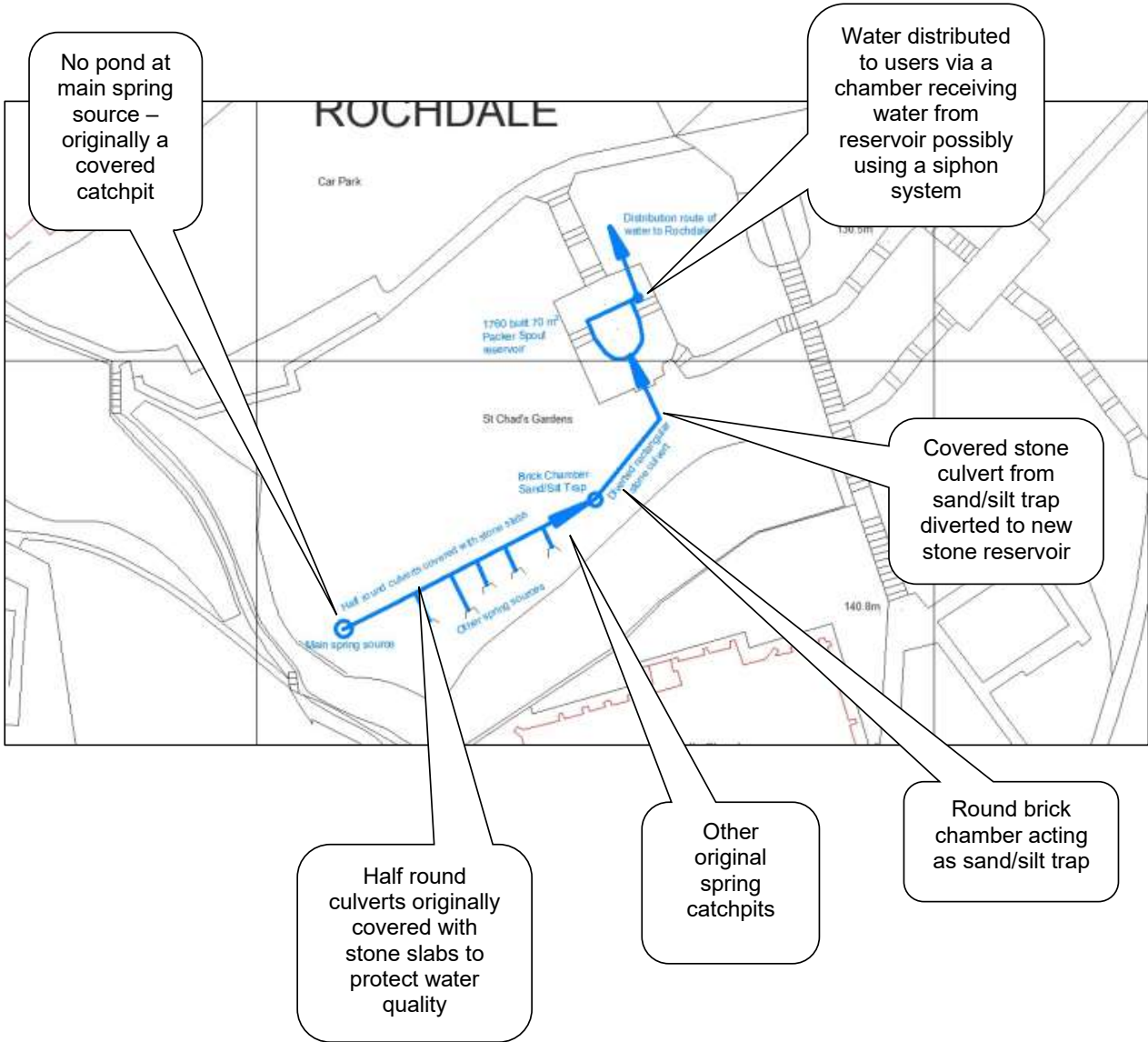
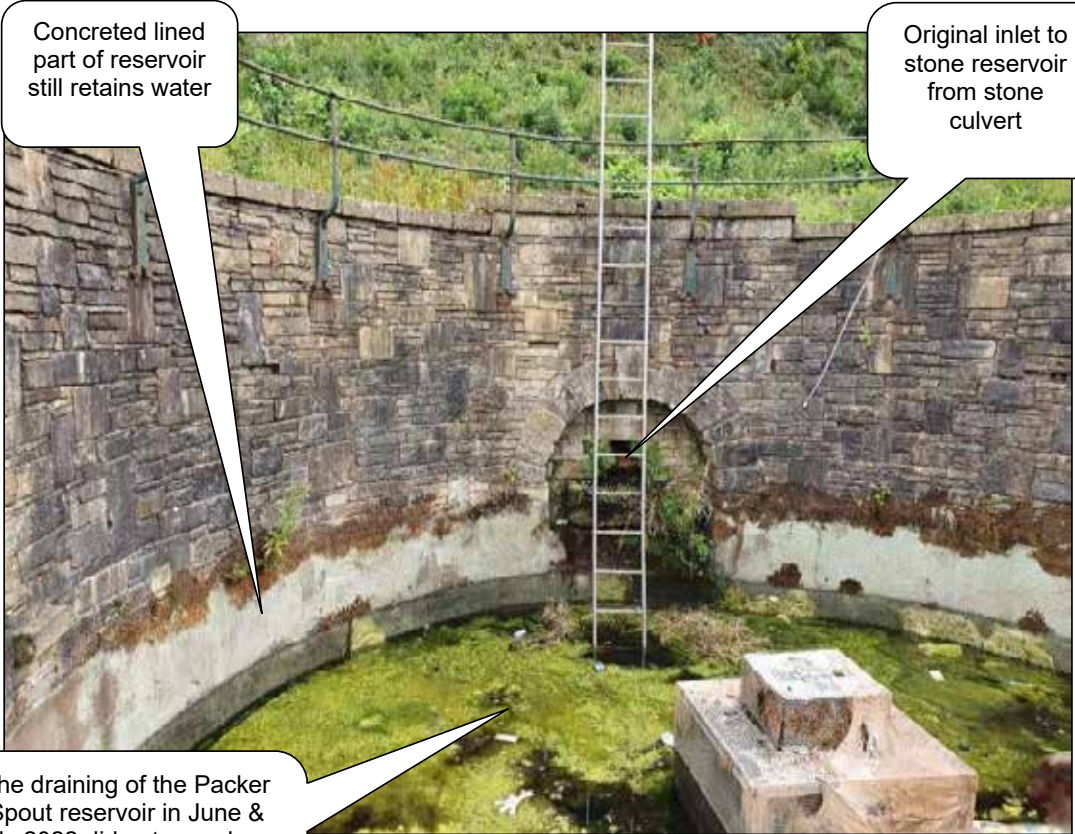


Fig. 3.2.
Larger Reservoir Constructed for Packer Spout in 1760

The first reservoir was constructed about the year 1760, by Messrs. Ralph and Samuel Taylor, and John Clegg. It is situated in Leyland's Brow, near the church steps, 25 feet above the centre of the town. Its low level confined the supply only to the inhabitants who resided in the lower parts of the town. However, its value in giving a supply of water inside the house led to the formation of a Water Works Company, who purchased it for an annuity of £80 per annum; and, in



The draining of the Packer Spout reservoir in June & July 2022 did not reveal any water outlet pipes near the base of the reservoir

Fig. 3.3.
Details of Original Water Inlet to 1760 Stone Reservoir

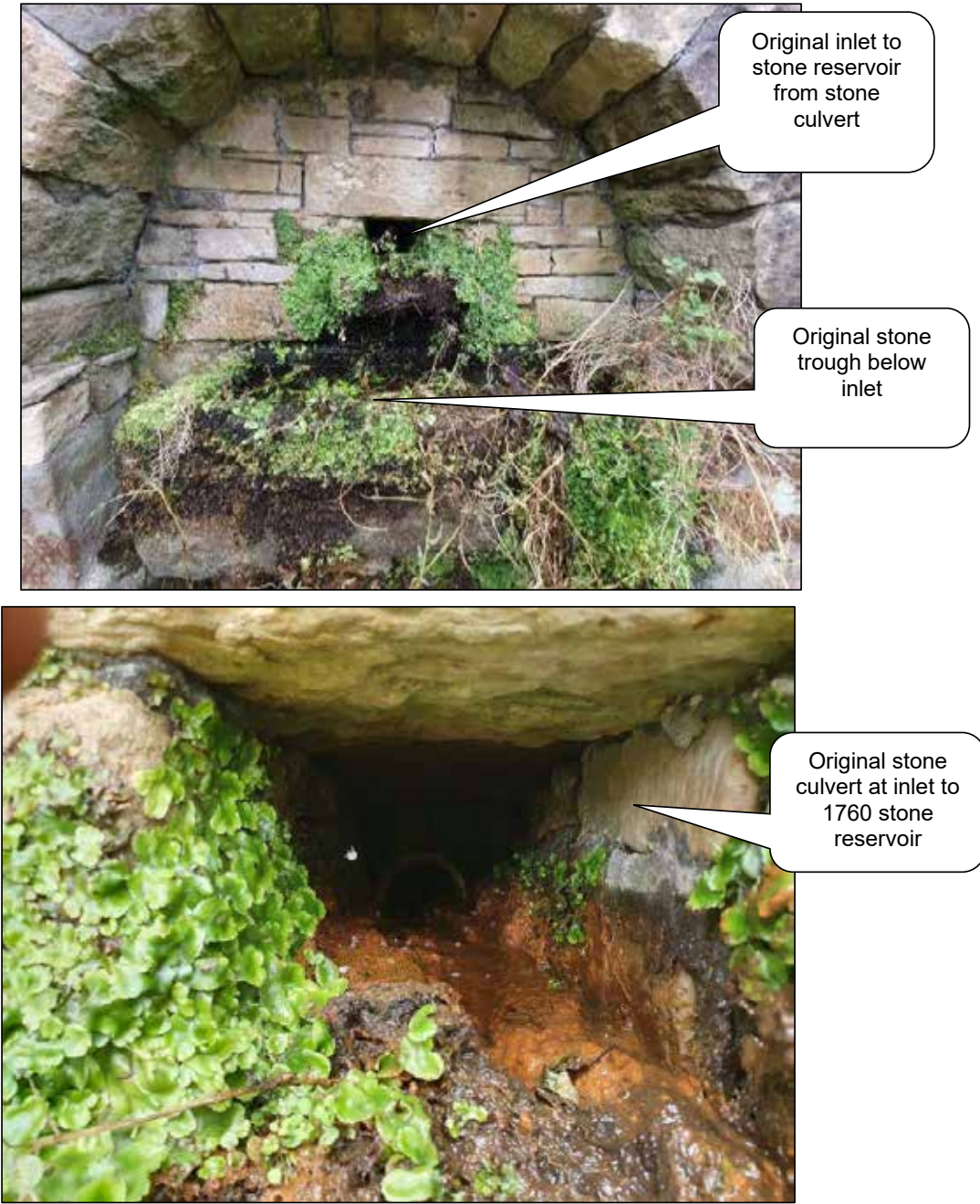


Fig. 3.4.
Details of Probable Original Siphon Water Outlet System for 1760 Stone Reservoir



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Fig. 3.5.
Details of Water Distribution Chamber Receiving Water from Siphon Outlet



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4.0 1764 Construction of Water Supply for Fulling Mill & Dyehouse for the Processing of Woven Wool Fabrics

In 1764 the glebe land on which the Rochdale Town Hall (RTH) now stands was leased by the vicar to John Hamer, and a water supply was constructed to provide water to a fulling mill and dyehouse for the processing of woven woollen cloth (Fig. 4.1).

The fulling process consists of the closing together of the threads of newly woven woollen fabric with the assistance of washing treatments and water powered mallets with the end purpose of producing a grease free cloth of the correct thickness for future use, including dyeing. The washing process involved the use of a number of different cleaning agents such as fuller's earth and stale urine. After mixing water and the cleaning agents together, the cloth was pounded by water powered fulling stocks or beaters (Fig. 4.1). Consequently, the water supply would have been vital to the fulling mill and dyeing operations.

The fulling mill water supply system was in operation at the same time as the Packer Spout system, and used different water sources to those supplying drinking water to the Packer Spout reservoir (Fig. 4.2).

The fulling mill water supply was provided by a combination of runoff from St Chad's Church, a lower spring line on the St Chad's Garden slopes and water supplies from the vicarage gardens to the SW (Fig. 4.2).

The water sources were culverted to an underground brick chamber which was probably used to store water and regulate the supply of water to the fulling mill.

Once used in the fulling mill, the water was culverted to the River Roach via the low lying glebe land on which the RTH was built.

The 1851 town map shows that the components of the fulling mill water supply were still in existence and where still channelling water to the River Roch (Fig. 4.3).

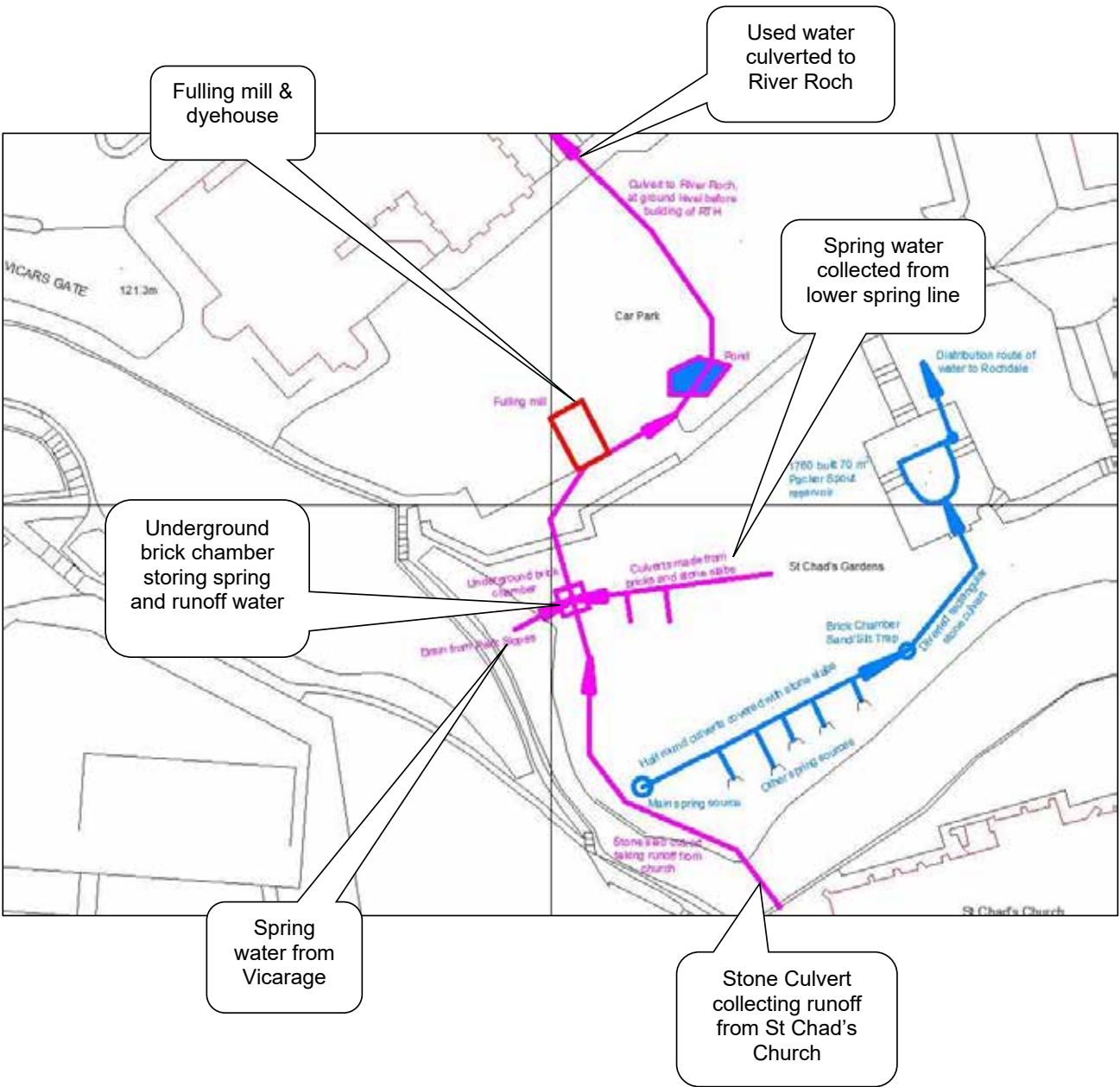
The stone culvert from St Chad's Church to the underground brick chamber still exists, however it has been covered with soil for part of its length and is blocked (Fig. 4.4).

The underground brick chamber still exists (Fig. 4.5), however the culverts which supplied spring water from the west and the east have become blocked (Fig. 4.6). As a result of this the lower spring line in St Chad's Garden is no longer drained and is a permanent bog.

Fig. 4.1.
Extract from Book *Reminiscences of Rochdale* - 1764 Lease of Glebe Land Including Supply of Water for a Fulling Mill & Dyehouse & Example of a Water Powered Fulling Mill

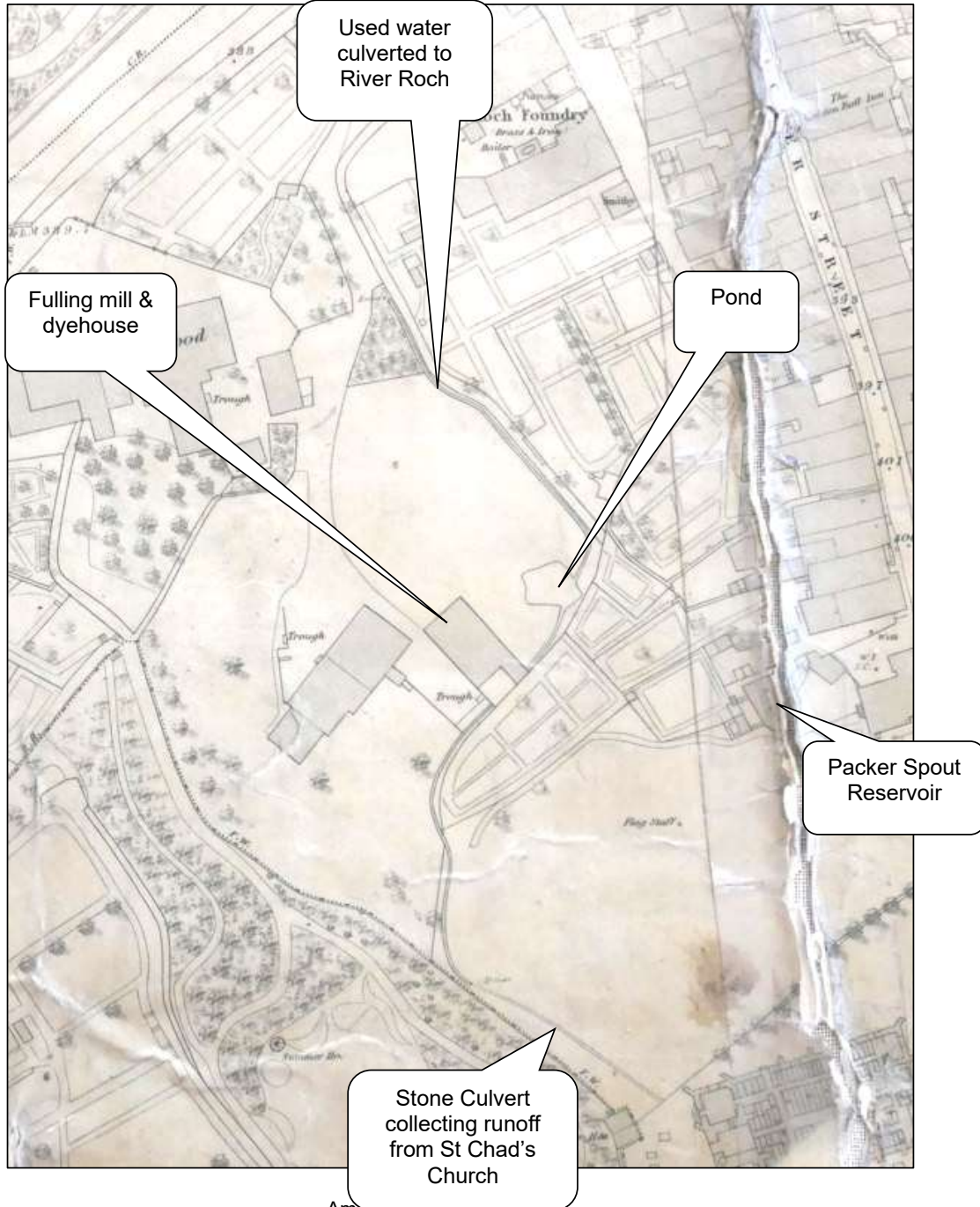


Fig. 4.2.
1764 Water Supply for Fulling Mill and Dyehouse (Purple Lines)
(Operating at Same Time as Packer Spout Drinking Water Supply)



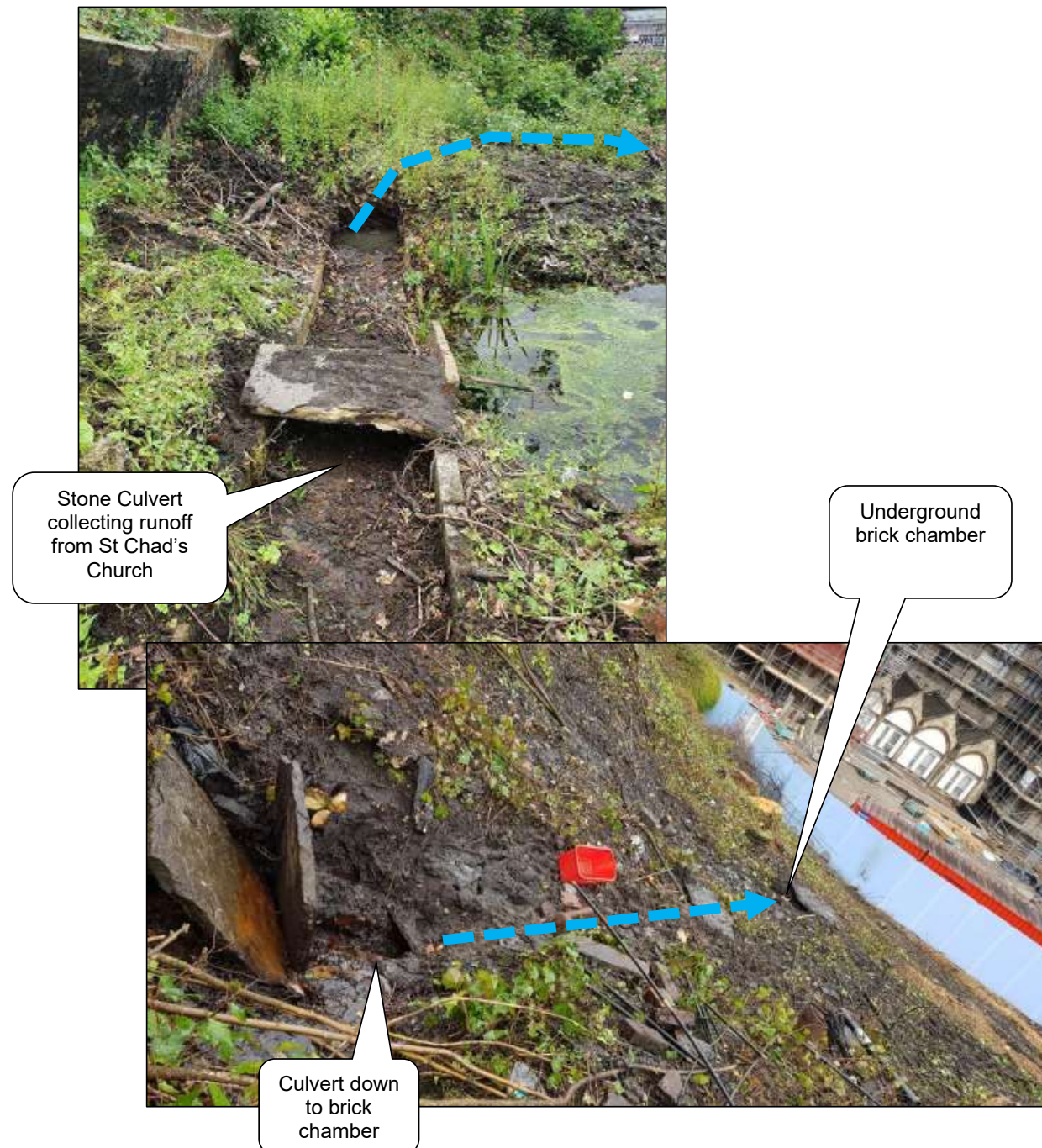
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Fig. 4.3.
Extract from 1851 Town Map Showing Details of 1764 Water Supply
for Fulling Mill and Dyehouse



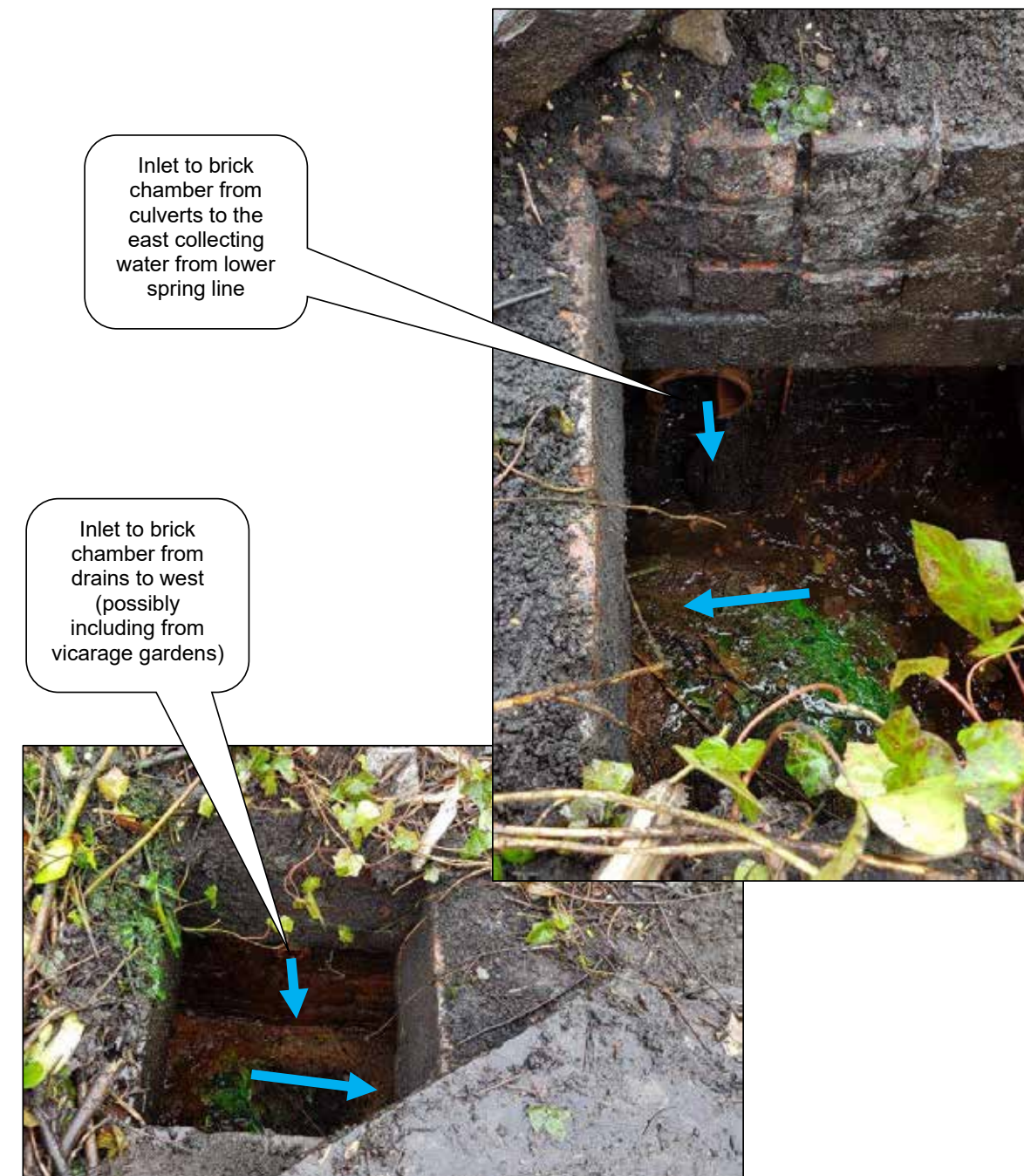
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Fig. 4.4.
Details of Stone Culvert Leading from St Chad's Church to
Underground Brick Chamber



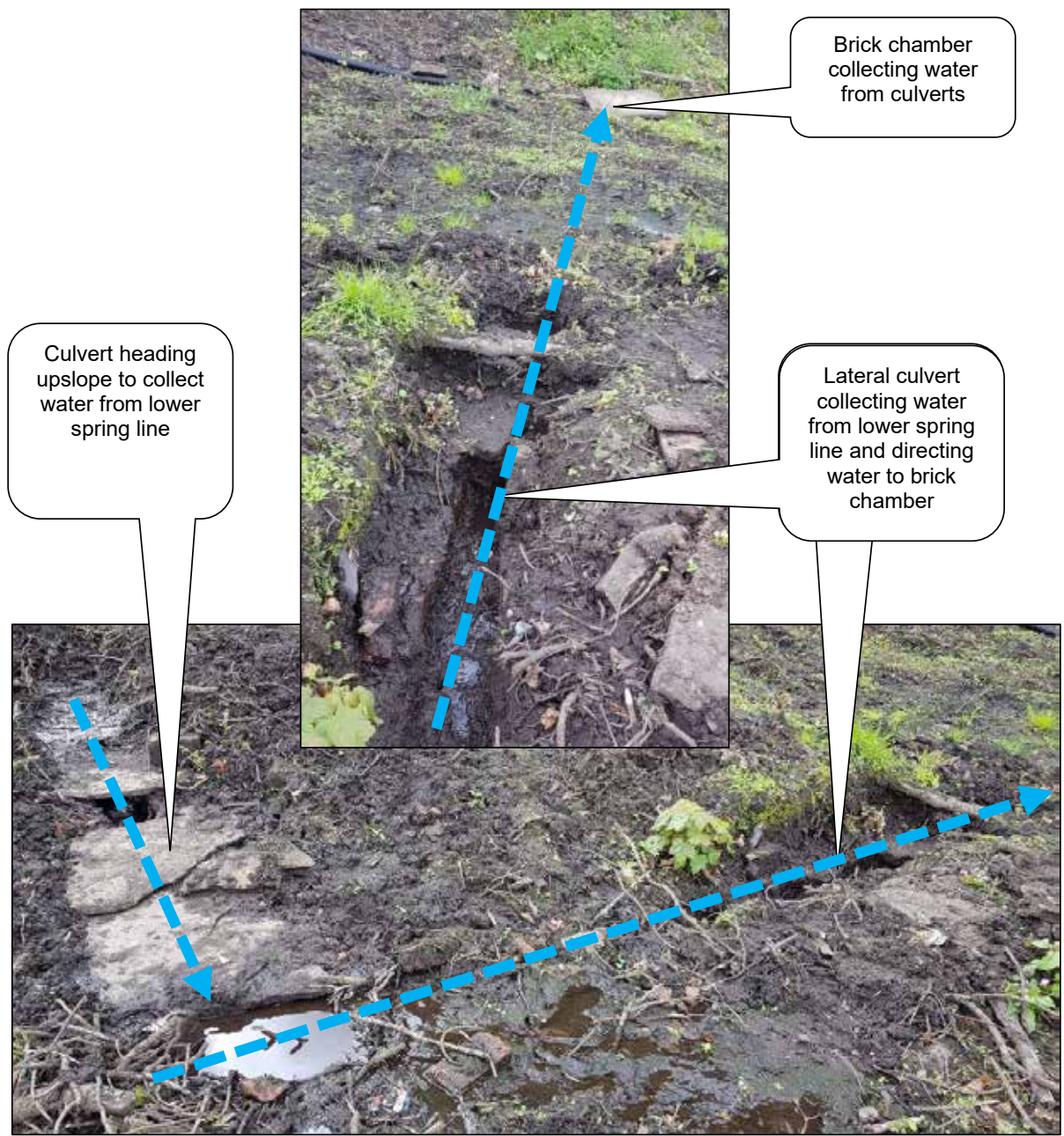
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Fig. 4.5.
Details of Underground Brick Chamber with Drain Inlets to the West & East



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Fig. 4.6.
Details of Culverts Collecting Water from Lower Spring Line and Channelling it to
Underground Brick Chamber



5.0 Changes to the Two Water Supply Systems Following Construction of the Rochdale Town Hall and the Central Corn Mill

In 1864 the glebe land including the Dyehouse Field, Anchor Meadow and the Wood Villa was purchased to build Rochdale Town Hall and the fulling mill is demolished.

By 1871 the construction of Rochdale Town Hall was completed (Fig. 5.1).

Also, in the early 1870's, a water powered central corn mill was built immediately to the north of the Packer Spout reservoir using it as water source to power water turbine (Figs. 5.2, 5.3 & 5.4).

The construction of the Rochdale Town Hall involved raising the ground level round the Town Hall by 4 m (Fig. 5.5).

It is probable that a second smaller water powered mill was built over the brick chamber of the fulling mill water supply (Fig. 5.5).

The changes to the use and routing of the Packer Spout and fulling mill water sources are summarised in Fig. 5.6.

The access chamber in Fig. 5.7 was probably for the culvert taking water away from the water turbine in the basement of the central corn mill to the River Roch via the eastern side of RTH.

A new 6" pipe was installed to take the fulling mill water supply to the River Roach via the western side of RTH (Fig. 5.8)

The 1876 *As Built RTH Drainage and Sewers Plan* (Fig. 5.9) gives details of 2 deep manholes (which both go down to the original pre RTH ground level) which were probably parts of the new routes of Packer Spout and Old Fulling Mill water sources to the River Roch.

Figure 5.1
 1910 Rochdale OS Town Map Following Construction of RTH and Central Corn Mill

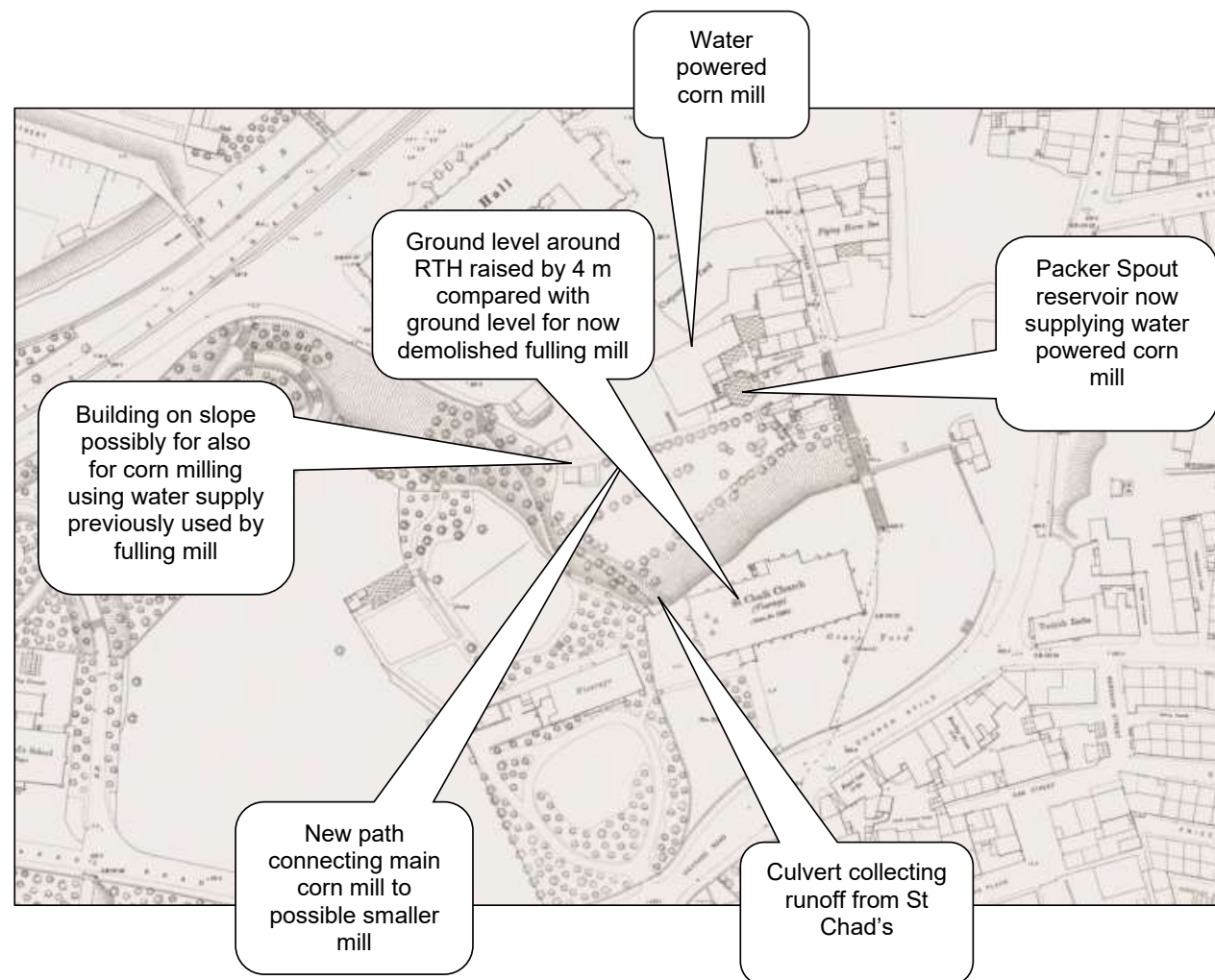


Fig. 5.2.
 Photo of Water Powered Central Corn Mill

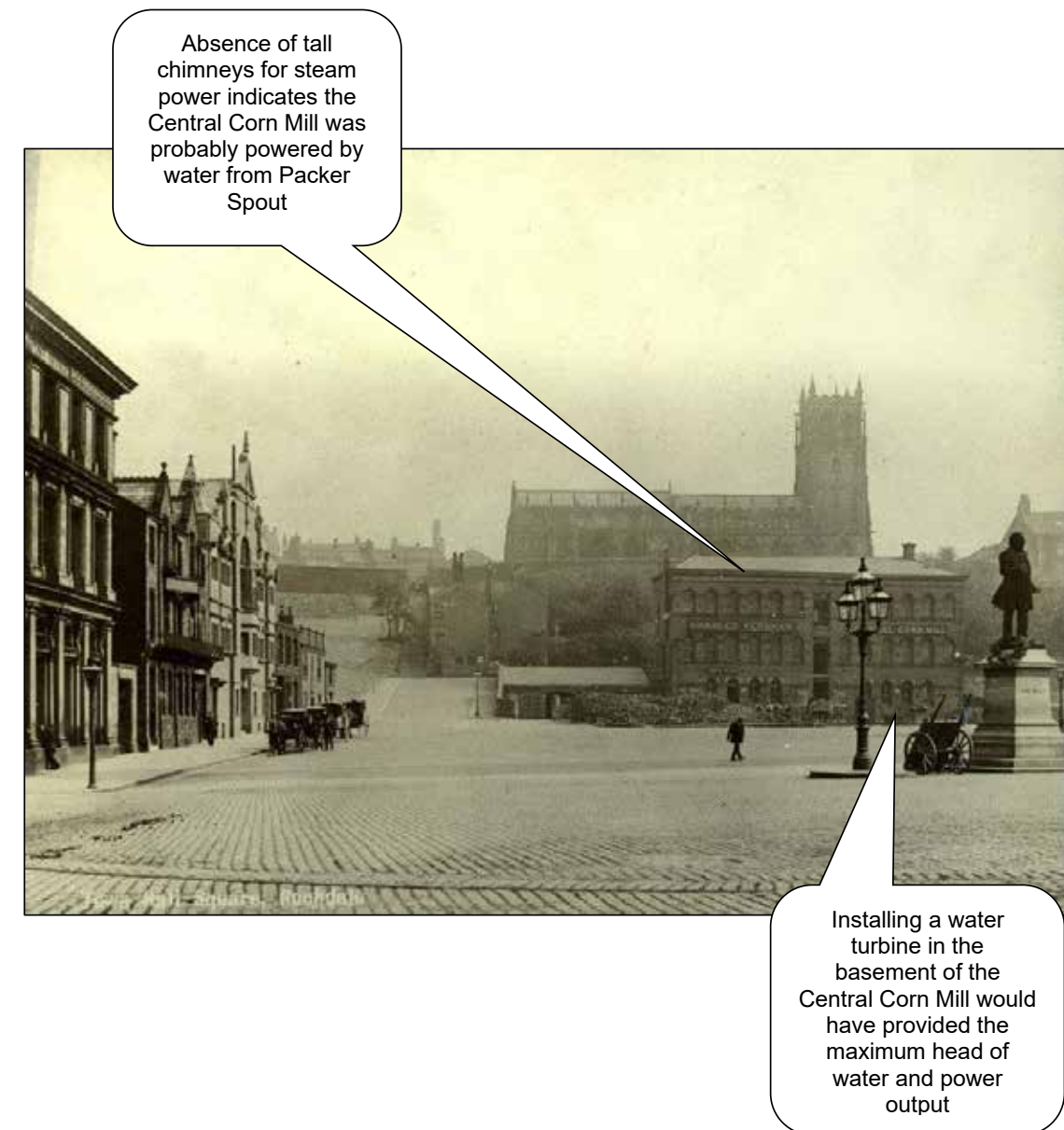
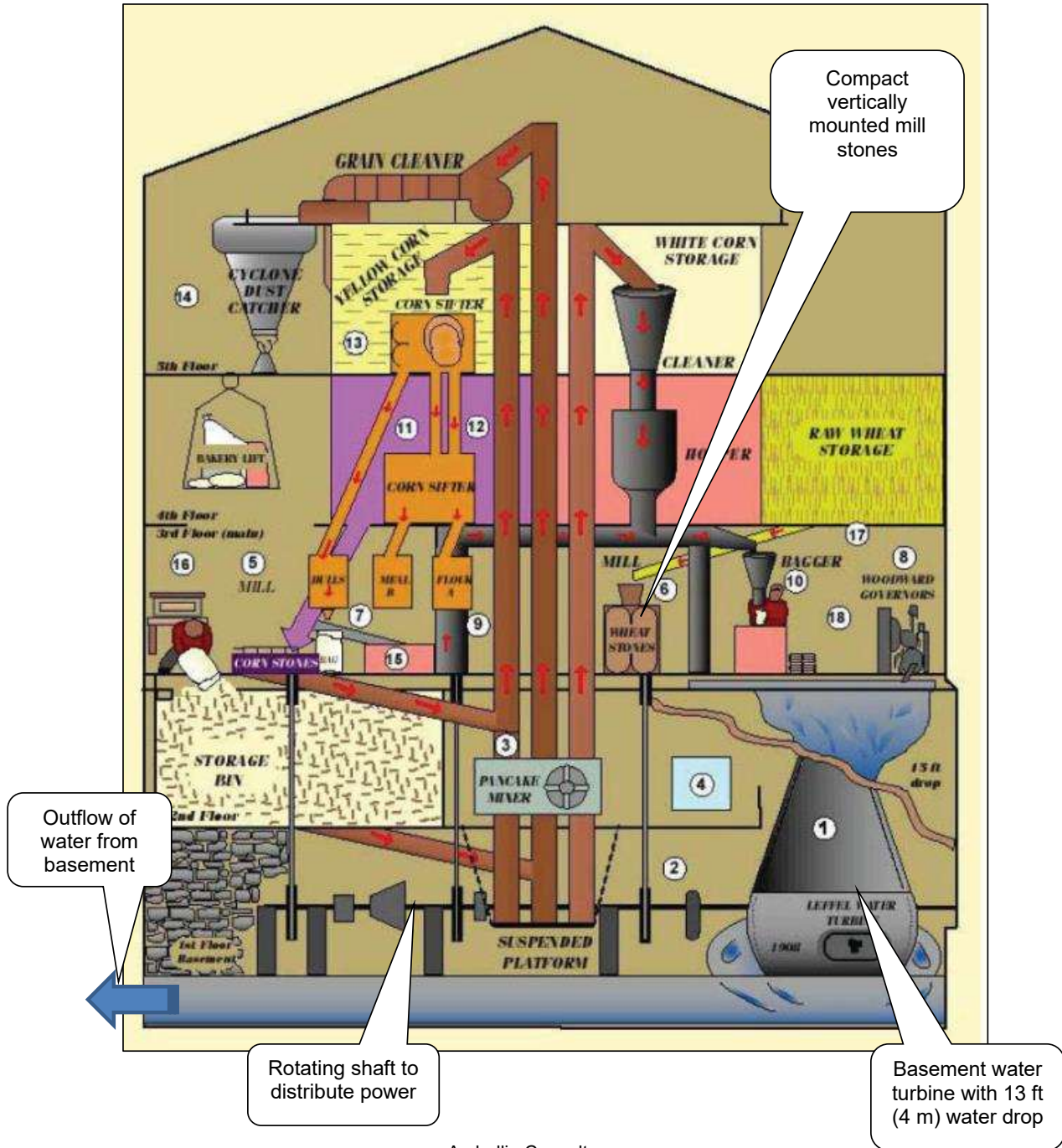


Figure 5.3
Example Layout of 1880s Corn Mill with Water Turbine in Basement

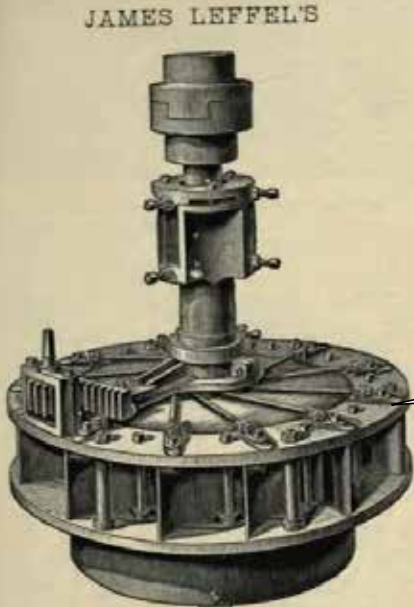


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Figure 5.4
Details of 1880s Water Turbines and Compact Vertical Mill Stones

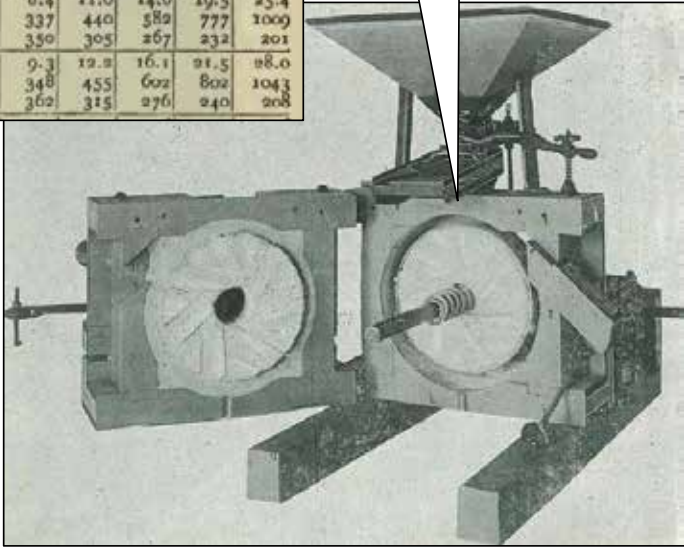
Table Specially Arranged, Expressly for James Leffel's Improved Double Turbine.

W.T.A.	SIZE OF WHEELS.	6 1/4	7 1/4	8 1/4	10	11 1/4	13 1/4	15 1/4	17 1/4	20	23	26 1/4
3	Horse Power...	.15	.20	.25	.33	.44	.58	.70	.87	1.1	1.7	2.2
	Cubic Feet....	99	130	160	210	280	360	440	540	660	1000	1280
	Revolutions....	360	480	600	800	1080	1440	1800	2160	2640	4000	5120
4	Horse Power...	.22	.29	.36	.48	.64	.81	.99	1.2	1.5	2.2	2.8
	Cubic Feet....	133	177	220	290	380	480	590	720	880	1300	1660
	Revolutions....	416	548	680	900	1180	1480	1800	2160	2640	4000	5120
5	Horse Power...	.31	.41	.51	.67	.89	1.1	1.3	1.6	2.0	3.0	3.8
	Cubic Feet....	191	254	317	410	540	680	840	1020	1240	1800	2280
	Revolutions....	465	616	770	1000	1320	1680	2040	2400	2880	4320	5440
6	Horse Power...	.41	.54	.67	.89	1.1	1.3	1.6	2.0	2.5	3.8	4.8
	Cubic Feet....	254	337	420	540	700	880	1080	1300	1600	2280	2880
	Revolutions....	510	670	840	1100	1440	1800	2160	2640	3200	4800	6080
7	Horse Power...	.52	.69	.86	1.1	1.3	1.6	2.0	2.5	3.0	4.5	5.8
	Cubic Feet....	320	420	520	670	880	1100	1360	1660	2000	2880	3680
	Revolutions....	551	720	900	1180	1560	1960	2360	2800	3360	4800	6080
8	Horse Power...	.63	.84	1.0	1.3	1.6	2.0	2.5	3.0	3.8	5.5	7.0
	Cubic Feet....	370	490	610	790	1040	1300	1600	1960	2360	3360	4240
	Revolutions....	588	770	960	1260	1680	2100	2520	3000	3600	5040	6320
9	Horse Power...	.76	1.0	1.2	1.6	2.0	2.5	3.0	3.8	4.8	7.0	8.8
	Cubic Feet....	460	600	740	960	1260	1600	2000	2400	2880	4000	5040
	Revolutions....	624	816	1000	1300	1720	2160	2600	3120	3720	5120	6320
10	Horse Power...	.89	1.2	1.5	1.9	2.4	3.0	3.8	4.8	6.0	8.8	11.0
	Cubic Feet....	530	690	860	1100	1440	1800	2200	2640	3200	4560	5760
	Revolutions....	658	870	1080	1400	1840	2300	2800	3360	4000	5440	6720
11	Horse Power...	1.0	1.3	1.6	2.0	2.5	3.0	3.8	4.8	6.0	8.8	11.0
	Cubic Feet....	590	770	960	1240	1600	2000	2400	2880	3520	4960	6160
	Revolutions....	690	910	1120	1440	1880	2360	2880	3440	4160	5600	6880
12	Horse Power...	1.1	1.4	1.7	2.2	2.8	3.4	4.2	5.2	6.4	9.2	11.4
	Cubic Feet....	610	800	1000	1280	1680	2100	2560	3120	3760	5200	6400
	Revolutions....	721	950	1160	1500	1960	2440	2960	3560	4320	5840	7120
13	Horse Power...	1.3	1.6	2.0	2.5	3.0	3.8	4.8	5.8	7.2	10.0	12.4
	Cubic Feet....	660	870	1080	1380	1800	2240	2760	3360	4000	5440	6720
	Revolutions....	750	990	1200	1540	2000	2500	3040	3640	4320	5840	7120
14	Horse Power...	1.5	1.9	2.3	2.9	3.6	4.4	5.4	6.6	8.0	11.2	13.8
	Cubic Feet....	730	960	1180	1500	1940	2400	2960	3600	4320	5840	7120
	Revolutions....	779	1020	1240	1600	2080	2580	3120	3760	4480	6000	7280
15	Horse Power...	1.6	2.0	2.5	3.1	3.8	4.6	5.6	6.8	8.2	11.4	14.0
	Cubic Feet....	760	1000	1220	1560	2000	2480	3000	3640	4360	5880	7160
	Revolutions....	806	1060	1280	1640	2120	2620	3160	3800	4520	6040	7320
16	Horse Power...	1.7	2.1	2.6	3.2	4.0	4.8	5.8	7.0	8.4	11.6	14.2
	Cubic Feet....	790	1040	1260	1620	2080	2560	3080	3720	4440	5960	7240
	Revolutions....	832	1090	1320	1680	2160	2660	3200	3840	4560	6080	7360



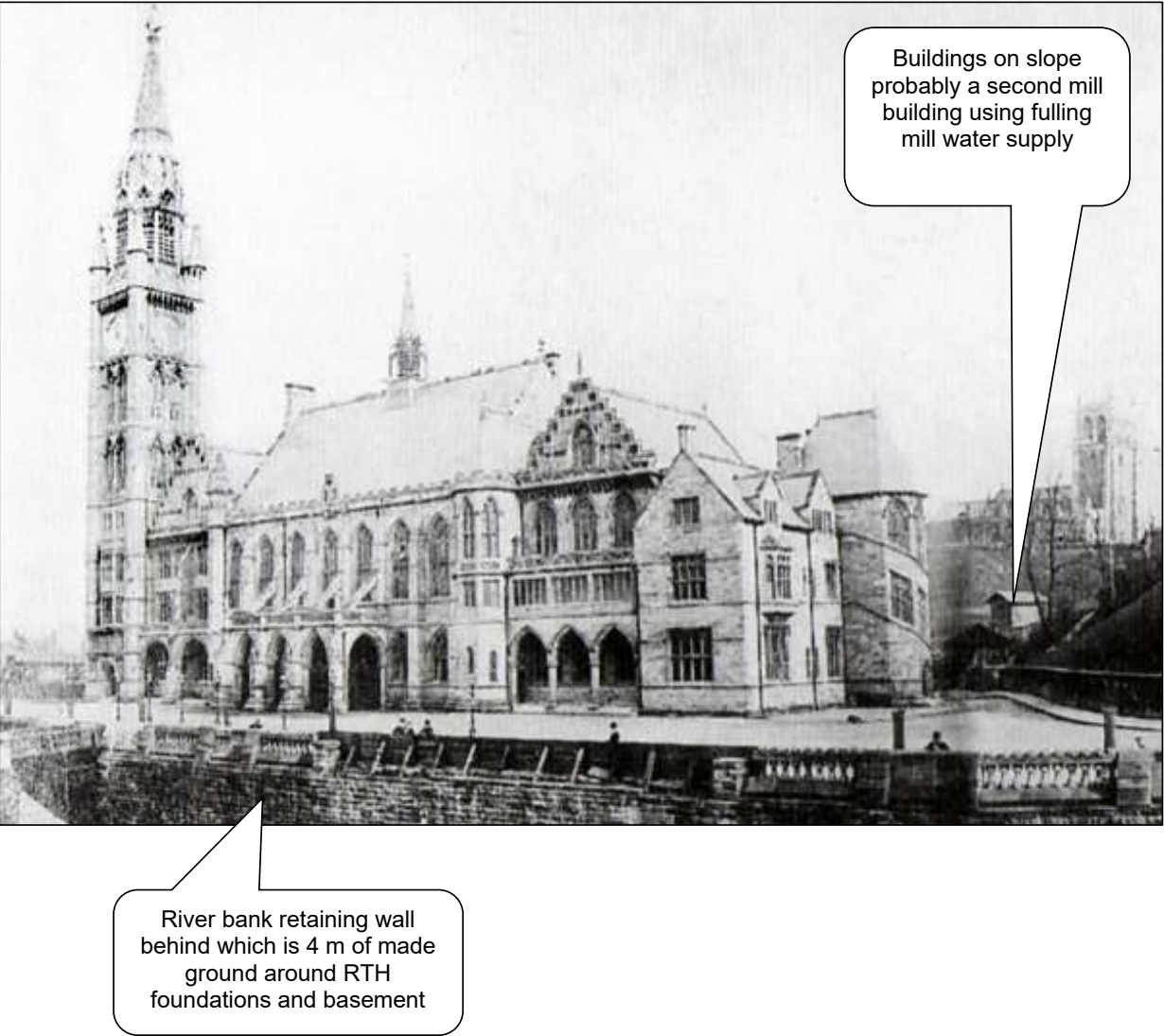
Example 1880s
mill water
turbine

Example 1880s
compact vertical
mill stones



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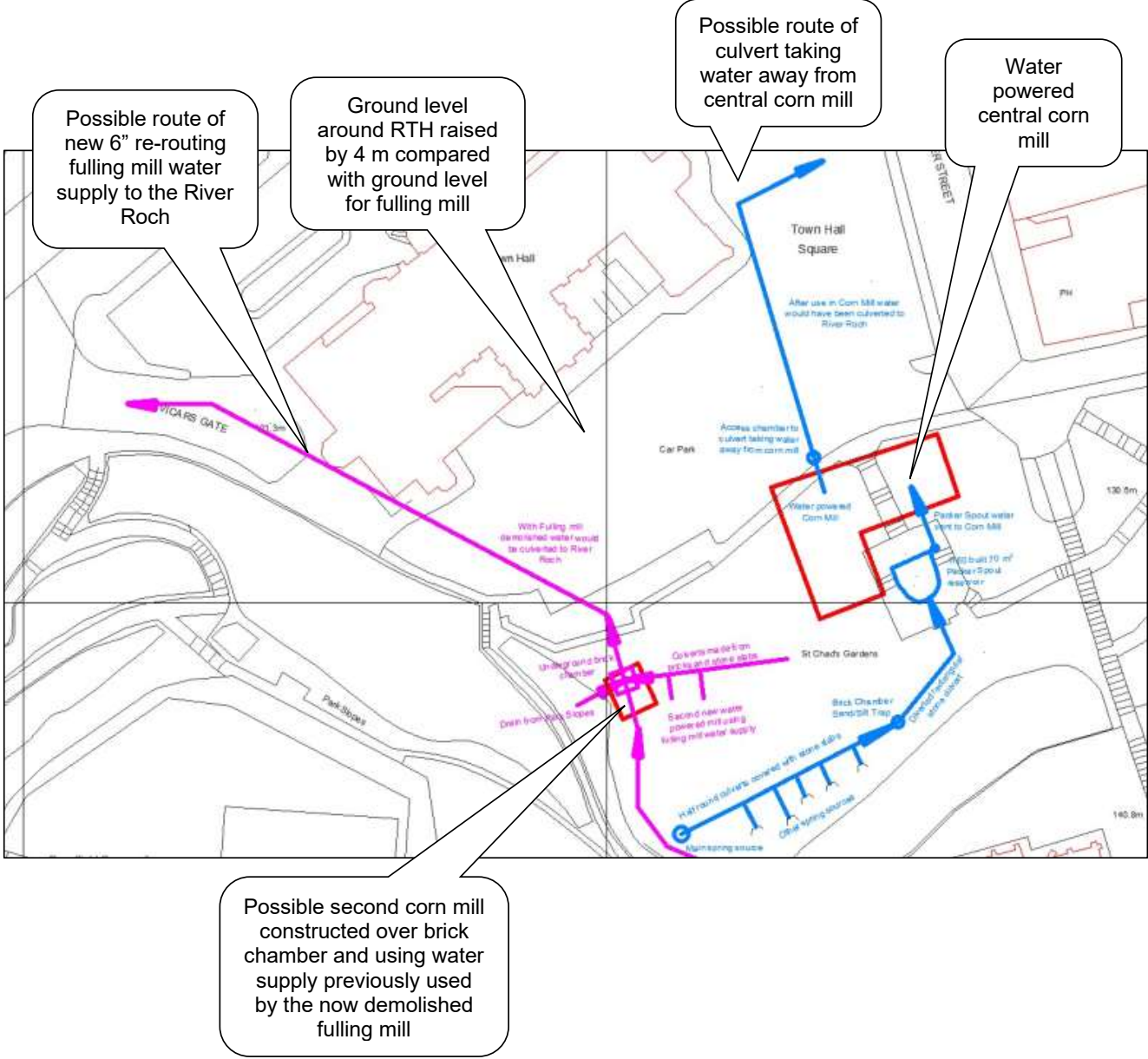
Figure 5.5
1874 Photo of Rochdale Town Hall Showing Possible Second Mill
(3 Years after Opening in 1871)



River bank retaining wall behind which is 4 m of made ground around RTH foundations and basement

Buildings on slope probably a second mill building using fulling mill water supply

Fig. 5.6
Plan Showing Changes to the Two water Supply Systems Following Construction of
Rochdale Town Hall, the Central Corn Mill and Possible Second Mill



Possible route of new 6" re-routing fulling mill water supply to the River Roch

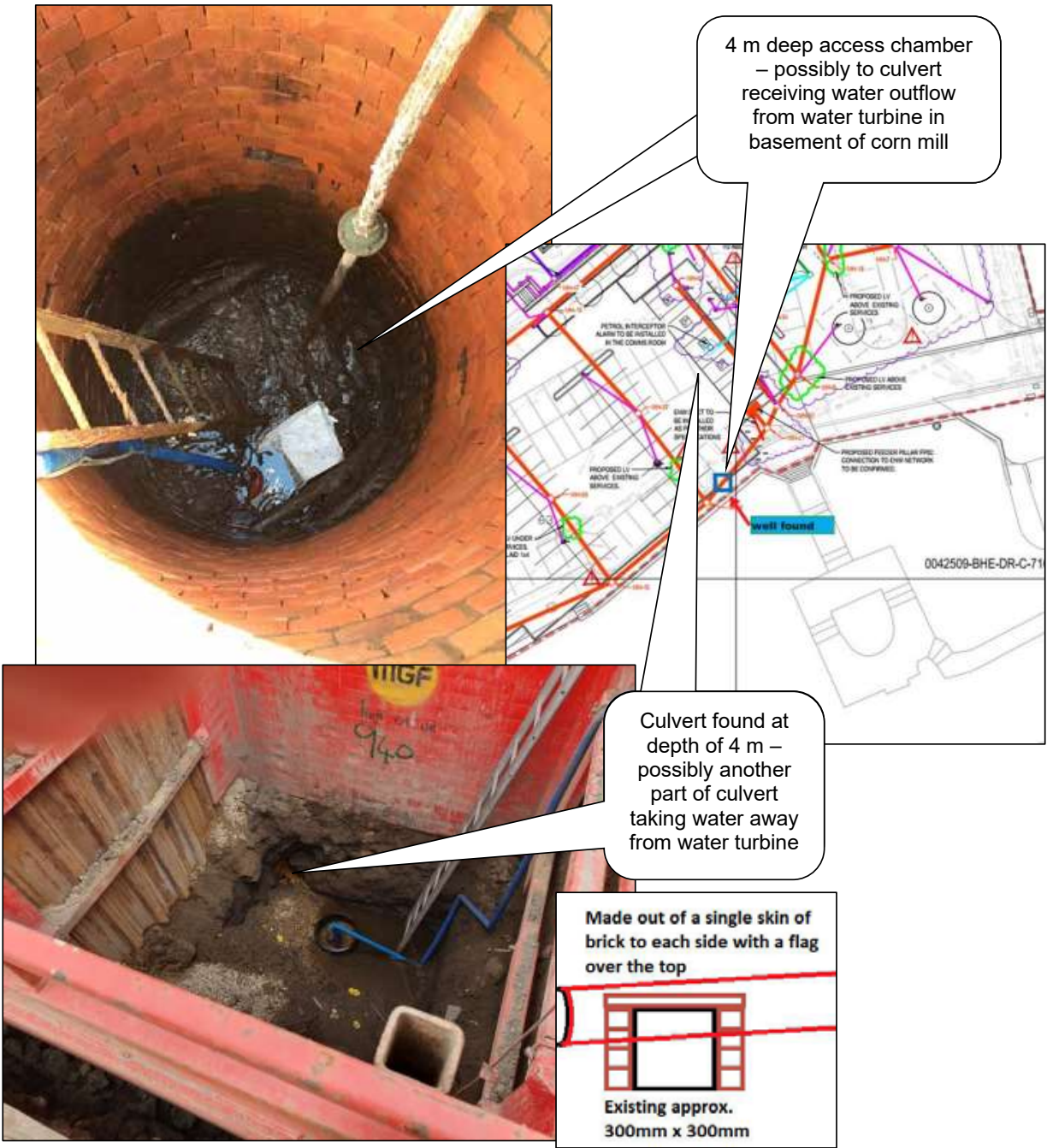
Ground level around RTH raised by 4 m compared with ground level for fulling mill

Possible route of culvert taking water away from central corn mill

Water powered central corn mill

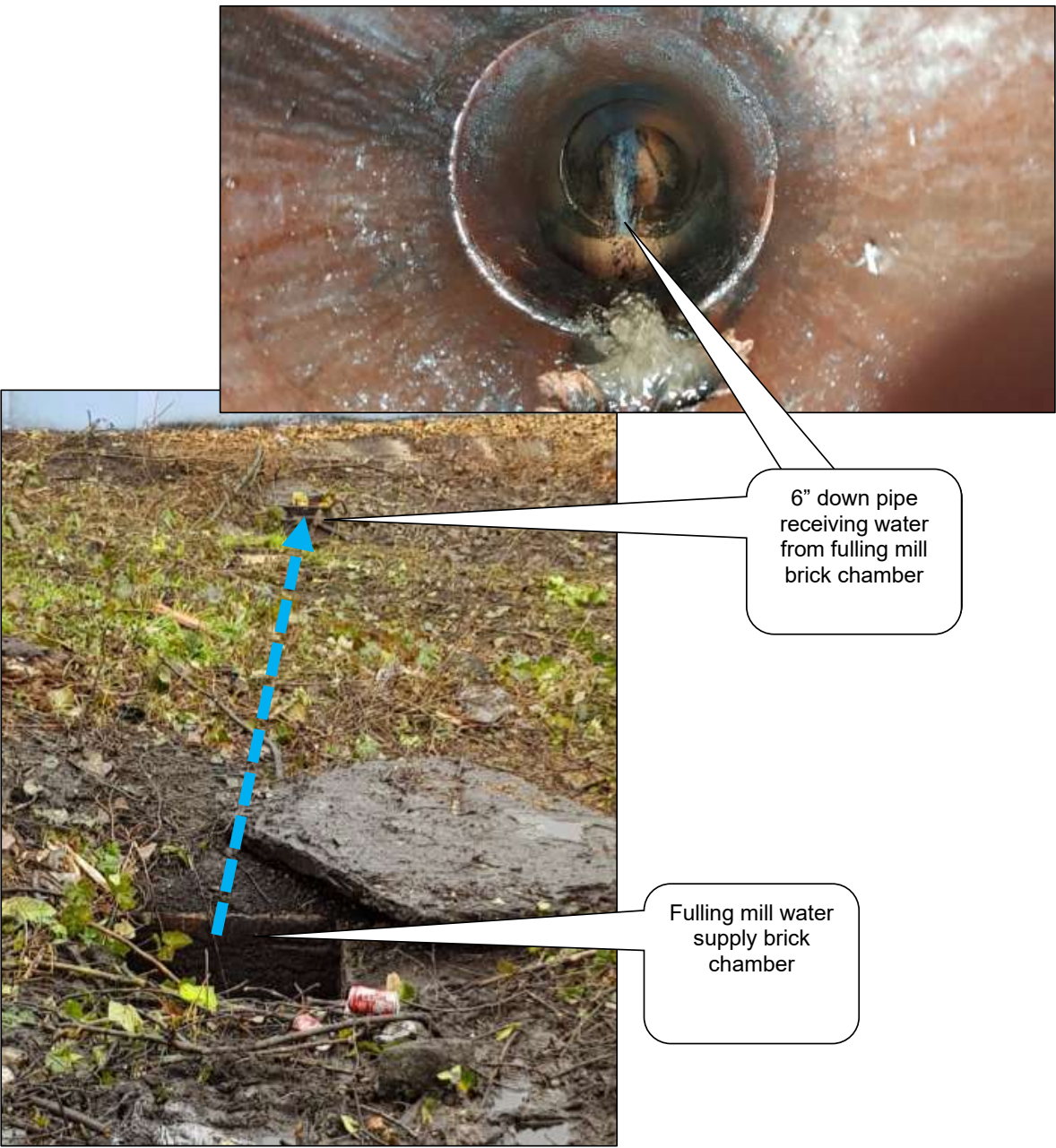
Possible second corn mill constructed over brick chamber and using water supply previously used by the now demolished fulling mill

Figure 5.7
Possible Access Chamber to Culvert Taking Water Away from Basement Level
Water Turbine in the Central Corn Mill



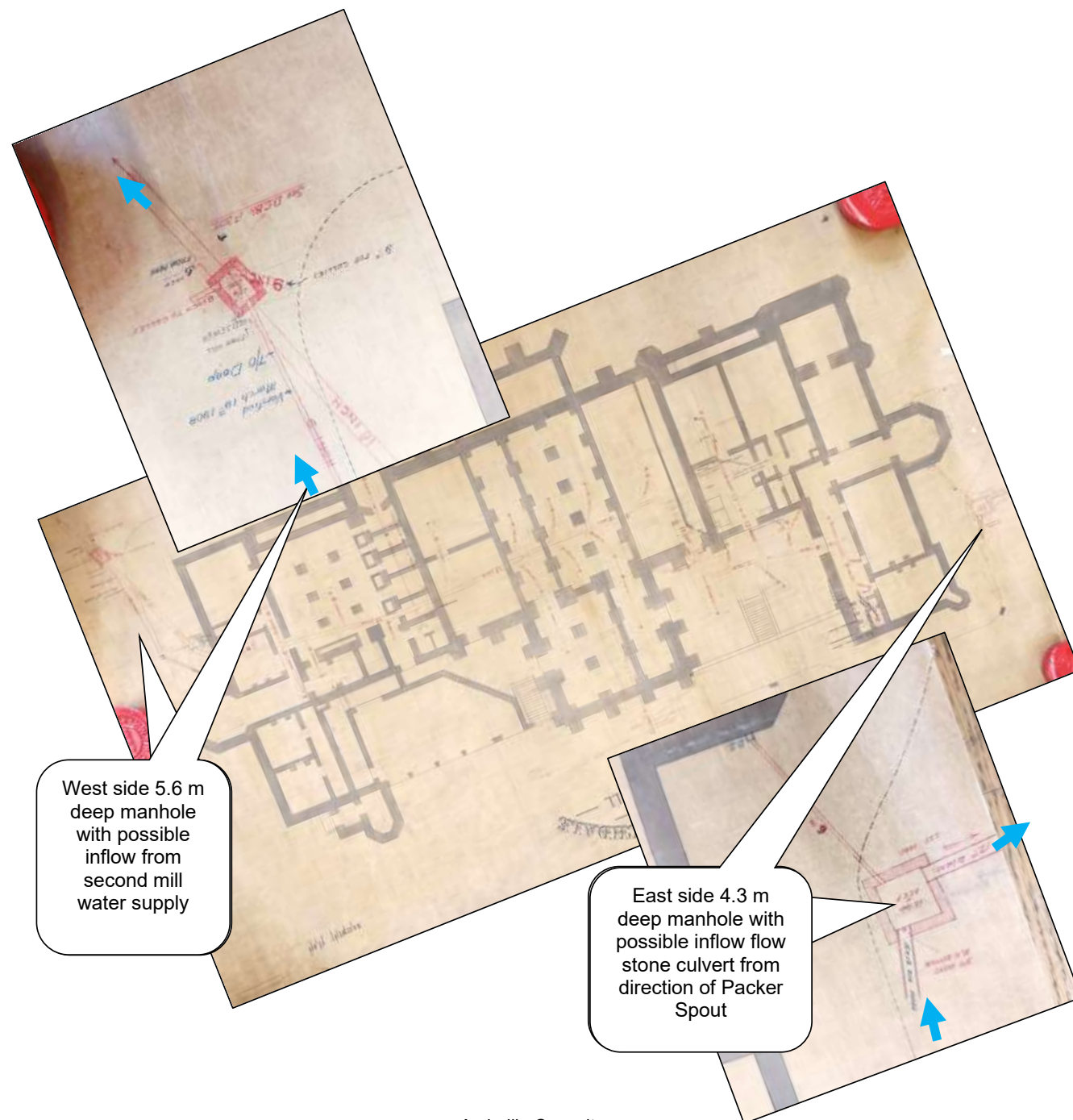
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Figure 5.8
New 62 Pipe Re-Routing Fulling Mill Water Supply to the River Roch



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Figure 5.9
1876 As Built RTH Drainage & Sewers Plan Showing Parts of Routes of Packer Spout and Old Fulling Mill Water Sources to the River Roch



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6.0 Changes to the Two Water Supply Systems Following the Demolition of the Central Corn Mill and Prior to Current RTH Restoration

When the Central Corn Mill (and other smaller mill building) were demolished in 1934, there was no longer any industrial need for both the Packer Spout and fulling mill water supply systems. As a result, the slopes to the south of RTH were redeveloped as municipal gardens (Figs.6.1).

The changes to the two water supply systems are summarised in the plan in Fig. 6.2. Unfortunately, this redevelopment resulted in significant damage to the slopes drainage.

With respect to the Packer Spout water supply system, at the main spring source the catchpit was removed and a pond created. The stone slab covers were also removed from the half-round culverts (Fig. 6.3). This allowed sand and plant debris to enter the culverts resulting in blockages and overflowing of the culverts. Packer Spout spring water supply drains appear on post 1930's OS maps because they are now visible due to the removal of the stone slab covers

Other spring catchpits were broken or covered with soil and the half-round culverts from those catchpits had the cover slabs removed, black plastic perforated pipes installed and were covered with soil, all resulting in impeded drainage (Fig. 6.4).

Water flow was diverted away from, and to the east of the Packer Spout reservoir (via a concrete ring chamber silt trap) enroute to cascades using 10 cm diameter plastic pipe which is easily blocked (Figs. 6.5, 6.6 & 6.7). The drain holes at base of upper cascade take water to Packer Street Drains, however dye tracing has indicated that the water is also leaking to old culverts near RTH basement.

Concerning the fulling mill water supply, the stone culvert leading from St Chad's Church to the underground brick chamber was covered with soil and blocked, resulting in the runoff water entering the Packer Spout culverts and not reaching the underground brick chamber (Fig. 6.8).

The downslope and lateral culverts collecting water from the lower spring line and channelling it to the underground brick chamber were also covered with soil and consequently blocked, resulting in permanently boggy ground along (Fig. 6.9).

To compensate for the impeded drainage, a field drain was installed at the base of the slope to the west of the Packer Spout reservoir to collect runoff and send it to the top of down pipe which receives water From underground brick chamber (Fig. 6.10).

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Fig. 6.1
Photos of Conversion of Slopes to Landscaped Gardens after Demolition of Mill
Buildings

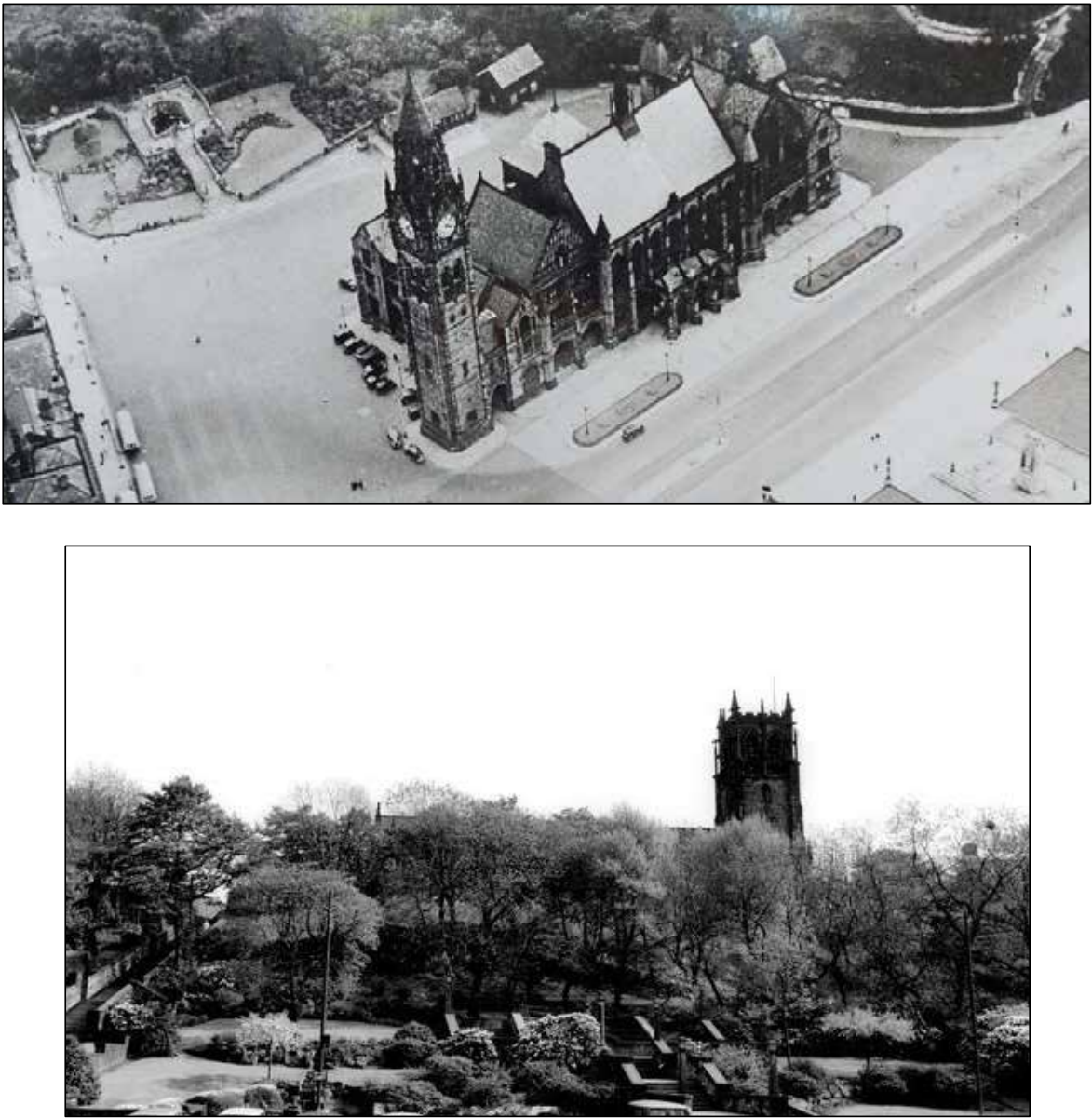
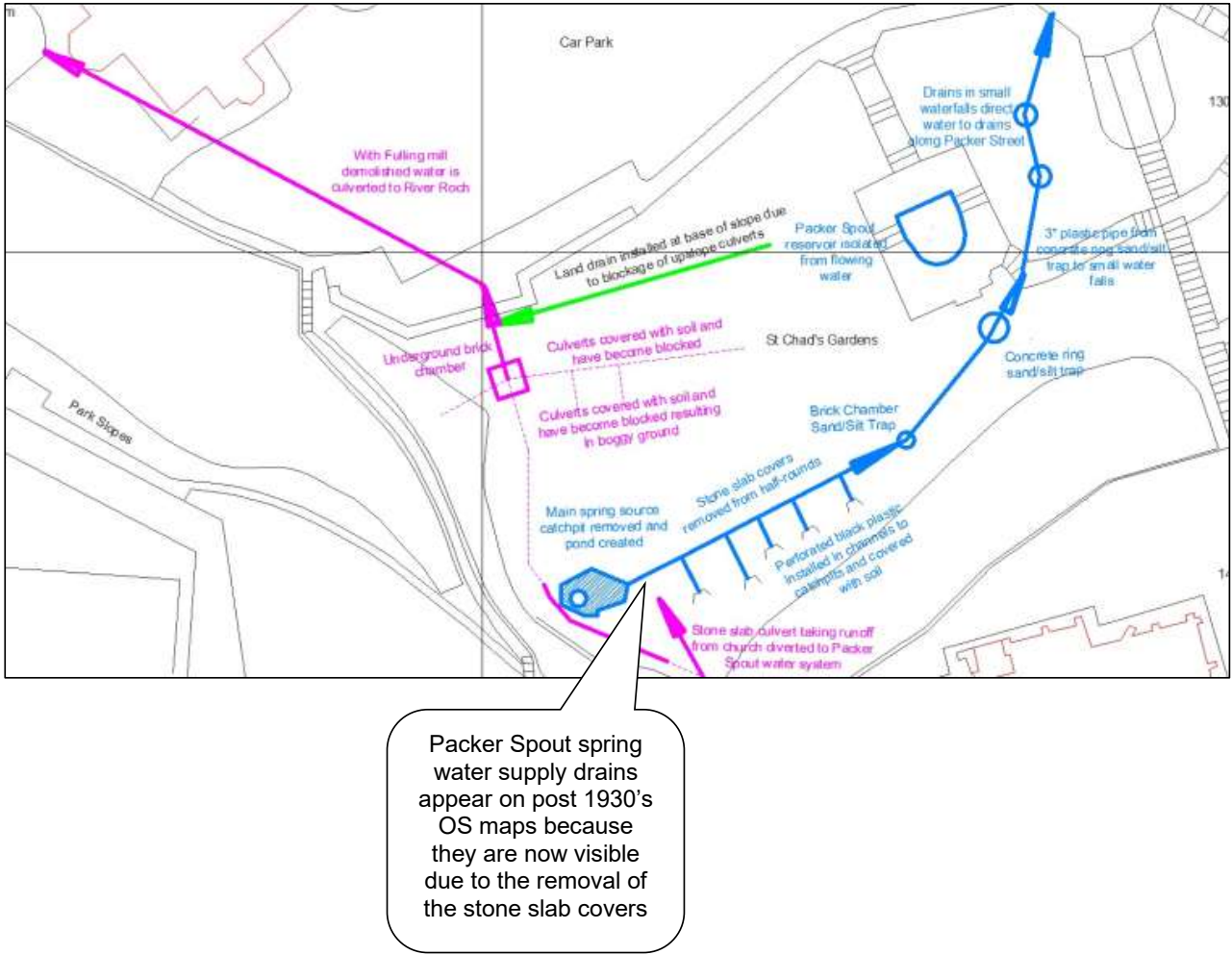


Figure 6.2
Summary of Changes to Slopes Water Supply Systems and Drainage Following
Demolition of Central Corn Mill in 1934



Packer Spout spring
water supply drains
appear on post 1930's
OS maps because
they are now visible
due to the removal of
the stone slab covers

Fig. 6.3
At Main Spring Source for Packer Spout the Spring Catchpit was Removed
and Pond Created - Stone Slab Covers Removed from Half-Round Culverts



Figure 6.4
Packer Spout Top Spring Line Catchpits Broken or Covered With Soil –
Half-Round Culverts from Catchpits Have Cover Slabs Removed, Black Plastic
Perforated Pipes installed and Are Covered with Soil Resulting In Impeded Drainage



Fig. 6.5
Packer Spout Reservoir No Longer Has Water Flowing Into It – Spring Water
Diverted to the East to the Water Falls/Cascades

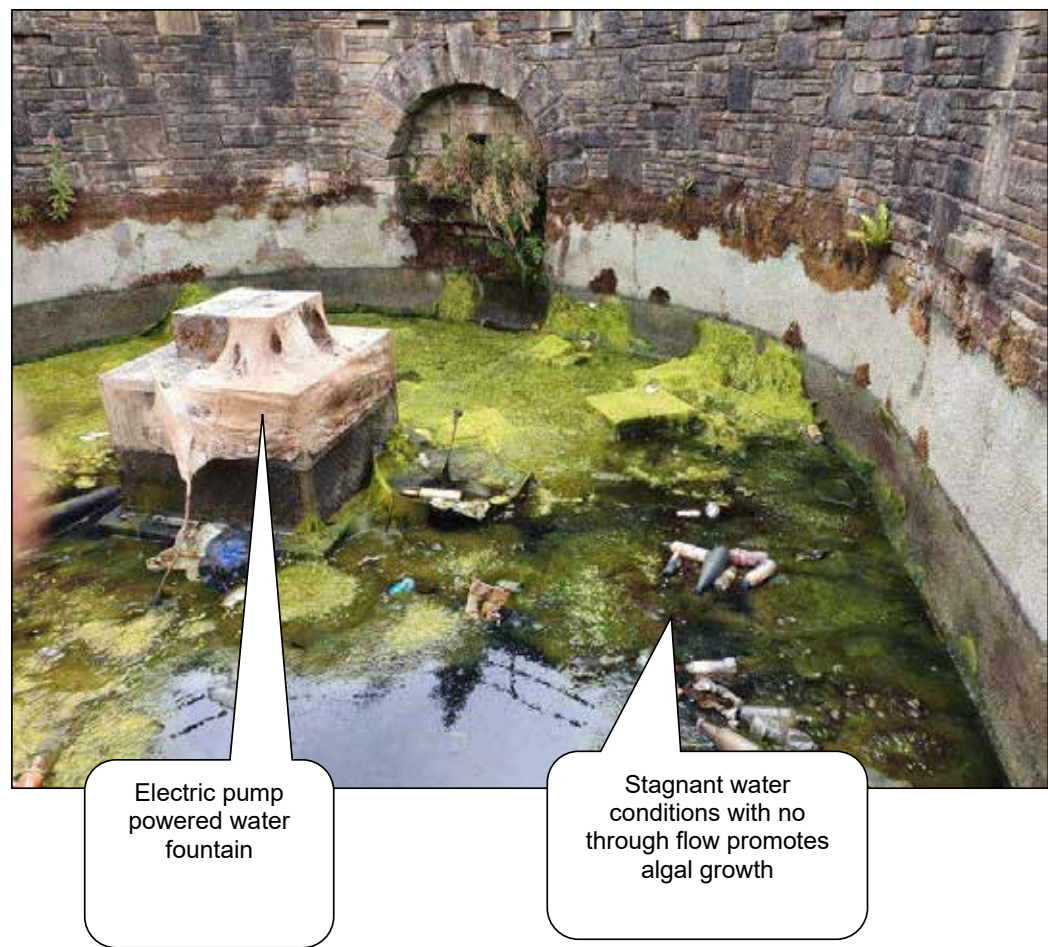
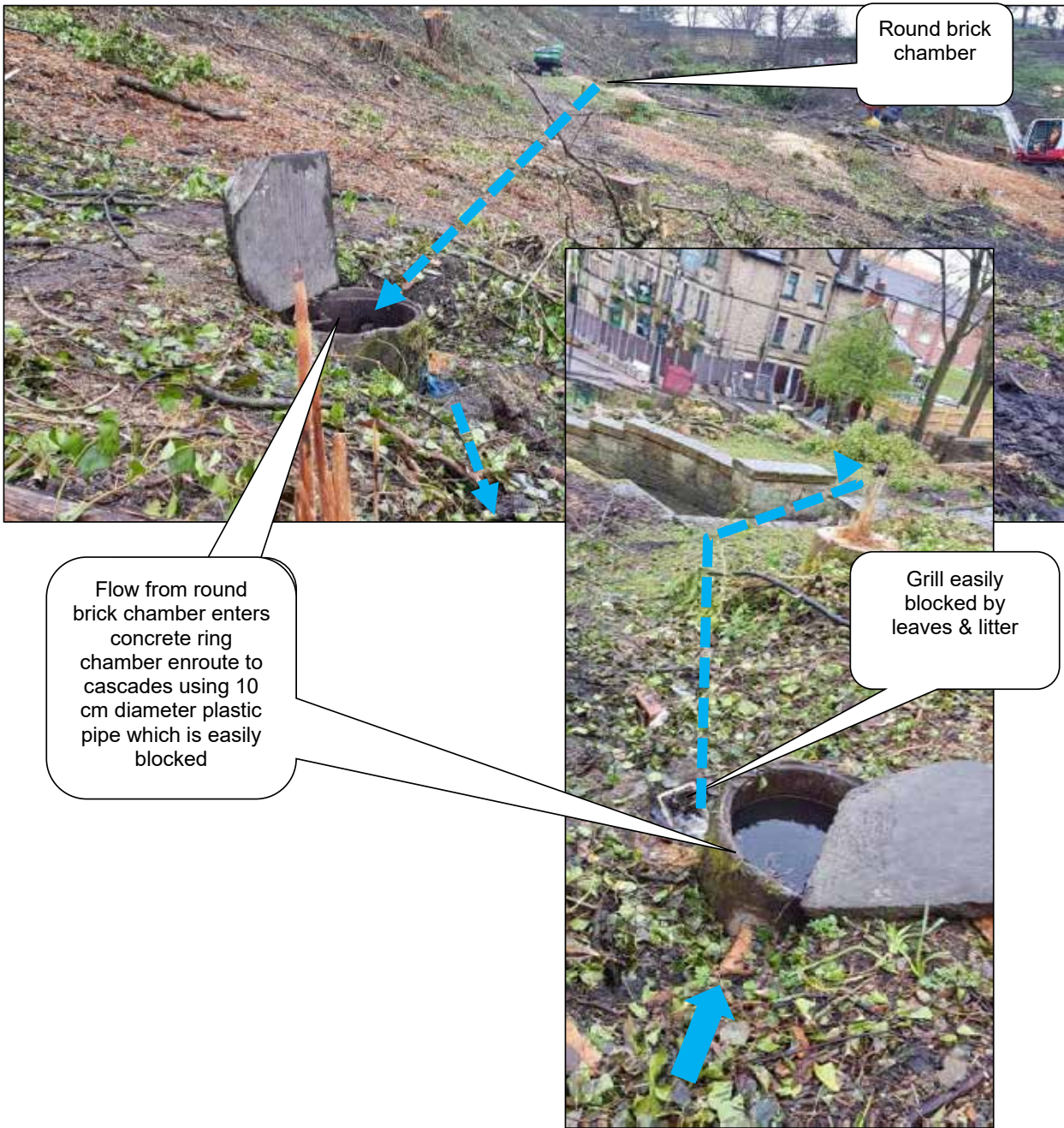
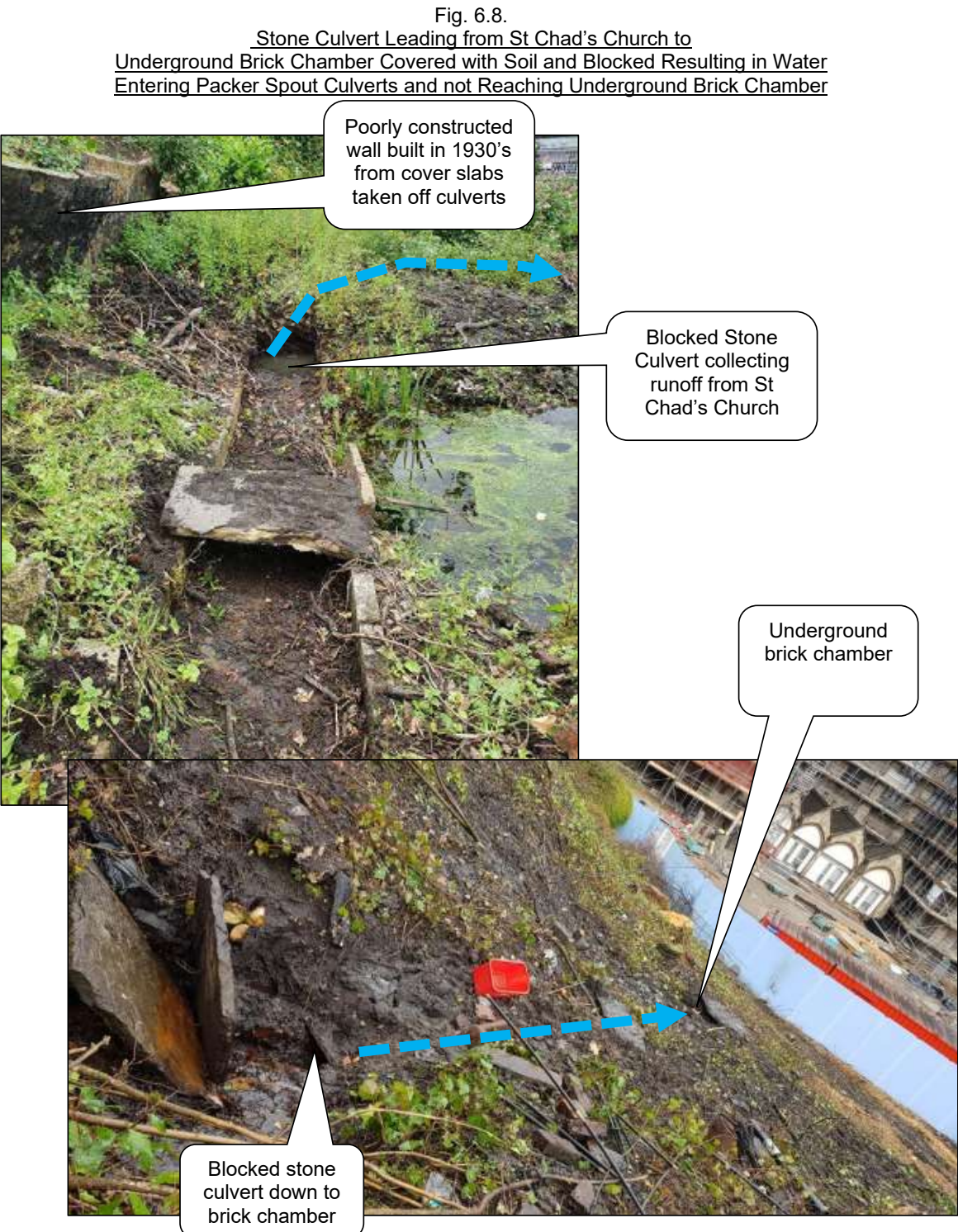


Fig. 6.6
Water Flow Diverted to East of Packer Spout Reservoir
Flow from Round Brick Chamber enters Concrete Ring Chamber Silt Trap Enroute to
Cascades Using 10 cm Diameter Plastic Pipe Which is Easily Blocked



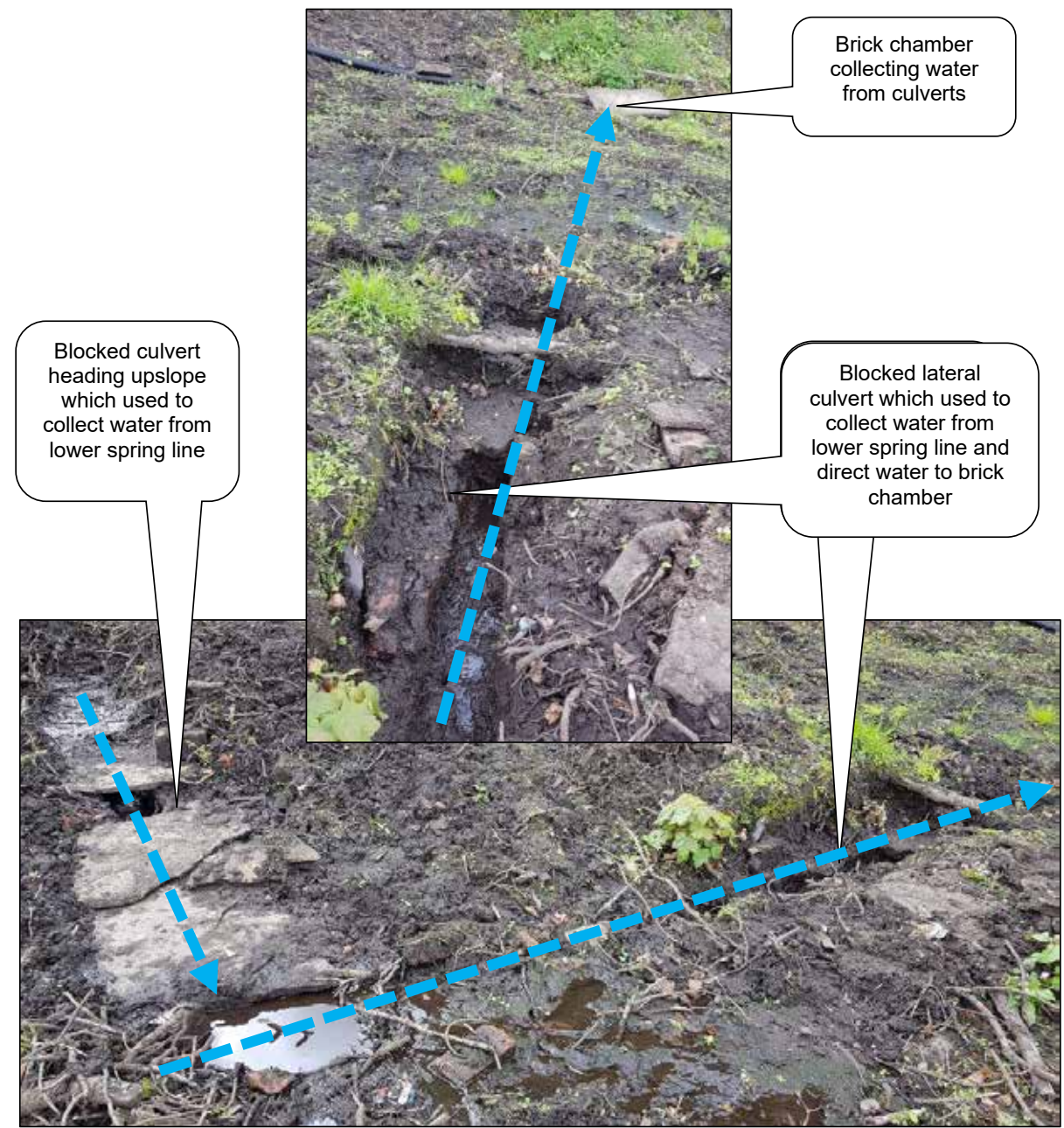


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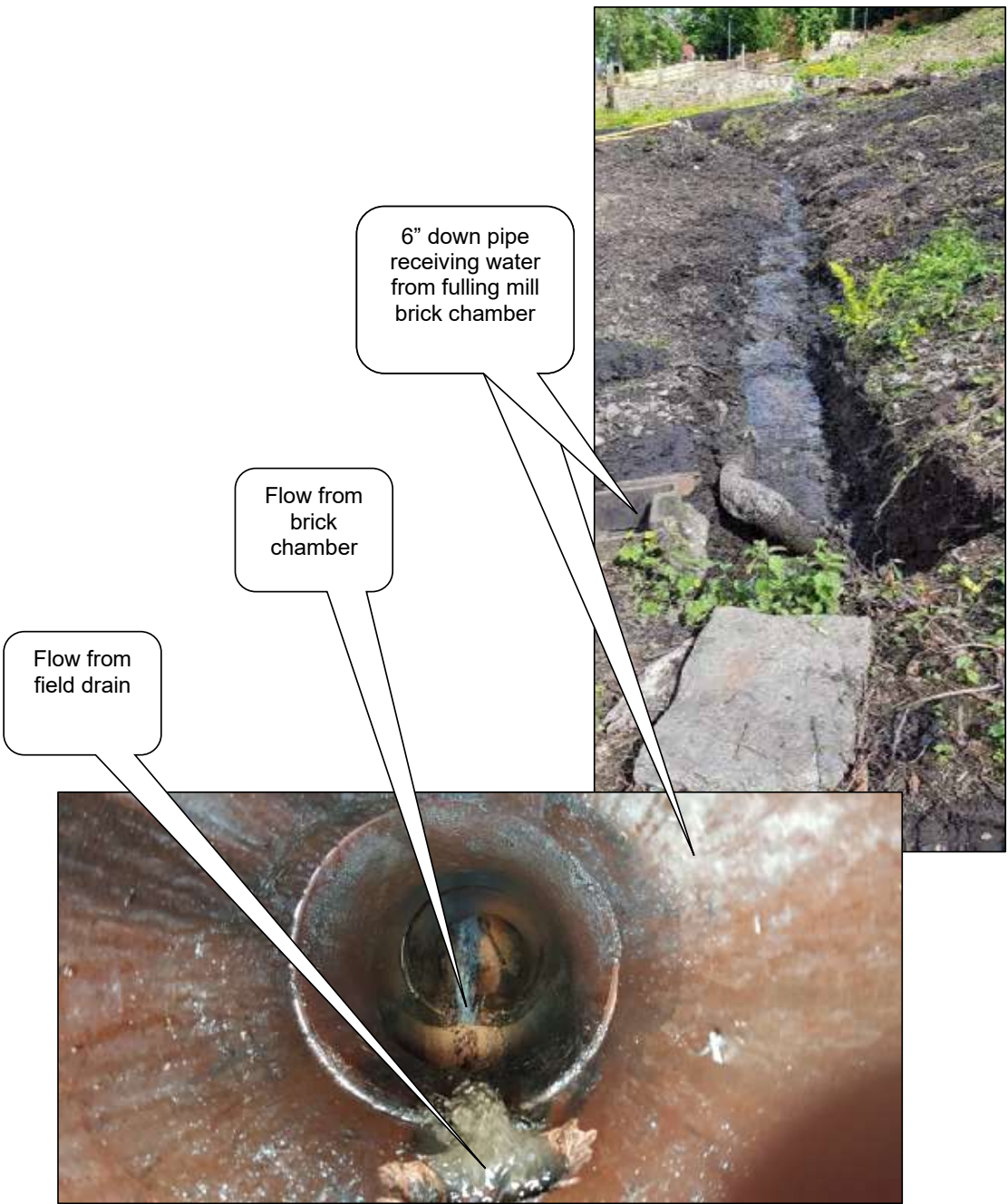
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Fig. 6.9.
Culverts Collecting Water from Lower Spring Line and Channelling it to Underground
Brick Chamber Covered with Soil and Blocked



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Fig. 6.10
Field Drain Installed at Base of Slope to West of Packer Spout Reservoir to Collect
Runoff – Field Drain Enters top of Down Pipe which Receives Water From
Underground Brick Chamber



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7.0 Impact of March/April 2022 Tree Clearance on Slope Drainage

As part of the RTH Slopes Project, trees clearance was undertaken in March and April 2022.

The slopes tree clearance caused some unavoidable problems with the existing slopes drainage, primarily due to the exposure of the unvegetated slopes resulting in the mobilisation of silt.

The trench of gravel of the field drain at the base of the slope became blocked with silt resulting in water flowing over the top of the field drain and over the retaining wall (Fig. 7.1).

The gravel of the field drain became rapidly blocked with silt due to the absence of a geotextile membrane above the top of the gravel layer which would have prevented the silt ingress. As a result of the blockage of the field drain, no water was flowing along the black perforated pipe within the field drain (Fig. 7.1).

Yellow perforated filed drain pipes leading down to the base of slope field drain were also broken during the slopes clearance which would have resulted in more drainage problems and more silt laden surface runoff blocking the field drain at the base of the slope (Fig. 7.2).

Silt and shredded wood also accumulated in the half-round surface drainage pipes at the top of the slopes, causing blockage of the buried pipes leading to the water cascades, which resulted in surface flow towards the RTH (Fig. 7.3).

To prevent the surface runoff reaching RTH and causing problems with work on the area surrounding RTH, remedial action had to be taken on the slopes drainage in April 2022 as described in Section 8.

Fig. 7.1
Blocked Field Drain at Base of Slope –
(Due to Silt Generated by Slope Clearance & Absence of Geotextile Membrane at
Top of Field Drain)

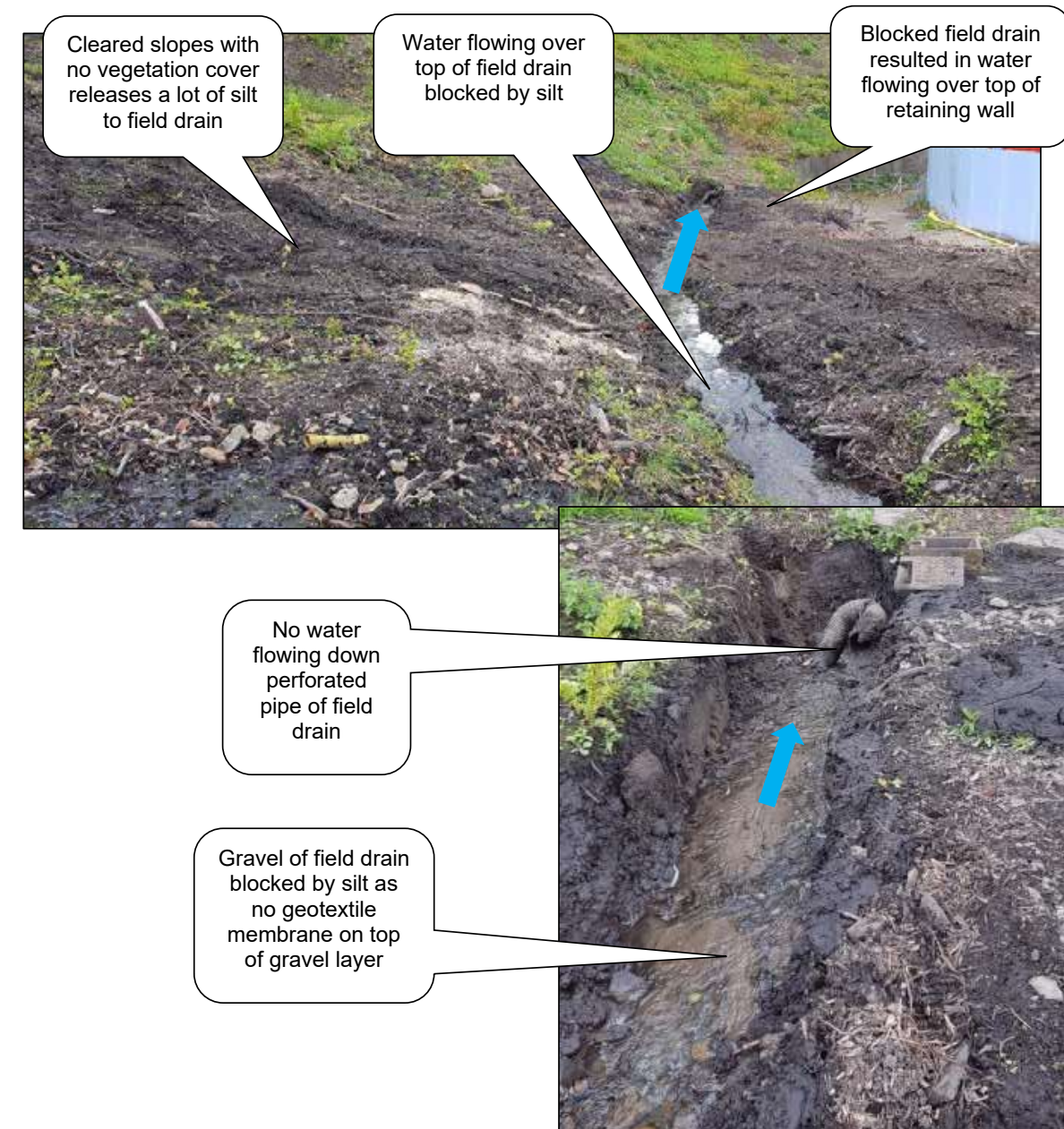
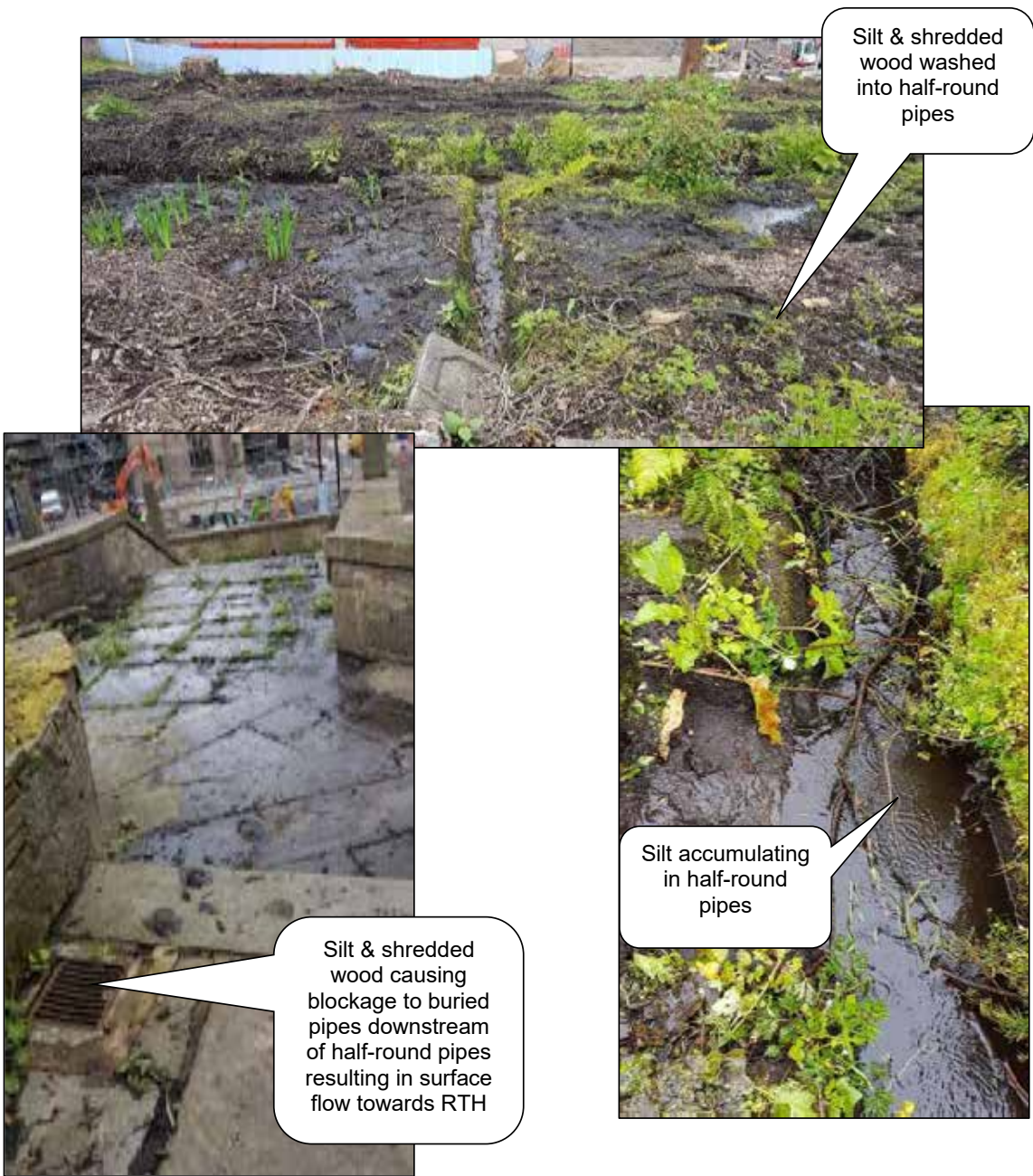


Fig. 7.2
Broken Yellow Field Drain Pipes above Field Drain at Base of Slope



Fig. 7.3
Silt and Shredded Wood Washed into Top Slope Half Round Pipes Causing Blockage of Downstream Buried Pipes Resulting in Surface Flow Towards RTH



8.0 Remedial Action on Slope Drainage to Prevent Surface Runoff Reaching RTH

As discussed in Section 7, the March/April 2022 slopes tree clearance caused problems with the existing slopes drainage primarily due to the exposure of the unvegetated slopes resulting in the mobilisation of silt.

The remedial actions taken to help prevent runoff reaching the level ground to the south of RTH are listed below.

It is emphasised that these remedial actions are only intended to be short term, prior to the redesign of the slopes' drainage.

New drainage should be installed as soon as possible to control water flows and the accumulation of silt.

The outfall of the blocked field drain at the base of slope was excavated so that water can now flow directly into the drains connected to the new RTH drains (Fig. 8.1).

The trench connected to the field drain at the base of the slope was extended to the east to intercept surface runoff from higher up the slopes which may occur due to, for example, storms and blocked pipes leading to the cascades (Fig. 8.2).

The pond below the vicarage gardens was deepened and widened to retain more water and the pond outfall was directed (using a temporary surface black plastic pipe) to the brick chamber on the eastern side of the slopes, which is connected to the new RTH drainage, (Fig. 8.3).

The half-round pipes at the top of the slopes were dammed at their mid-point to send as much water as possible to the pond – thus avoiding blockage prone pipes leading to cascades (Fig. 8.4).

The water reaching the concrete ring silt trap above Packer Spout was diverted to the trench leading to the lower slope field drain, again to prevent water flowing down the blockage prone pipes leading to cascades (Fig. 8.5).

The photo in Fig. 8.6. presents a summary of the remedial temporary drainage changes implemented to try and prevent surface water flow to the RTH and to help start the drying out process of the slopes.

The requirement for these remedial drainage works restricted the time available for the original planned activities such as the hand-digging of the original spring water collection drainage systems.

Fig. 8.1
Outfall of Blocked Field Drain at Base of Slope Excavated so Water Can Flow Directly into Drains Connected to New RTH Drains – Avoiding Water Flowing Over Retaining Wall



Fig. 8.2
Trench Connected to Field Drain at Base of Slope Extended to Intercept Surface
Runoff from Higher Up the Slopes (Due to e.g., storms and blocked pipes)



Trench Connected to Field Drain at Base of Slope Extended to Intercept Surface Runoff from Higher Up the Slopes

Fig. 8.3
Pond Deepened and Widened to Retain More Water and Pond Outfall Directed to Chamber on Eastern Side of Slope Connected to New RTH Drainage



Pond Deepened and Widened to Retain More Water

Pond Outfall Directed to Chamber on Eastern Side of Slope Connected to New RTH Drainage using black plastic pipe

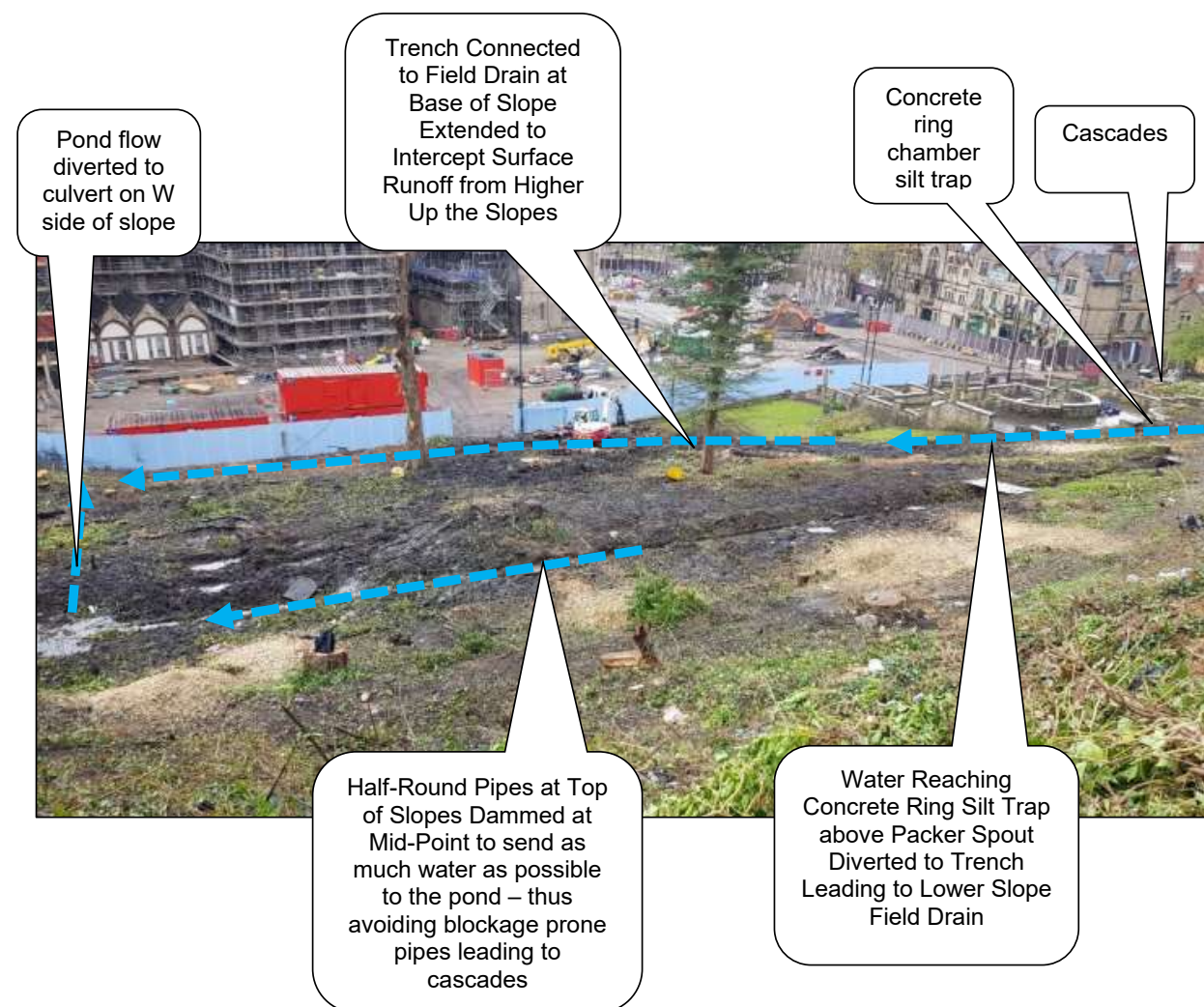
Fig. 8.4
Half-Round Pipes at Top of Slopes Dammed at Mid-Point



Fig. 8.5
Water Reaching Concrete Ring Silt Trap above Packer Spout Diverted to Trench Leading to Lower Slope Field Drain Slope



Fig. 8.6
Photo of Summary of Remedial Temporary Drainage Changes to Prevent Surface
Water Flow to the RTH



9.0 Hydrogeological Model for the Springs Water Flows for the Packer Spout and Fulling Mill Water Systems Flows

This section summaries the hydrogeological model developed for the superficial glaciofluvial sand and gravel aquifer which outcrops at the site and is the source of the spring water issuing from the slopes to the south of RTH. Also included are estimates of the main Packer Spout spring flow rate during June & July 2022 based on data logger water level recordings.

The surface superficial geology in the vicinity of the site is illustrated in Fig 9.1. The key to the colour coding of the superficial geology is shown in Fig. 9.2. A '3D' Representation of the Superficial Geology in the Vicinity of the Site is presented in Fig. 9.3. Superficial geology refers to the glacial and fluvio-glacial deposits left after the retreat of the glaciers and ice sheets associated with the last ice age, and landslip.

The site is on an extensive area of Devensian glaciofluvial sheet deposits which are comprised of sands & gravels interbedded with clay layers and were deposited by large rivers fed by meltwater from retreating glaciers (Fig. 9.4)

At the base of the sands and gravels is a layer of glacial till clay. Till is deposited directly beneath a glacier as it melts and is the material ground down by the abrasive movement of the glacier over the bedrock. As the glaciers retreated further the till deposits would have been eroded and sorted in size by the rivers of flowing meltwater which deposited the overlying glaciofluvial sand and gravels and interbedded layers of clay (Fig. 9.5).

A geological log for a borehole drilled adjacent to RTH shows the different layers of superficial deposits (Fig. 9.6). The Lidar DTM data contour map, with the two spring lines plotted is presented in Fig. 9.7, and was used to help plot a N-S geological section through the superficial Devensian glaciofluvial sand & gravel sheet aquifer (Fig. 9.8).

The glaciofluvial sands and gravels are shown to have a thickness of about 30 m underneath the Broadfield Park plateau, which is at an elevation of about 142 m AOD. At the base of the slope, at 121 m AOD, the thickness of the sands and gravels is about 4 m underneath about 4 m of made ground around the foundations and basement of the RTH. The 4 m of made ground indicates that the ground level at the RTH before its construction was at about 177 m AOD. The clayey soil at that original ground level still appears to be in place and probably represent deposits on the floodplain prior to the building of RTH.

Spring lines emerge at elevations where a high permeability sand and gravel is underlain by relatively thin clayey layers. The two spring lines are estimated to be at 131 m AOD (for the Packer Spout water supply) and at 127 m AOD (for the Fulling Mill water supply) (Fig. 9.8).

The Devensian glaciofluvial sand & gravel sheet deposits at the site (which have a thickness of about 30 m below the Broadfield Park plateau) are classified as a Secondary A aquifer (Figs. 9.9 & 9.10).

Secondary A aquifers have permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers.

Data logger water level measurements indicated that the main spring source for the Packer Spout water supply had a flow rate of between 1 l/s and 8 l/s (Fig. 9.11).

This flow rate is in general agreement with the ReFH2 estimated of base flow for a 10.24 ha catchment area of the Broadfield Park plateau to the south of RTH (Fig. 9.12).

Figure 9.1
Superficial Geology in the Vicinity of the Site

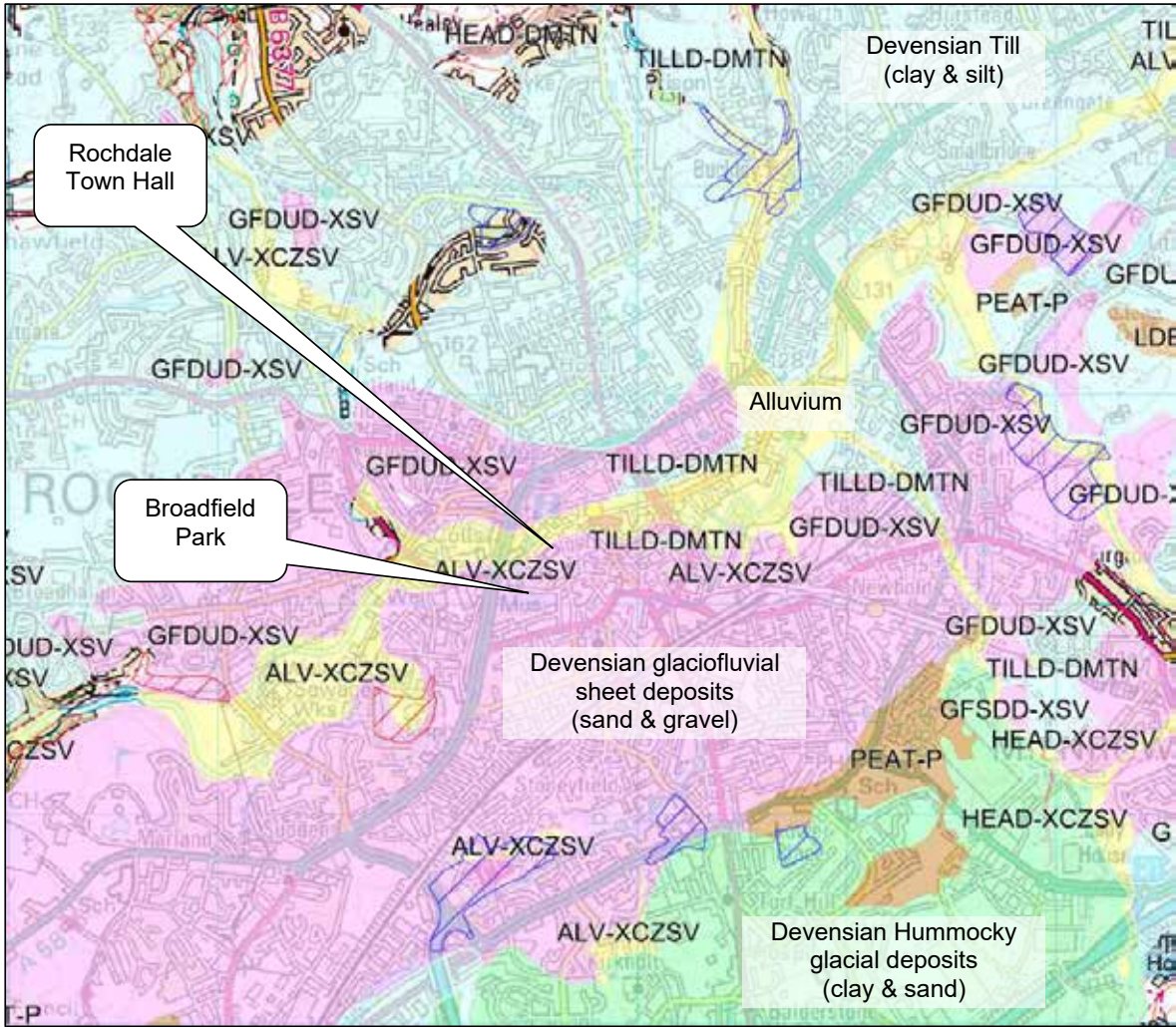


Figure 9.2
Key to Superficial Geology

KEY TO SUPERFICIAL GEOLOGY				
Map Colour	LEX Code	Rock Name	Rock Type	Max Age (Period)
	ALV-XCZSV	Alluvium	Clay, Silt, Sand And Gravel	Quaternary
	HMGDD-DMTN	Hummocky (Moundy) Glacial Deposits, Devensian	Diamicton	Quaternary
	TILLD-DMTN	Till, Devensian	Diamicton	Quaternary
	GFDUD-XSV	Glaciofluvial Deposits, Devensian	Sand And Gravel	Quaternary
	GFSDD-XSV	Glaciofluvial Sheet Deposits, Devensian	Sand And Gravel	Quaternary
	LDE-C	Lacustrine Deposits	Clay	Quaternary
	HEAD-XCZSV	Head	Clay, Silt, Sand And Gravel	Quaternary
	HEAD-DMTN	Head	Diamicton	Quaternary
	PEAT-P	Peat	Peat	Quaternary

Figure 9.3
'3D' Representation of the Superficial Geology in the Vicinity of the Site

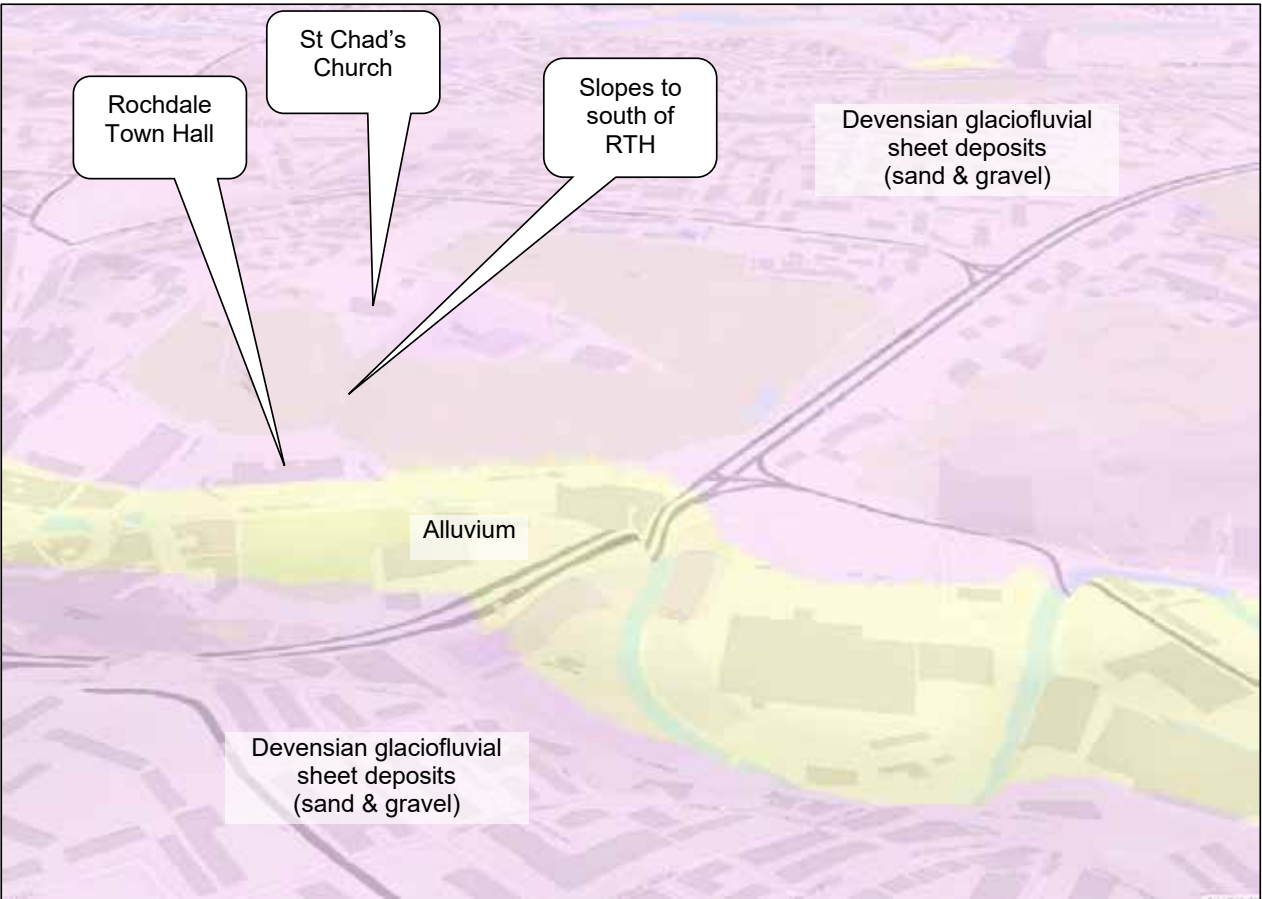


Figure 9.4
Schematic Sedimentological Log for Glacial Sediments below Broadfield Park

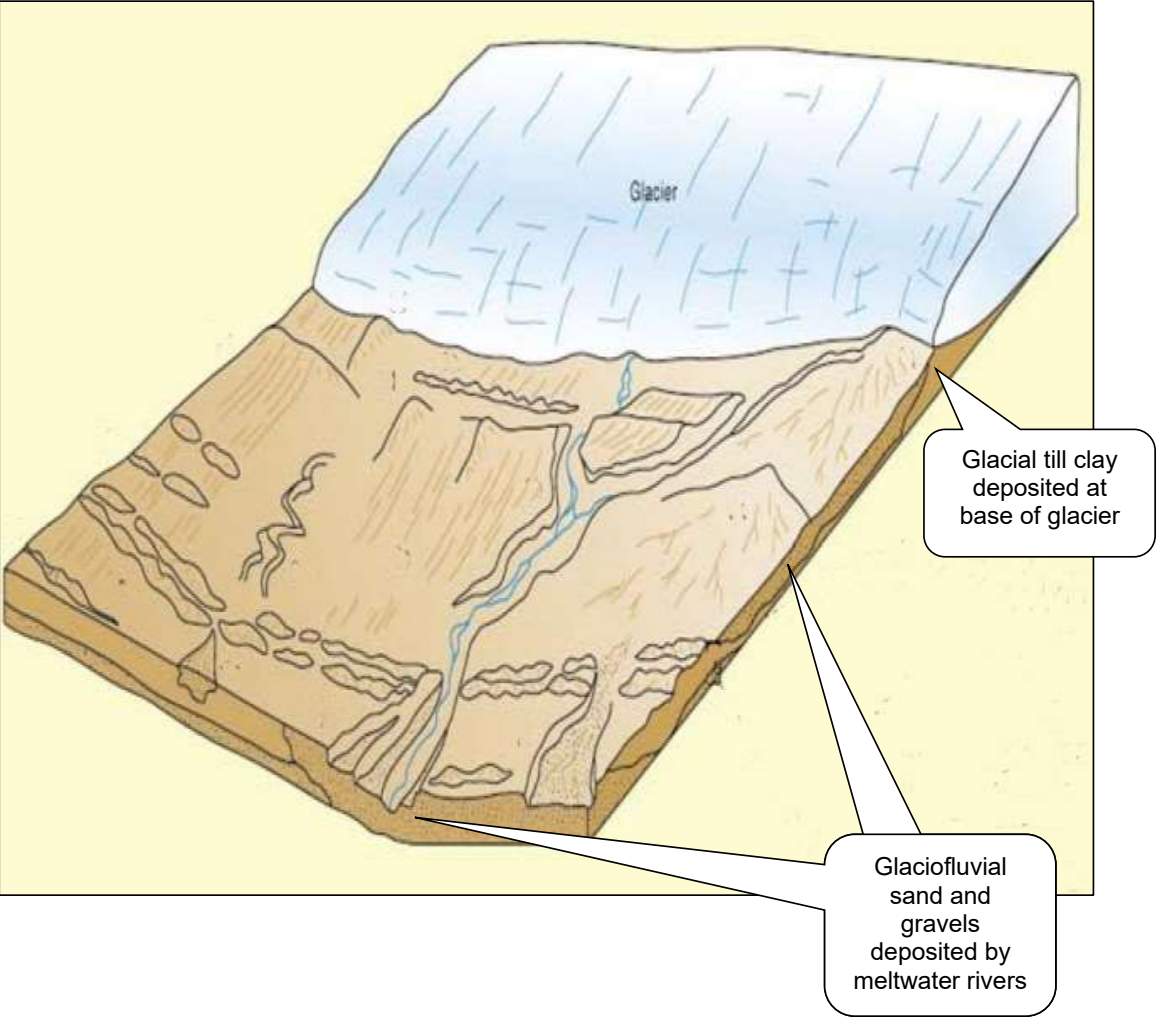


Figure 9.5
Example Image of Glaciofluvial Sands & Gravels Interbedded with Clayey Layers

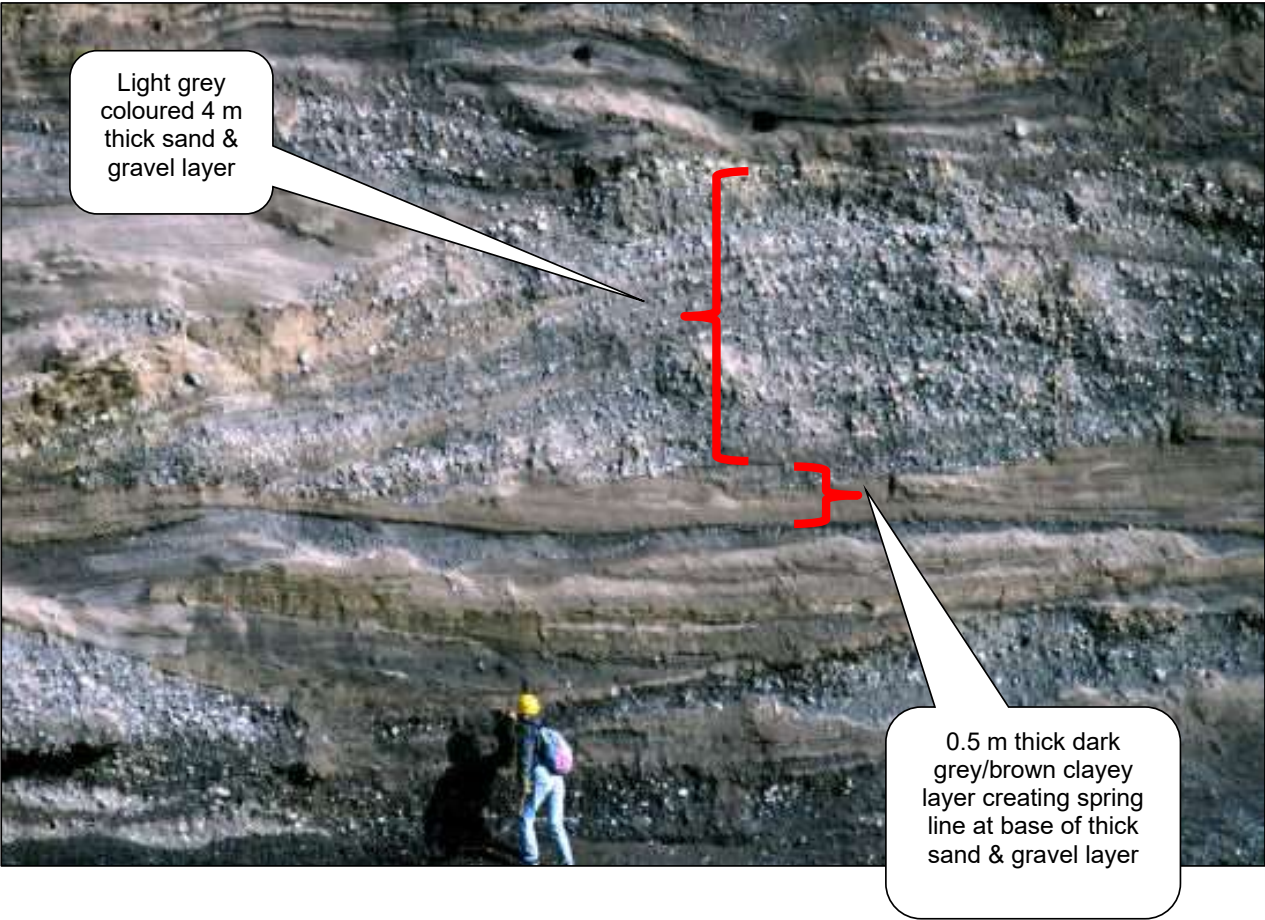


Figure 9.6
Geological Log for Borehole 13 m to NW of RTH

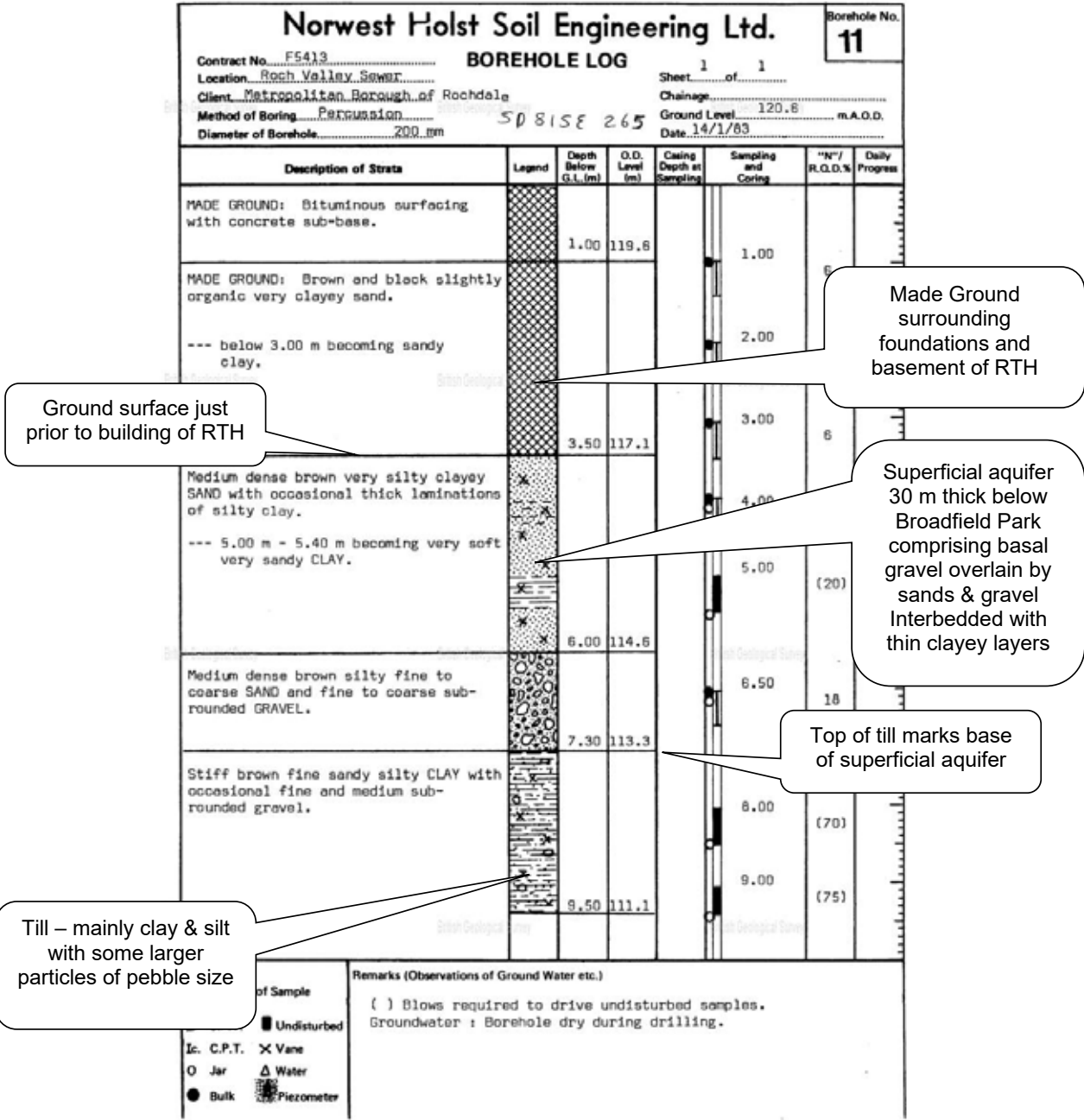


Figure 9.7
Lidar DTM Contour Plots Showing Spring Lines and Water Supply Systems Arrangements
Following Construction of Central Corn Mill

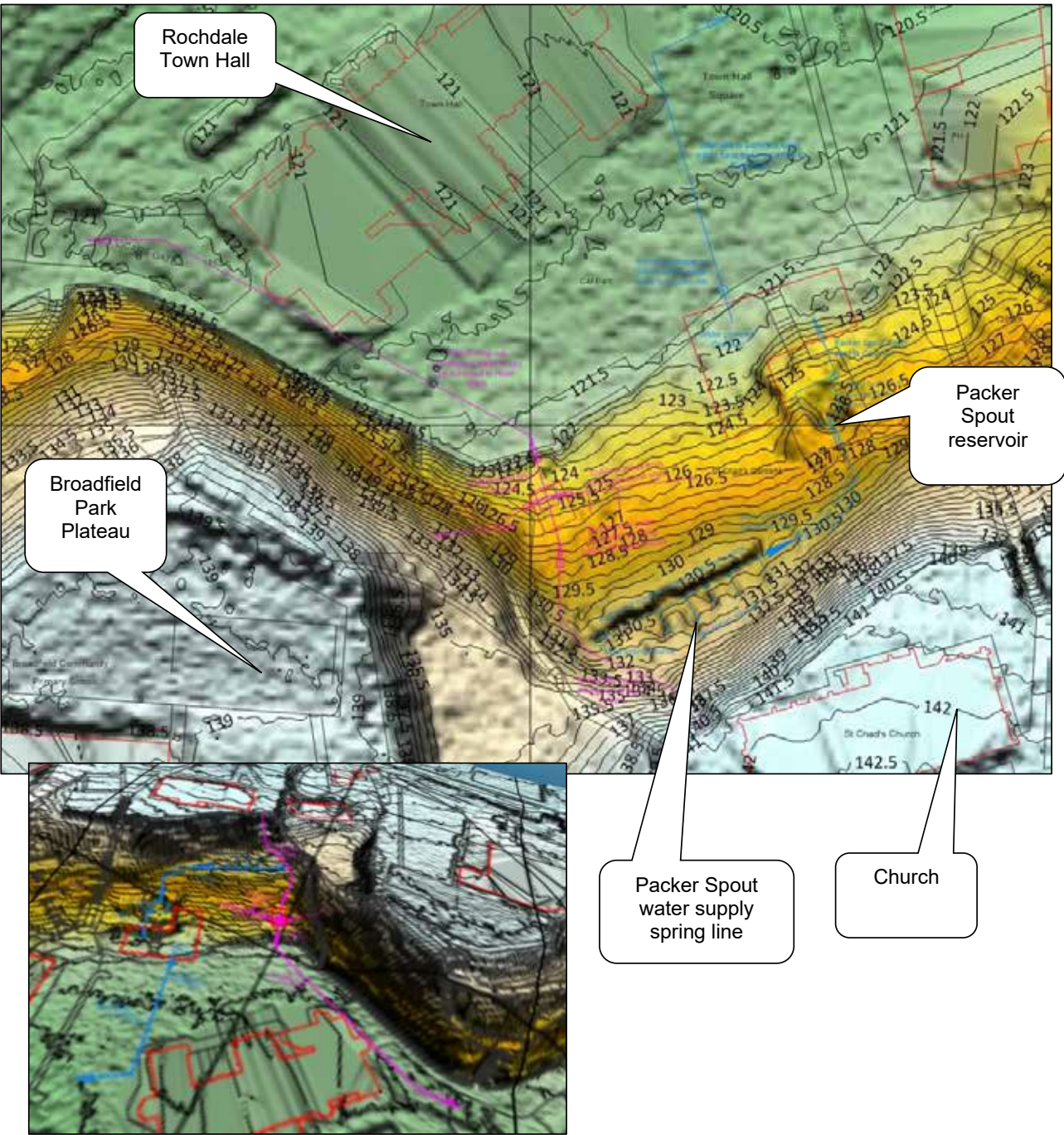


Figure 9.8
RTH SE-NW Geological Cross-Section

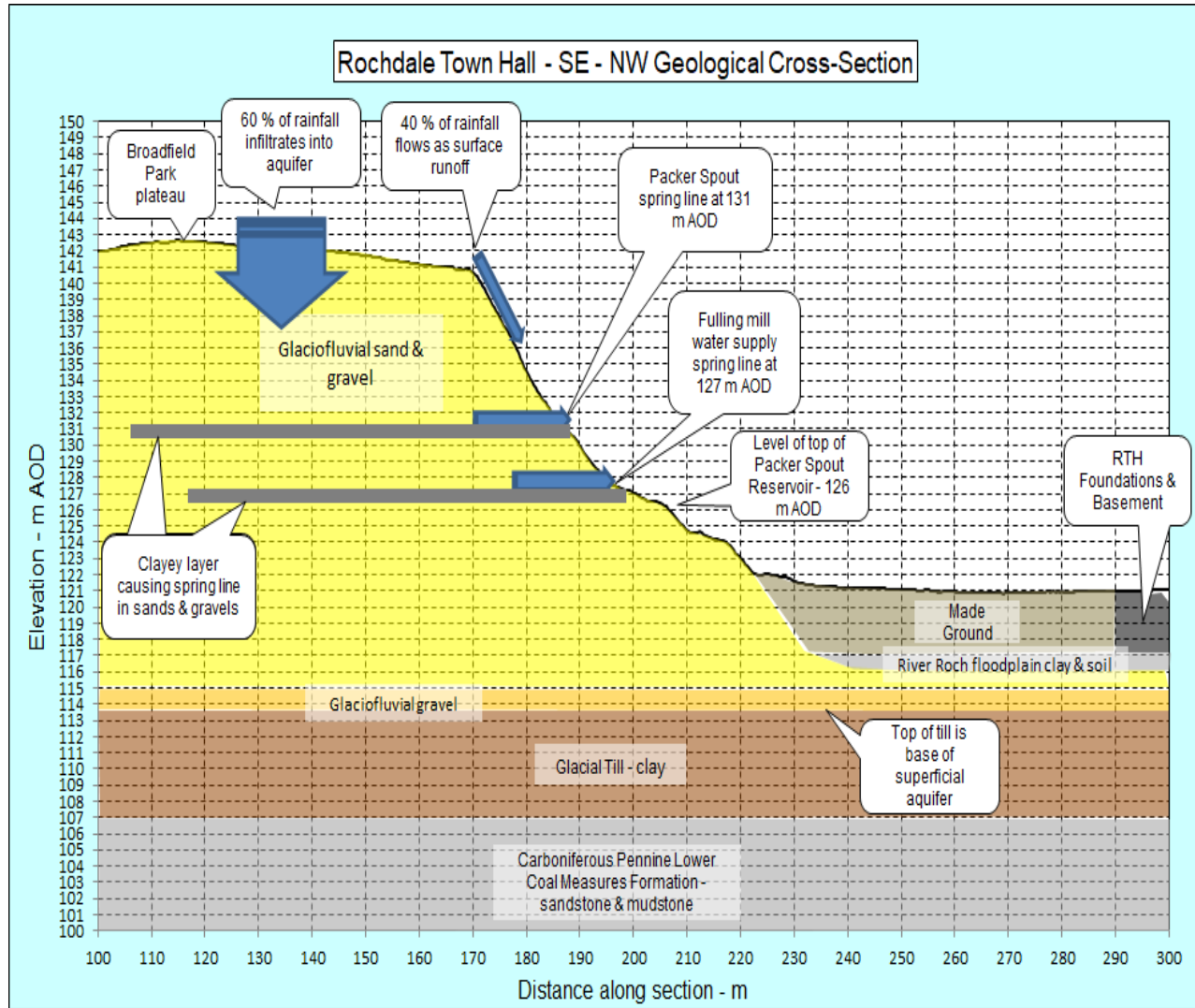


Fig. 9.9.
Superficial Deposits Aquifer Classification Map

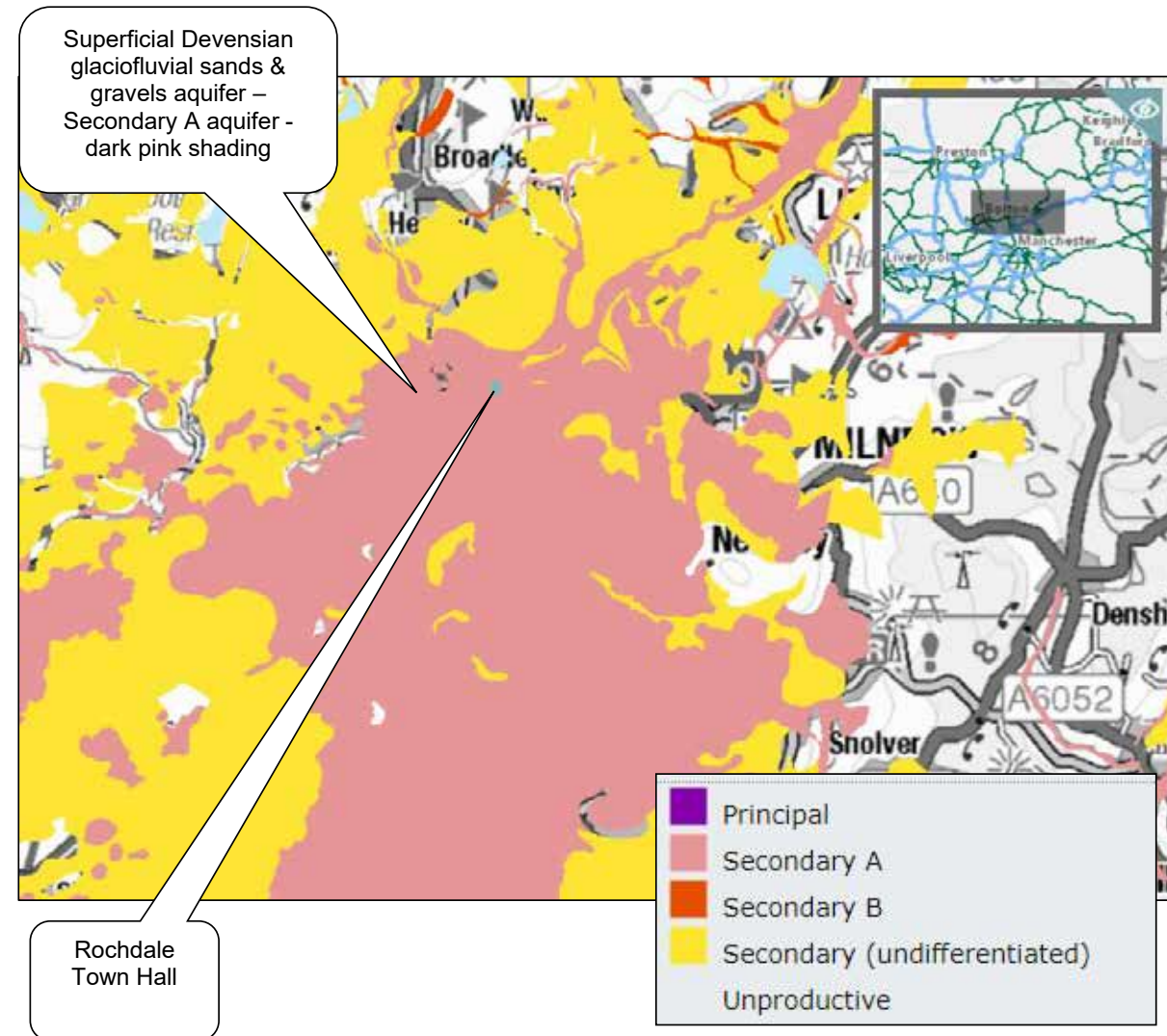


Figure 9.10
Aquifer Classification Descriptions

Principal Aquifers

These are layers of rock or drift deposits that have high intergranular and/or fracture permeability - meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale. In most cases, principal aquifers are aquifers previously designated as major aquifer.

Secondary Aquifers

These include a wide range of rock layers or drift deposits with an equally wide range of water permeability and storage. Secondary aquifers are subdivided into two types:

Secondary A - permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as minor aquifers;

Secondary B - predominantly lower permeability layers which may store and yield limited amounts of groundwater due to localised features such as fissures, thin permeable horizons and weathering. These are generally the water-bearing parts of the former non-aquifers.

Secondary Undifferentiated - has been assigned in cases where it has not been possible to attribute either category A or B to a rock type. In most cases, this means that the layer in question has previously been designated as both minor and non-aquifer in different locations due to the variable characteristics of the rock type.

Unproductive Strata

These are rock layers or drift deposits with low permeability that have negligible significance for water supply or river base flow.

Note: Only the principal and secondary aquifers are displayed as coloured areas on the maps. All uncoloured areas on the bedrock designation map will be unproductive strata.

Figure 9.11
Daily Rainfall and Packer Spout Main Spring Source Flow Rates for June & July 2022 Based on Datalogger Water Levels

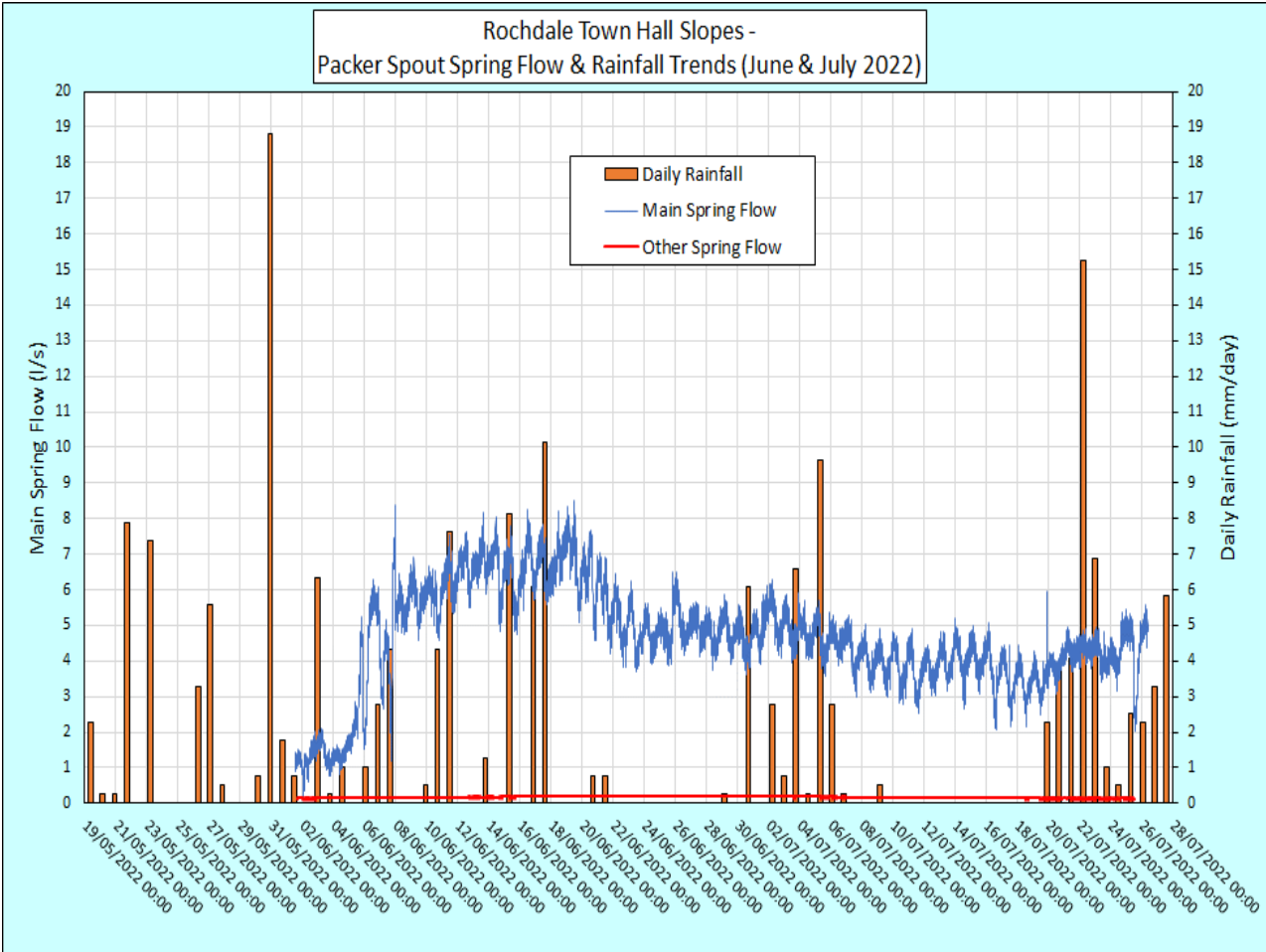
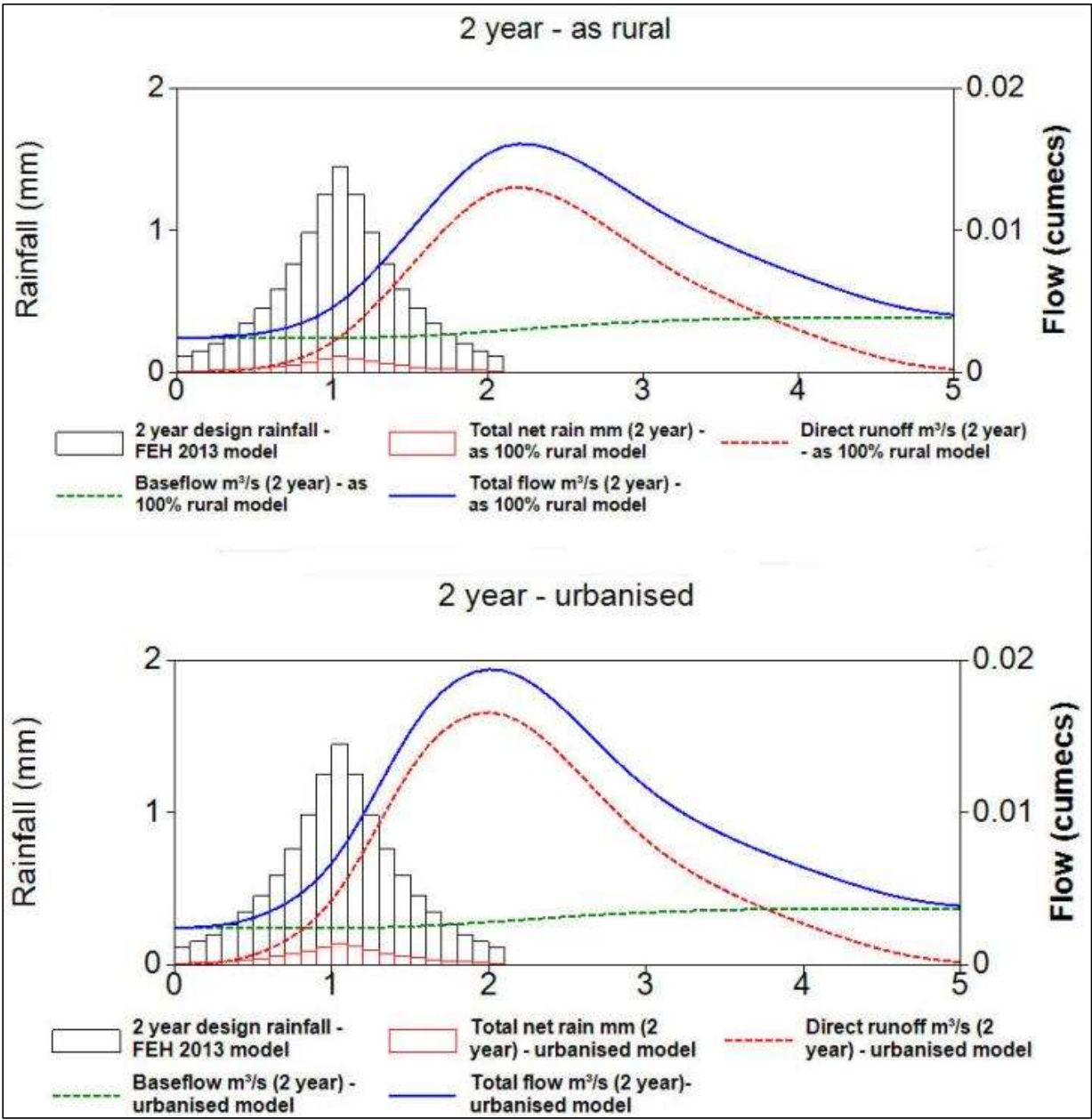


Fig. 9.12

ReFH2 Estimate of Base Flow for the RTH 10.24 ha Catchment Area above RTH



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10.0 Summary of Slopes Catchments & 100 Year Return Period Storm Runoff Rates and Volumes

In addition to the flow from springs, surface runoff will also flow off the slopes towards RTH. This runoff will have to be managed by the water drainage system surrounding RTH to avoid those drains being overwhelmed.

For each of the slopes' catchments from which runoff could flow towards RTH and into the drains surrounding it the current 100 year storm event runoff rates and volumes have been calculated to help determine whether the existing drainage systems can cope with the flows or whether additional flow attenuation should be installed such as cellular storage. It should be noted that the design of additional attenuation structures should also consider a 40% increase in storm intensity due to climate change.

Based on Lidar data contour and watershed maps the Broadfield Park slopes to the south of Rochdale Town Hall (RTH) have been divided into 6 main catchments (Figs. 10.1 to 10.5). The 6 catchments have been named

- 1 School Playing Fields
- 2 Old Vicarage
- 3 Church
- 4 Slopes West
- 5 Slopes Central
- 6 Slopes East

In defining the area of the catchments, it has been assumed that existing road drains will intercept any upslope runoff outside the catchments. The blue arrows in Figs. 10.1 & 10.3 indicate the main possible exit routes for runoff from the catchments.

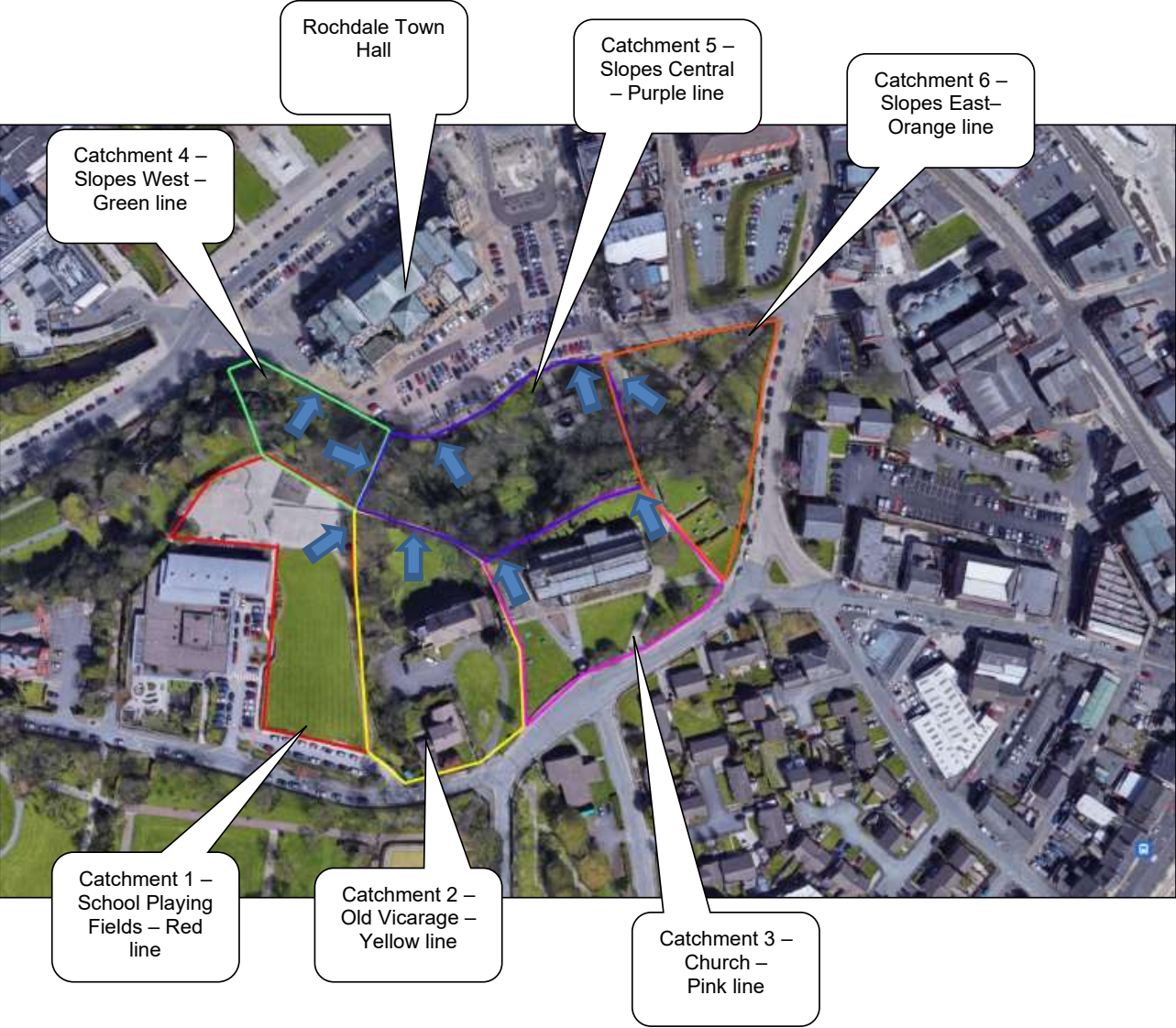
Details of the catchments including total area, the proportion of the surface area which is made up by impermeable surfaces, 100 year storm peak runoff volume and flow rate and the % of rainfall flowing as surface runoff are listed in Fig. 10.5. Example calculations of 100 year storm peak runoff volume and flow rate and the % of rainfall flowing as surface runoff are presented in Figs. 10.6 and 10.7.

On average about 41% of the volume of the 100 year storm event rainfall is expected to flow as surface runoff. The estimated current 100 year storm event runoff rates varied from 1.1 l/s to 4.0 l/s. Future 100 year storm event runoffs are expected to be significantly higher.

Later Sections discuss the slopes catchments in more detail and factors which need to be considered when controlling runoff from the catchments to RTH.

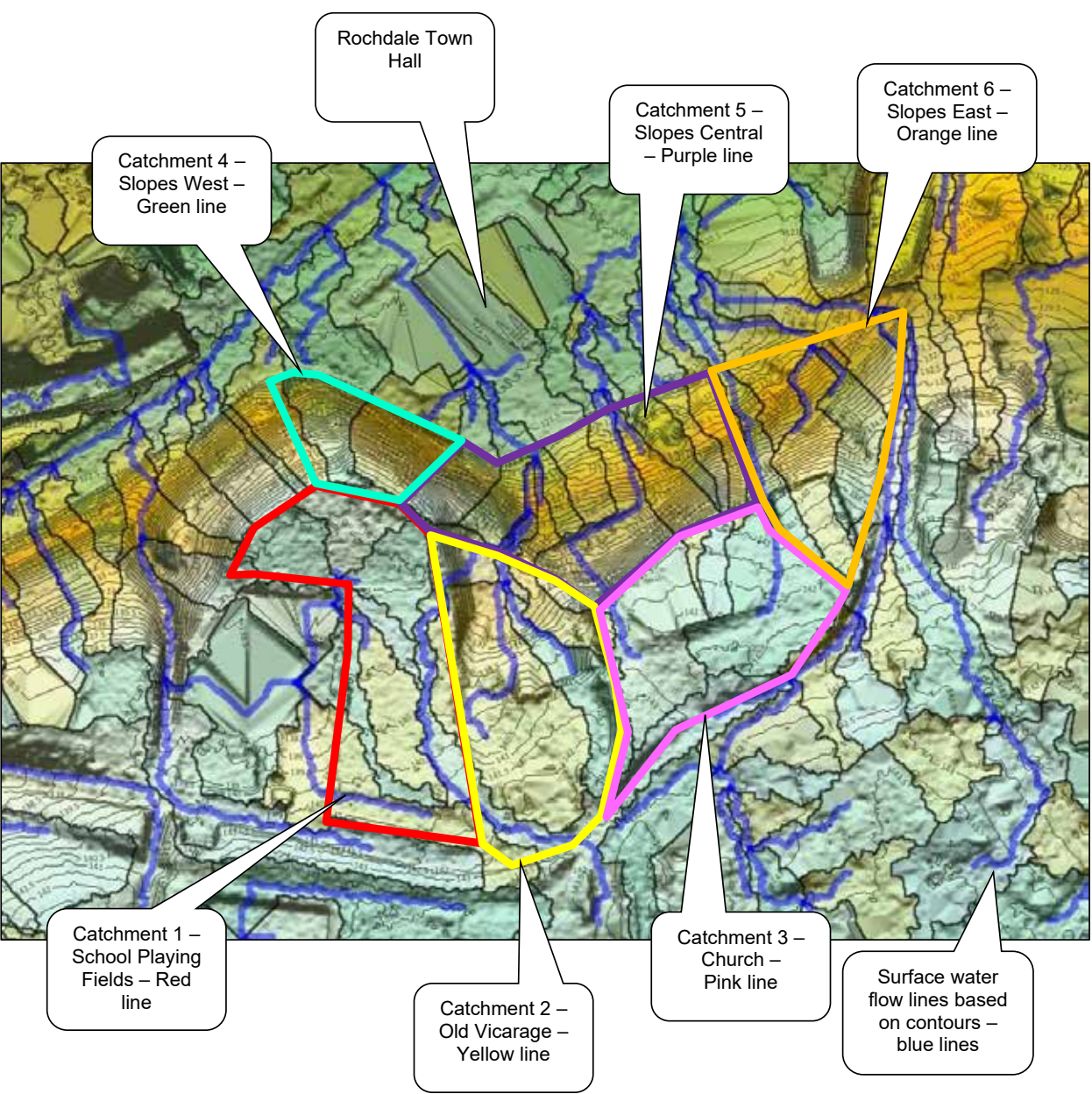
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Figure 10.1
Aerial Photo Showing 6 Main Water Runoff Catchments for the RTH Slopes
(Blue arrows indicate main exit routes for runoff from catchments)



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Figure 10.2
Lidar Contour & Water Flowline Plot Showing 6 Main Water Runoff Catchments for the RTH Slopes



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Figure 10.3
 '3D' Aerial Photo Showing 6 Main Water Runoff Catchments for the RTH Slopes
 (Blue arrows indicate main exit routes for runoff from catchments)

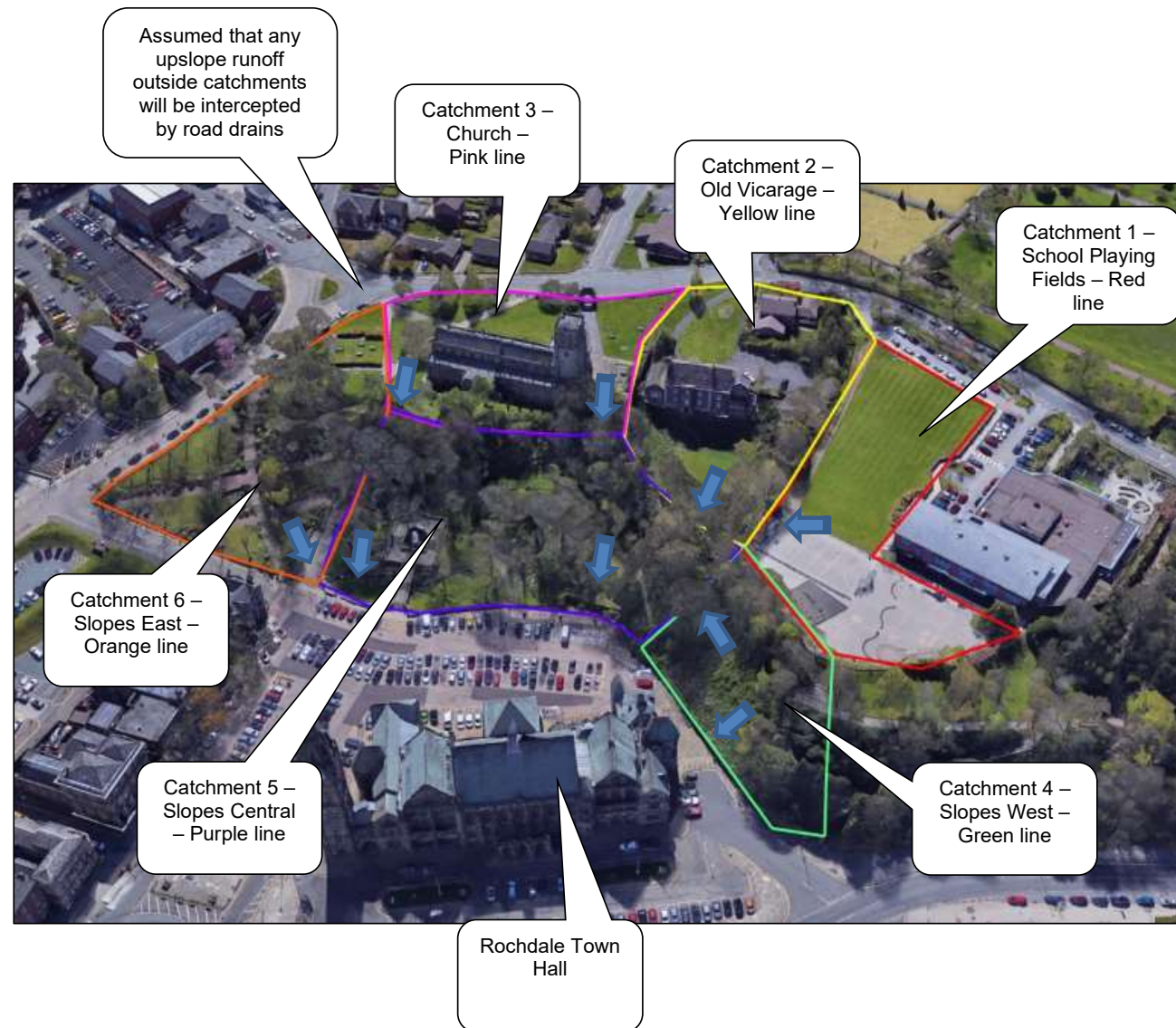


Figure 10.4
 '3D' Lidar Contour & Water Flowline Plot Showing 6 Main Water Runoff Catchments
 for the RTH Slopes

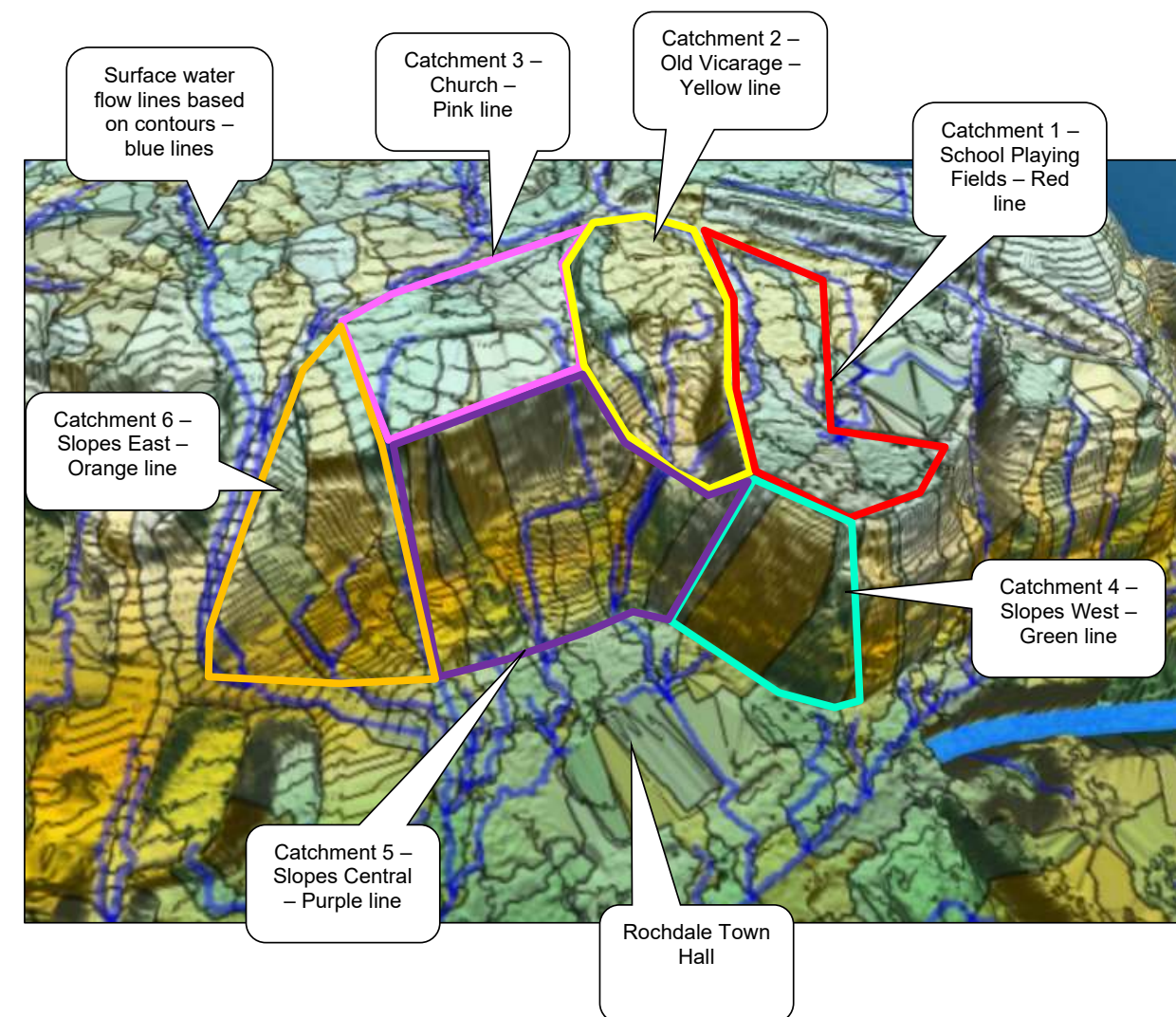


Figure 10.5
Details of Catchments Including Total Area, Impermeable Surfaces Proportion, 100
Year Storm Peak Runoff Volume & Flow Rate & % of Rainfall Flowing as Surface
Runoff

Catchment No.	Catchment Name	Total Area (m ²)	Impermeable Area (m ²)	Impermeable Area (fraction)	Current 100 years storm event runoff rate (l/s)	Current 100 year storm event runoff volume (m ³)	% of 100 year storm rainfall flowing as surface runoff
1	School Playing Fields	4751	1774	0.373	2.8	201.3	44.0
2	Old Vicarage	5723	1394	0.244	3.7	227.7	41.4
3	Church	4855	2275	0.469	4.0	214.9	46.0
4	Slopes West	2131	45	0.021	1.1	75.3	36.8
5	Slopes Central	5719	307	0.054	2.9	206.0	37.4
6	Slopes East	4476	1056	0.236	2.8	177.6	41.2

Figure 10.6
Example Calculation of Catchment 100 Year Storm Event Runoff Rate

Ambellia Consultancy		Page 1	
11 Longwood House		Catchment 3 - Church	
Love Lane		Rochdale Town Hall Slopes	
Cirencester, Glos, GL7 1YG		Rochdale, OL16 1AB	
Date 28/04/2022 18:49		Designed by Dr. R. Evans	
File RTH Catch 3 church.SRCX		Checked by	
Innovyze		Source Control 2020.1.3	

ICP SUDS Mean Annual Flood

Input

Return Period (years)	10	Soil	0.300
Area (ha)	0.486	Urban	0.469
SAAR (mm)	856	Region Number	Region 10

Results l/s

QBAR Rural 1.1

QBAR Urban 2.3

Q10 years 3.1

Q1 year 2.0

Q30 years 3.5

Q100 years 4.0

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Figure 10.7
Example Calculation of Catchment 100 Year Storm Event Runoff Volume

mbellia Consultancy		Page 1
11 Longwood House	Catchment 3 - Church	
Love Lane	Rochdale Town Hall Slopes	
Cirencester, Clos, GL7 1YG	Rochdale, OL16 1AB	
Date 28/04/2022 15:11	Designed by Dr. R. Evans	
File RTH Catch 3 church.SRCX	Checked by	
Innovyze	Source Control 2020.1.3	

Greenfield Runoff Volume

FEH Data

Return Period (years)	100
Storm Duration (mins)	960
FEH Rainfall Version	1999
Site Location GB 382150 163150 ST 82150 63150	
C (1km)	-0.023
D1 (1km)	0.388
D2 (1km)	0.382
D3 (1km)	0.208
E (1km)	0.278
F (1km)	2.507
Areal Reduction Factor	1.00
Area (ha)	0.486
SAAR (mm)	856
CWI	120.055
SPR Host	30.000
URBEXT (1990)	0.4660

Results

Percentage Runoff (%)	45.97
Greenfield Runoff Volume (m³)	214.944

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11.0 Slopes Catchments 1 – School Playing Fields - RTH Drainage
Considerations

As discussed in Section 12, catchment 1 is the playgrounds area of the school which has a surface area 4,751 m²,

The 1926 aerial photo (Fig. 11.1) shows the area of the catchments prior to the building of the school on the western side of the vicarage gardens.

It is possible that the runoff from the school playgrounds could follow the route of the original site drainage into the remaining part of the vicarage gardens (Fig. 11.2).

The silt trap manhole in the vicarage garden next to garden gate receives runoff from the direction of Catchment 1 (School playing fields) and so could potentially be receiving runoff from Catchment 1 (Figs. 11.3).

Fig. 11.1
1926 Aerial Photo Showing Catchment 1 in Vicarage Gardens Before Building of
Primary School

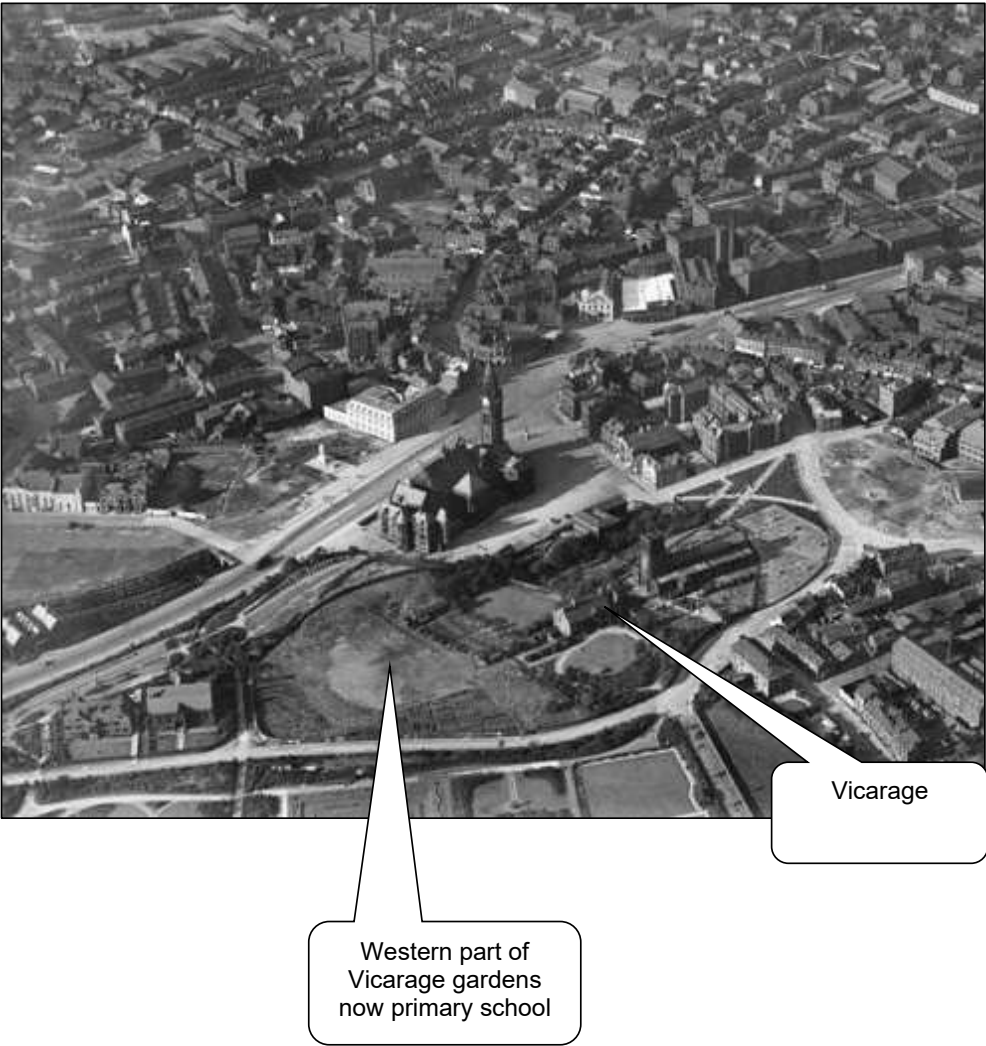


Fig. 11.2
Catchment 1 - Grassed & Hard Standing Playground Areas Could Still Possibly Drain
to Vicarage Garden as per Original Drainage Before Construction of School

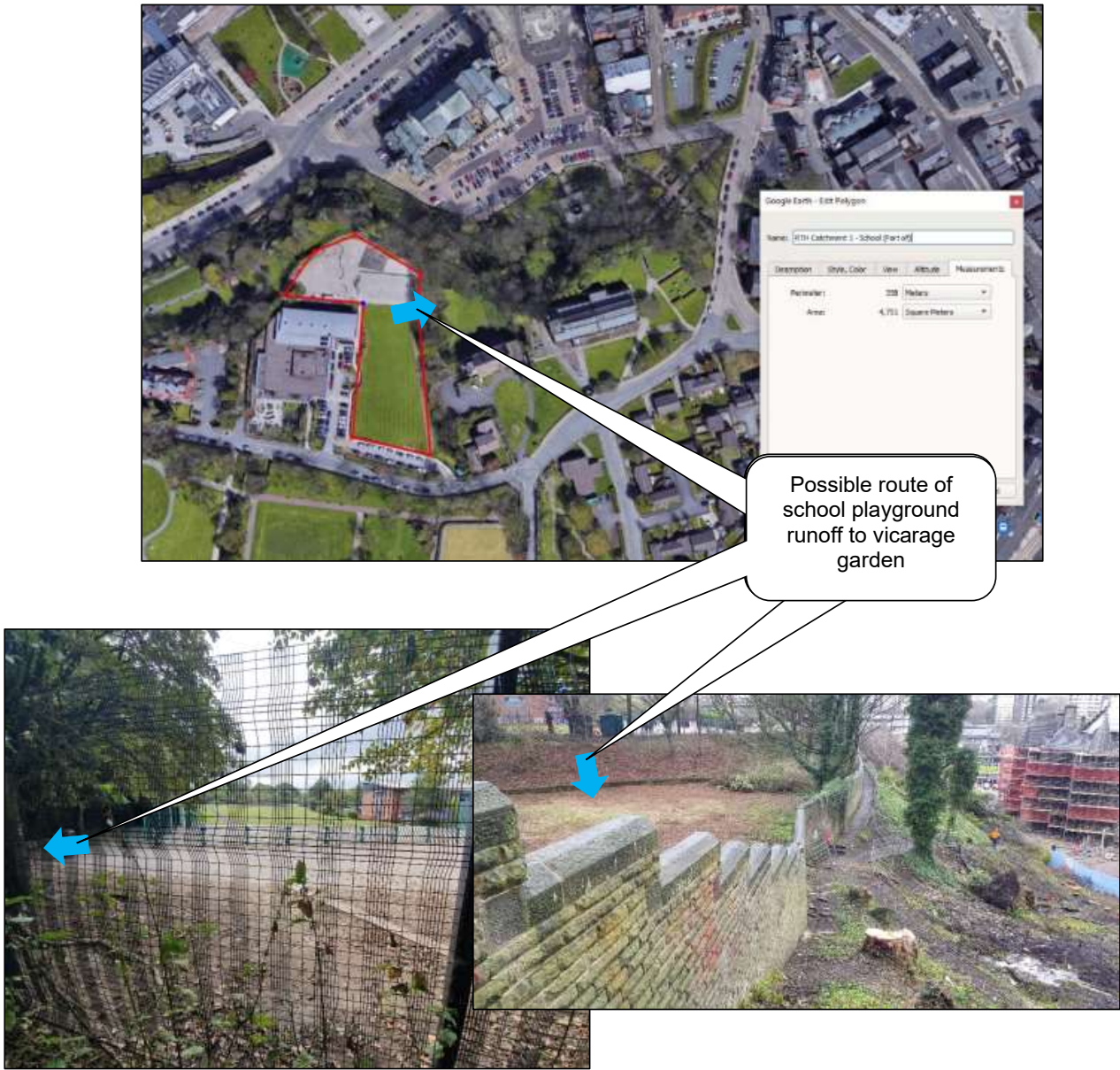


Fig. 11.3
Manhole Next to Garden Gate Receiving Runoff from Direction of Catchment 1
(School Playing Fields)



12.0 Catchment 2 – Old Vicarage (now Partnership House) - RTH
Drainage Considerations

Catchment 2 is the remaining part of the old vicarage with a surface area of 5,723 m² (Fig. 12.1).

The manhole in the vicarage garden next to garden gate (which leads to the slopes) receives runoff from the direction of Catchment 1 (School playing fields) and Catchment 2 (Vicarage) and discharges it to the Slopes (Fig. 12.2).

Dye tracer tests indicated that the water leaving the manhole in the Vicarage garden entered the slopes drainage system at, or below, the level of the underground brick chamber, which was originally part of the fulling mill water supply system (See Fig. 4.2).

Consequently, it is not thought that the outflow from the vicarage garden manhole enters the Packer Spout spring water supply.

The walkover investigations of drainage from the church and the old vicarage to the slopes revealed that it is highly probable that foul water from the church and the vicarage is being discharged to the slopes via the manhole adjacent to the gate in the wall. Fig. 12.3 shows the probable route of foul water from church to the slopes via a manhole in the vicarage gardens. The church website confirms that the church has toilet facilities (Fig. 12.4). The location of a possible septic tank or cesspit in church yard to the west of the church is also shown in Fig. 12.4.

It is possible that the 2 manholes in the terrace immediately to the north of the old vicarage are receiving combined foul water and roof runoff from vicarage and then discharging to the slopes via the same manhole receiving the foul water from the church (Fig. 12.5).

The extent of the treatment of the vicarage foul water is not known. It is not known if there is a septic tank or cesspit in the vicarage gardens.

Information needs to be obtained on the foul water management and treatment for the church and the vicarage.

If required, steps should be taken to rectify any inadequacies in the current foul water treatment by connecting the church and the vicarage to the mains sewer or installing package water treatment plants.

Fig. 6.3

Catchment 2 – Old Vicarage Discharging Runoff (& Probably Foul Water) to Slopes
Via Manhole by Gate

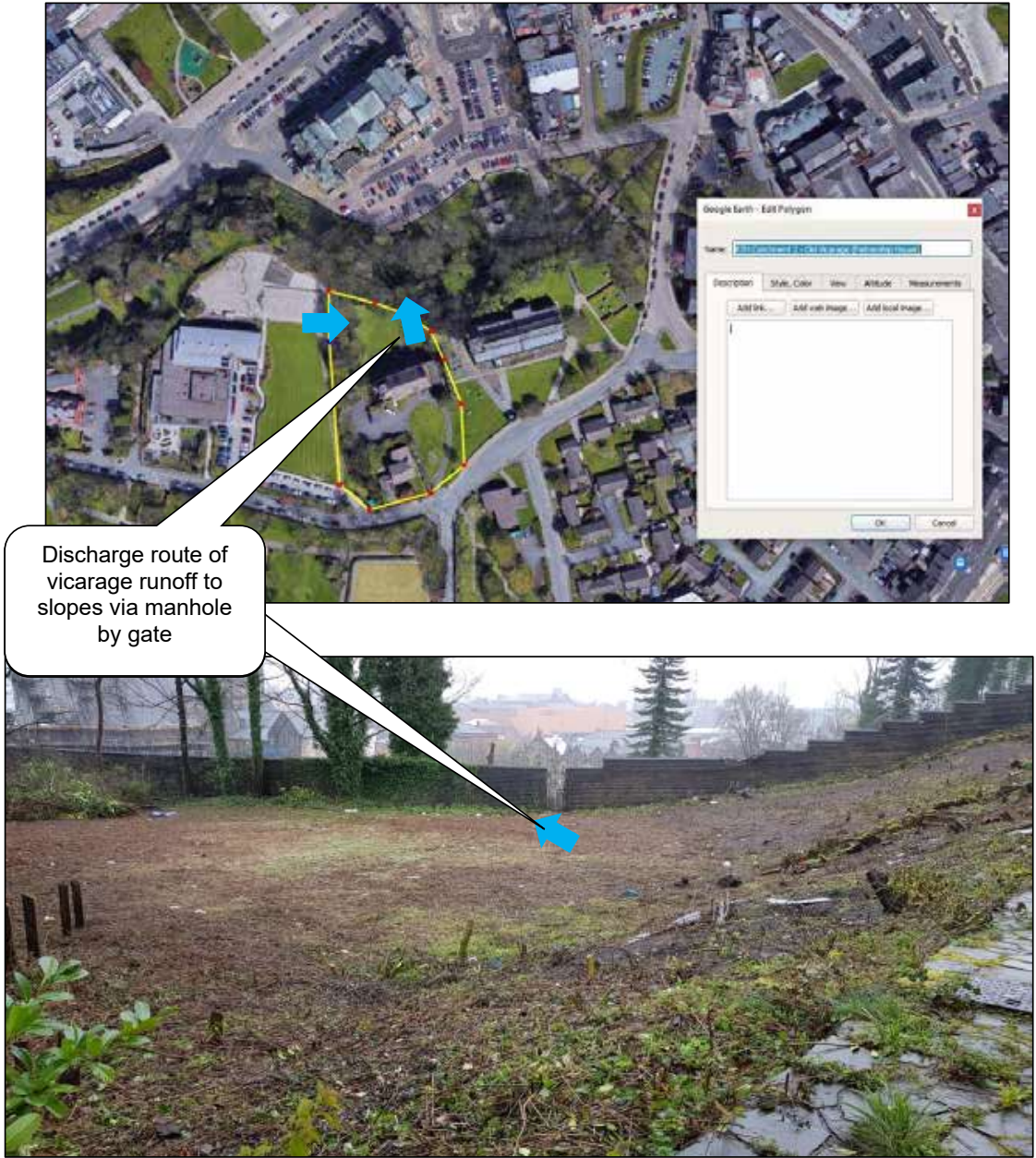


Fig. 12.2

Manhole Next to Garden Gate Receiving Runoff from Catchment 1 (School playing fields) and Catchment 2 (Vicarage) and then Discharging to the Slopes



Fig. 12.3

Probable Route of Foul Water from Church to the Slopes Via Vicarage Gardens

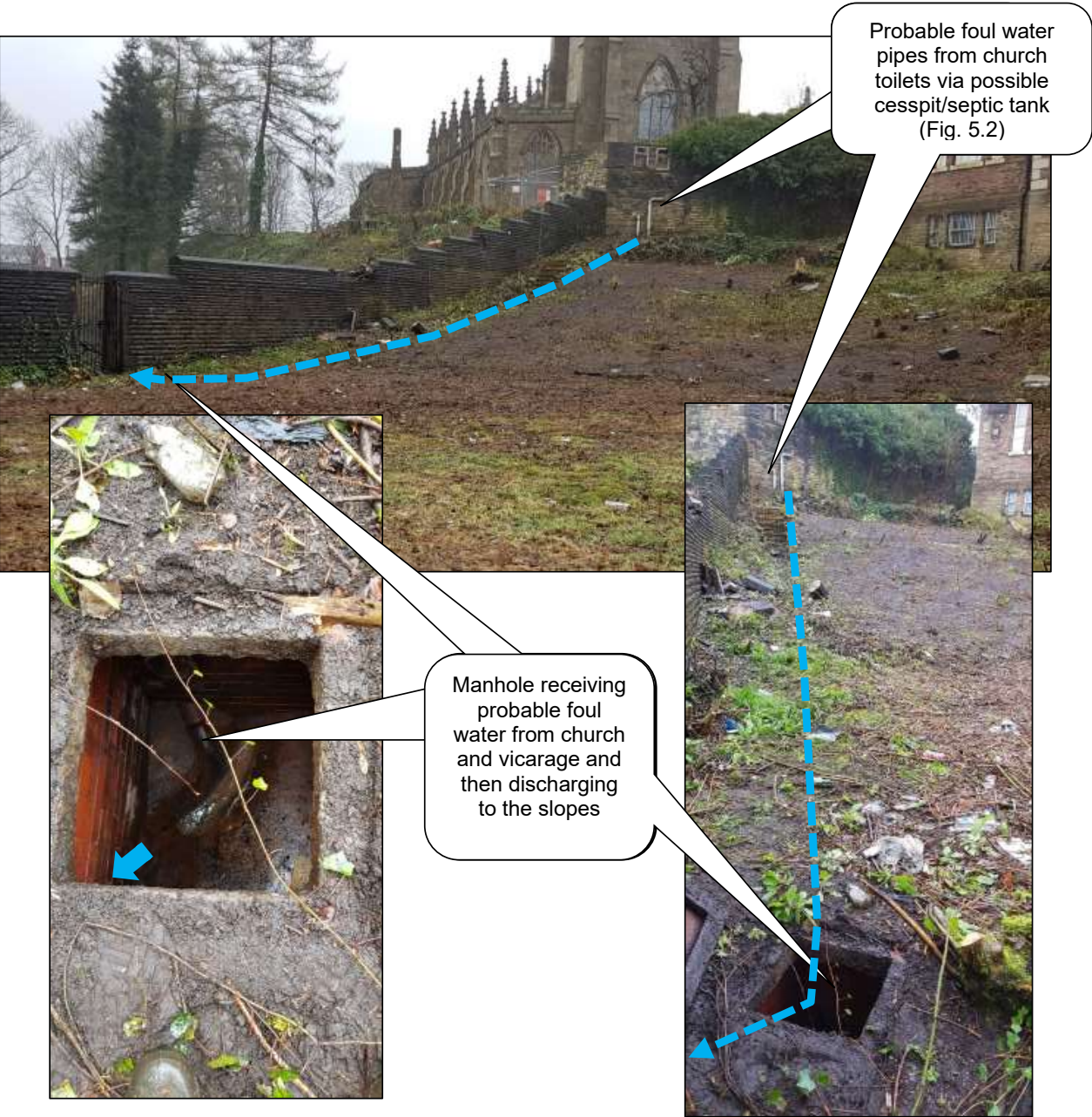


Fig. 12.4

Confirmation St Chad's Church has Toilets and Possible Septic Tank or Cesspit in Church Yard to the West of the Church

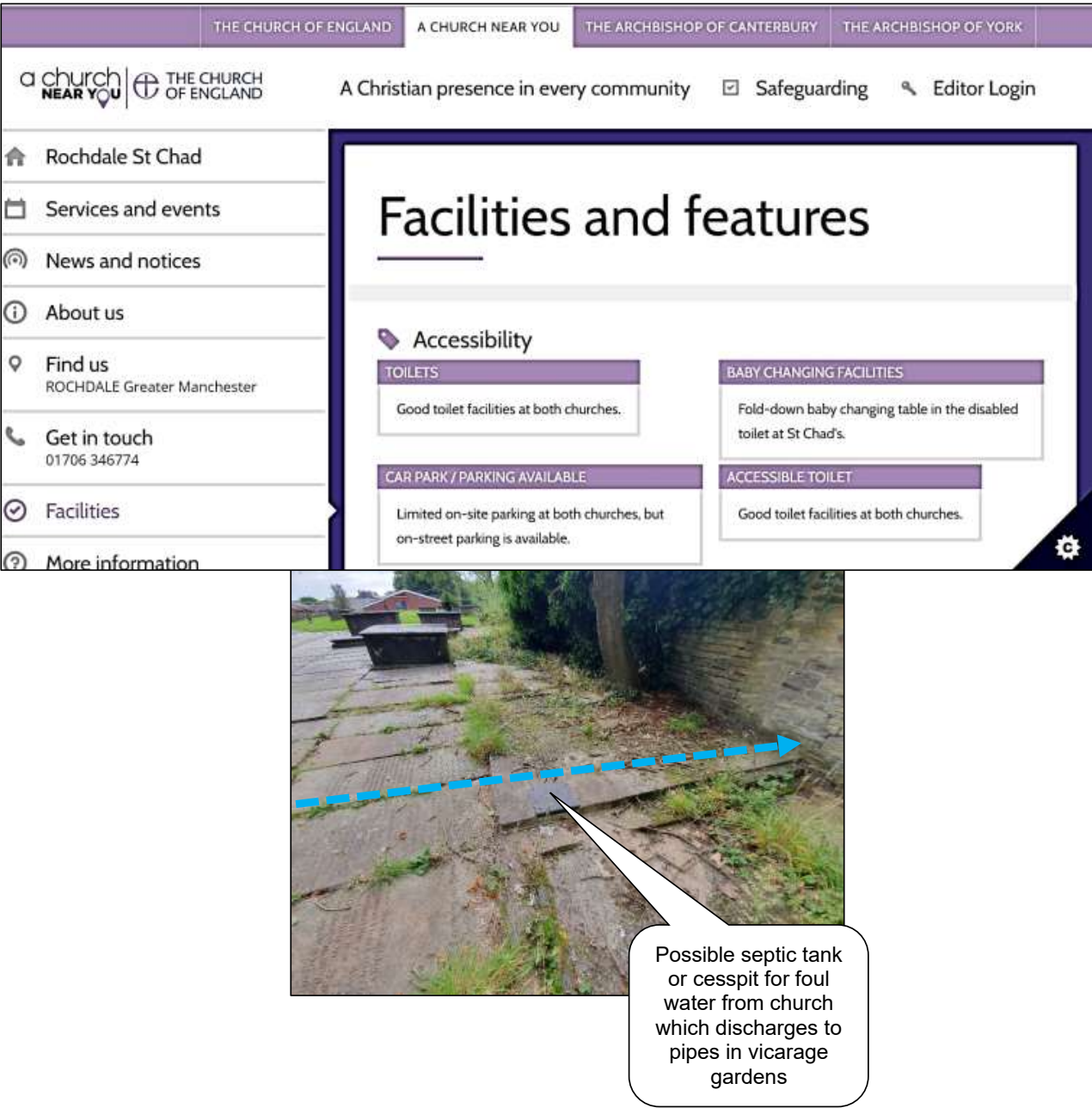
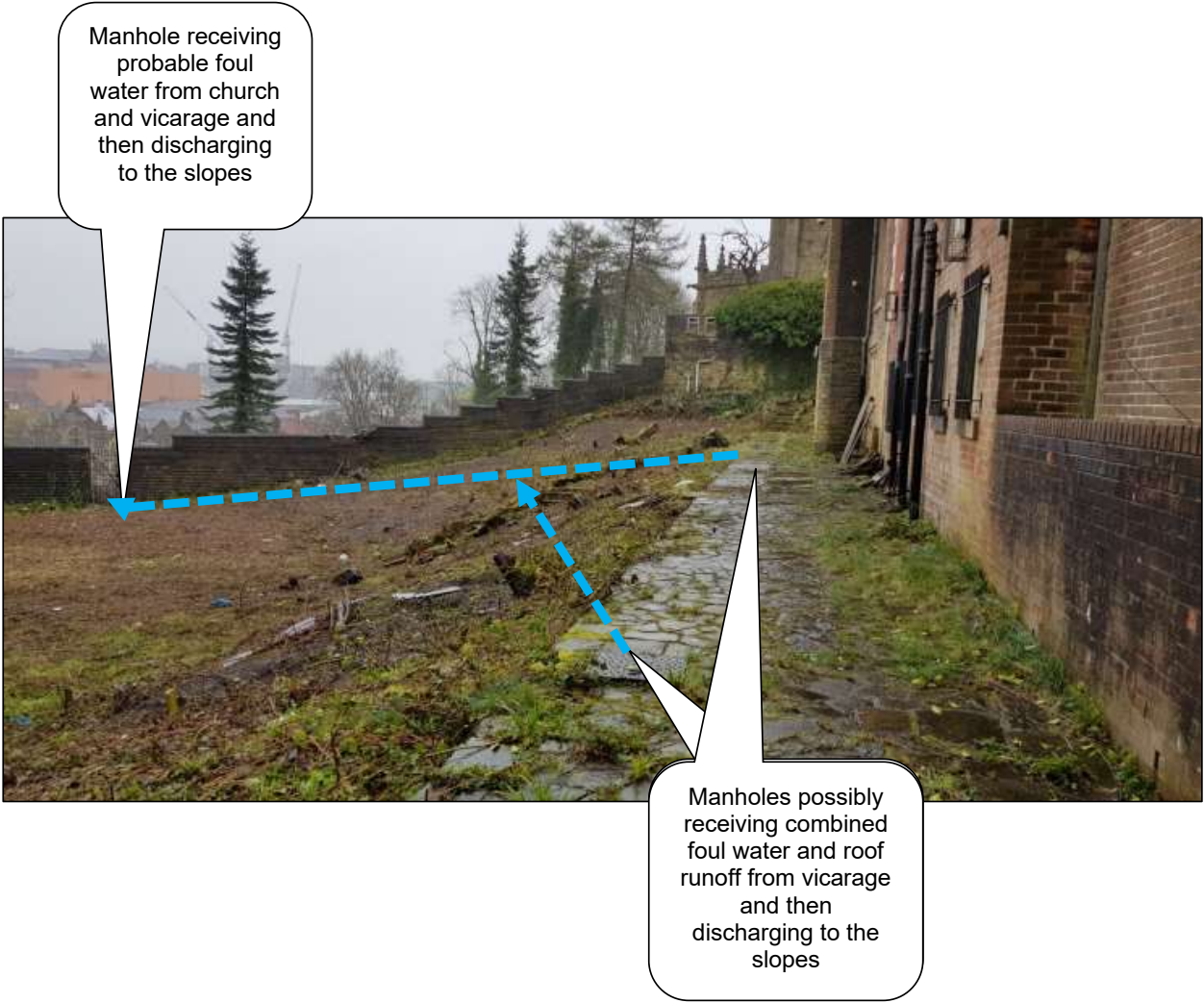


Fig. 12.5

Manholes Possibly Receiving Combined Foul Water and Roof Runoff from Vicarage and then Discharging to the Slopes



13.0 Catchment 3 – St Chad's Church - RTH Drainage Considerations

Catchment 3 is St Chad's church and church yard, which has a surface area 4,855 m² (Fig. 13.1)

Runoff from the church roof and paved church yard is discharged both to the west and east of the of the church (Figs 13.1 & 13.2).

On the western side of the church, an open channel discharges runoff into the pond and open channel of the Packer Spout water supply (Fig. 13.2).

On the eastern side of the church, the runoff is directed into the drains at the top of the church steps (Fig. 13.3).

Fig. 13.1
Catchment 3 – Church Discharging Runoff to Slopes to the West and East
of St Chad’s Church

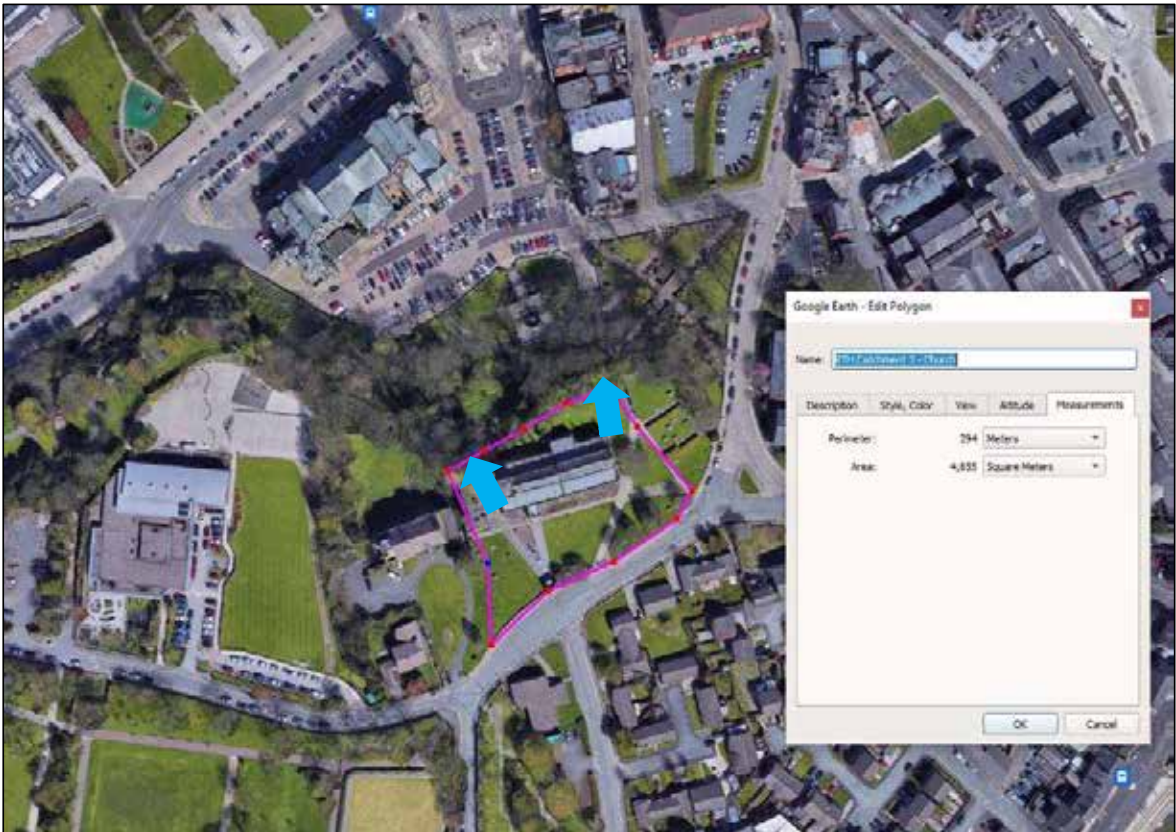
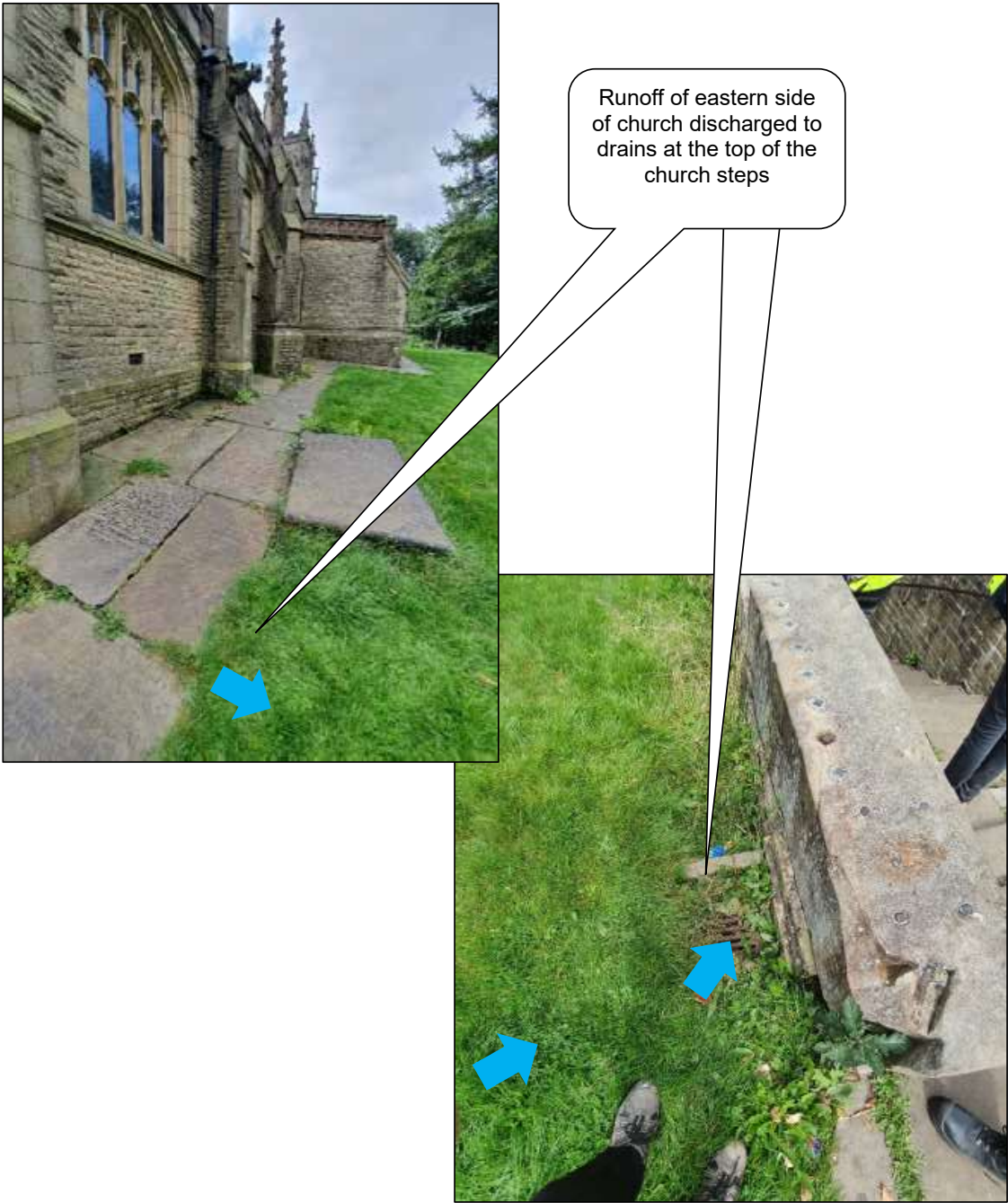


Fig. 13.2
West Side of Church Discharging Runoff to Slopes Via Open Channel to Packer
Spout Water Supply – Open Channel Splits into Two Channels Halfway Down Slope



Fig. 13.3
Church Discharging Runoff to Slopes Via Open Channel to Pond Below Vicarage
Gardens – Open Channel Splits into Two Channels Halfway Down Slope



14.0 Catchment 4 – Slopes West - RTH Drainage Considerations

Catchment 4 is the steep western side of the slopes (Slopes West) which has a surface area 2,131 m² (Fig. 14.1)

Part of the Slopes West runoff is drained to the culvert and underground brick chamber on west side of Slopes Central (catchment 5) (Fig. 14.2).

Some Slopes West runoff to flows to behind the retaining wall to west of RTH where seep holes release build-ups of water (Fig. 14.3).

Any new planting on the slopes should not disturb the existing drainage.

Fig. 14.1
Catchment 4 – Slopes West

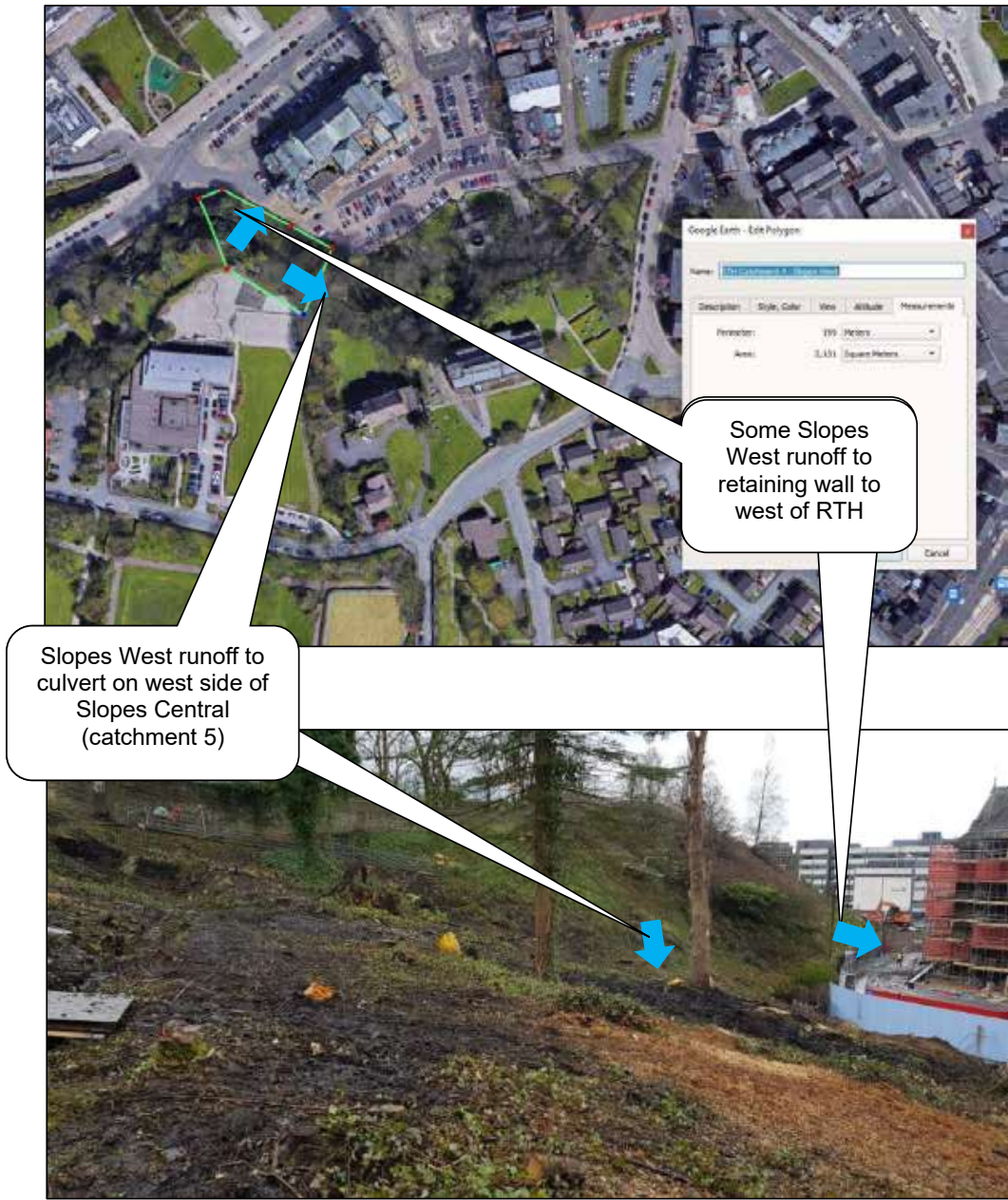


Fig. 14.2
Catchment 4 – Slopes West runoff to culvert and underground brick chamber on west side of Slopes Central (catchment 5)



Fig. 14.3

Catchment 4 – Some Slopes West runoff to retaining wall to west of RTH



15.0 Catchment 5 – Slopes Central - RTH Drainage Considerations

Catchment 5 is the Slopes Central which has a surface area 5,719 m² (Fig. 15.1). The evolution of the Slopes Central drainage in relation to the development of the two water supplies for, respectively, Packer Spout and the Fulling Mill since the mid-18th Century has been documented in Sections 2 to 5 of this report.

The recent state of the two water systems following the demolition of the Central Corn Mill in the 1930's, and the tree clearance in March and April 2002, has been discussed in Sections 6, 7 & 8. The hydrogeological model for the spring flows is presented in Section 9.

With the dual objectives of restoring the main historical features of the two water supply systems and improving the slopes drainage in general, the key recommended actions are discussed below.

The lateral land drains at the base of the slope need to be replaced, with the land drain gravel covered by a permeable geotextile membrane to prevent silt blocking the land drain gravel. Runoff attenuation should be installed if required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.

The key considerations regarding the Packer Spout water supply system are

- i) Restoring the catchpit for the main spring source so that sand is trapped in the catchpit and the spring water flows directly into half-round culvert and so removing the pond
- ii) Restoring the other 5 spring catchpits which can also act as sand/silt traps
- iii) Removing the back plastic pipes and other debris from the half round pipes
- iv) Refilling the round holes in the main west-east half-round culvert with lime mortar to stop water leaking to the slopes
- v) Recovering the half-round culverts with the 90 cm natural stone slabs
- vi) Restoring the round brick sand/silt trap at the eastern end of the main half-round culvert
- vii) Restoring the stone culvert from the round brick chamber to the Packer Spout reservoir
- viii) Cleaning out the Packer Spout reservoir
- ix) Restoring the inflow and outflow controls for the Packer Spout reservoir
- x) Laying new pipe from the Packer Spout Reservoir to the new RTH drainage to avoid current leaking drainage pipes
- xi) Determine if flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
- xii) Prepare maintenance plan including prevention of build-up of sand, silt and algae.

- The key considerations regarding the Fulling Mill water supply system are
- i) Restoring the complete length of the stone culvert from the western side of St Chad's church to the underground brick chamber (which is 11 m from the base of the slope)
 - ii) Restoring the system catchpits and culverts which is draining the lower spring line and channelling water to the underground brick chamber.
 - iii) Determine if flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
 - iv) Prepare maintenance plan

An example flood attenuation structure design for a cellular storage structure at the base of the western side of the central slopes catchment is presented in Fig 15.2.

The cellular storage structure (6 m wide * 20 m long * 2 m deep) was been designed on the basis of receiving 100 year storm flow (+40% for climate change) from a catchment area of 0.33 hectares. The maximum allowable discharge rate from the cellular storage was set at 5 l/s. The design calculations are shown in Fig. 15.3 confirming that the cellular storage would retain the storm flows (status OK).

Fig. 15.1
Catchment 5 – Slopes Central

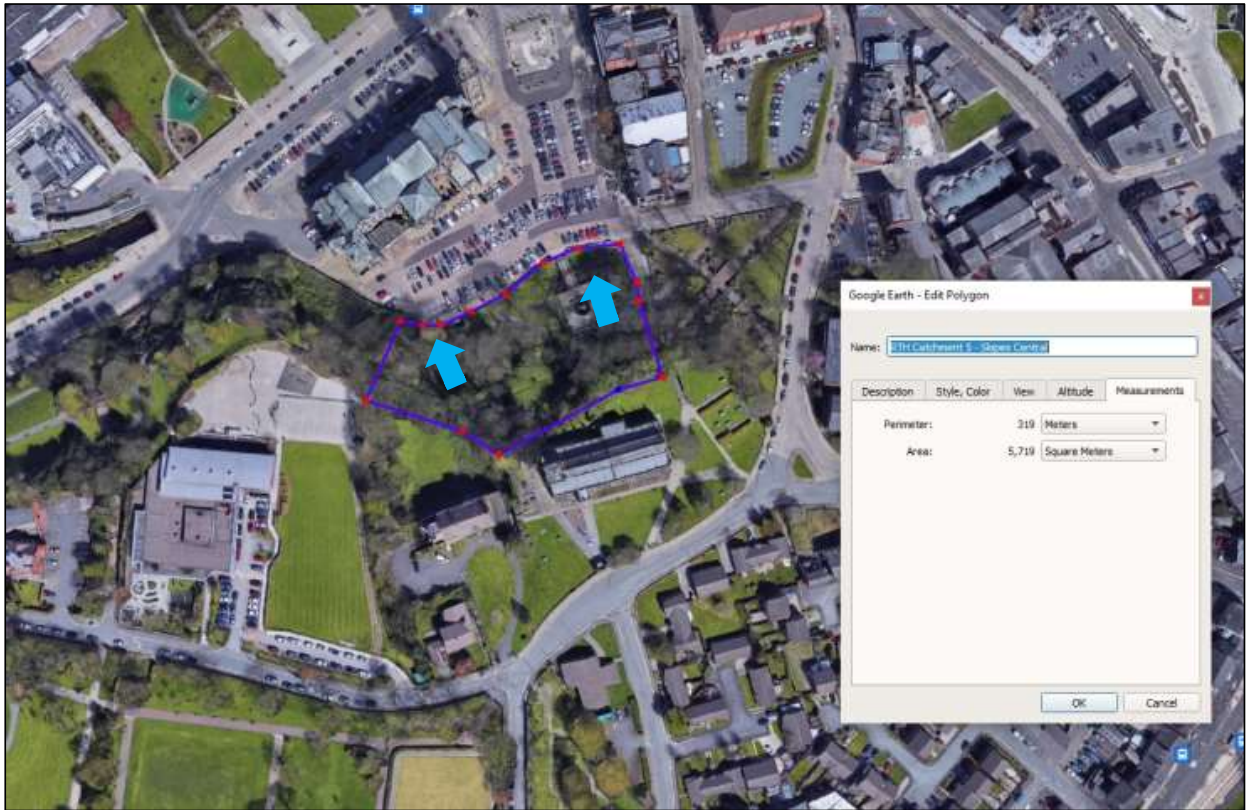
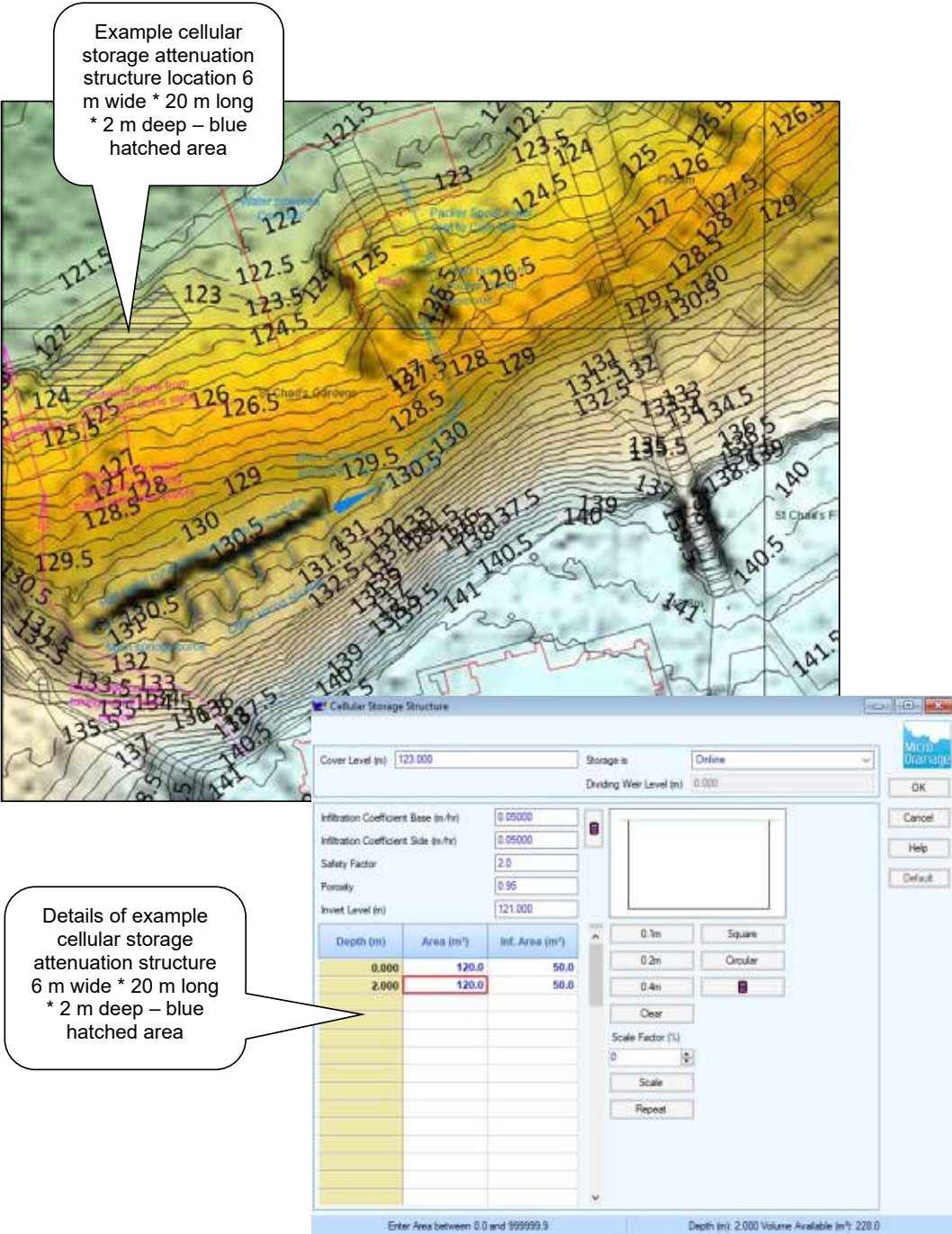
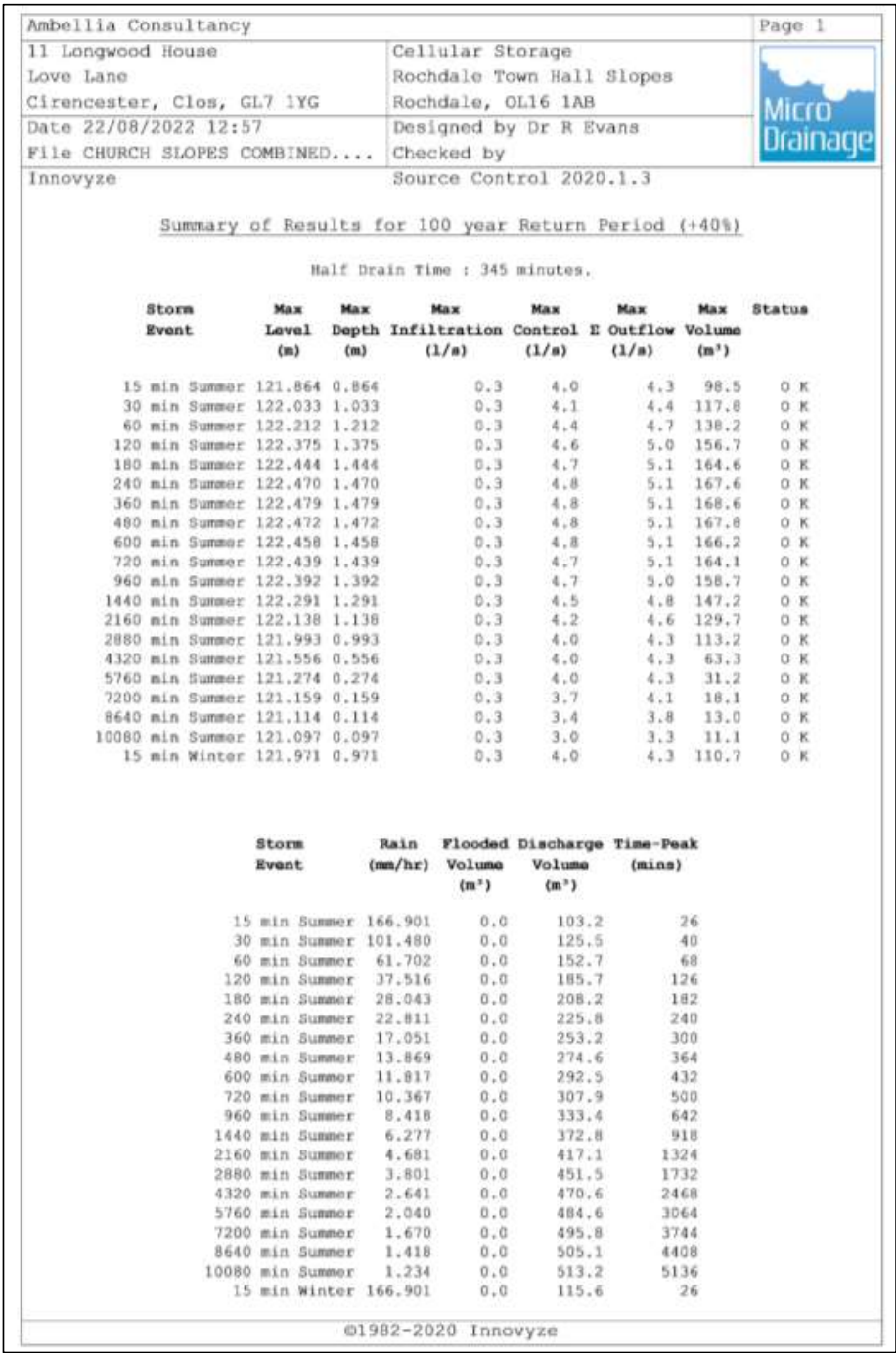


Figure 15.2
Example Cellular Storage Attenuation Structure Location & Dimensions



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Figure 15.3
Summary of Design Calculations for Cellular Storage



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Figure 15.3 (continued)
Summary of Design Calculations for Cellular Storage

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File CHURCH SLOPES COMBINED....

Cellular Storage

Rochdale Town Hall Slopes

Rochdale, OL16 1AB

Designed by Dr R Evans

Checked by

Page 2



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Source Control 2020.1.3

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
30 min Winter	122.164	1.164	0.3	4.3	4.6	132.7	O K
60 min Winter	122.370	1.370	0.3	4.6	5.0	156.2	O K
120 min Winter	122.567	1.567	0.3	4.9	5.3	178.7	O K
180 min Winter	122.659	1.659	0.3	5.1	5.4	189.1	O K
240 min Winter	122.701	1.701	0.3	5.1	5.5	193.9	O K
360 min Winter	122.715	1.715	0.3	5.1	5.5	195.6	O K
480 min Winter	122.704	1.704	0.3	5.1	5.5	194.3	O K
600 min Winter	122.684	1.684	0.3	5.1	5.4	191.9	O K
720 min Winter	122.654	1.654	0.3	5.0	5.4	188.5	O K
960 min Winter	122.577	1.577	0.3	4.9	5.3	179.8	O K
1440 min Winter	122.412	1.412	0.3	4.7	5.0	160.9	O K
2160 min Winter	122.170	1.170	0.3	4.3	4.6	133.3	O K
2880 min Winter	121.948	0.948	0.3	4.0	4.3	108.1	O K
4320 min Winter	121.310	0.310	0.3	4.0	4.3	35.3	O K
5760 min Winter	121.124	0.124	0.3	3.5	3.9	14.2	O K
7200 min Winter	121.095	0.095	0.3	2.9	3.2	10.8	O K
8640 min Winter	121.081	0.081	0.3	2.4	2.8	9.2	O K
10080 min Winter	121.072	0.072	0.3	2.1	2.4	8.2	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
30 min Winter	101.480	0.0	140.6	39
60 min Winter	61.702	0.0	171.0	68
120 min Winter	37.516	0.0	208.0	124
180 min Winter	28.043	0.0	233.2	180
240 min Winter	22.811	0.0	252.9	236
360 min Winter	17.051	0.0	283.6	338
480 min Winter	13.869	0.0	307.6	382
600 min Winter	11.817	0.0	327.6	460
720 min Winter	10.367	0.0	344.9	538
960 min Winter	8.418	0.0	373.4	692
1440 min Winter	6.277	0.0	417.6	990
2160 min Winter	4.681	0.0	467.1	1424
2880 min Winter	3.801	0.0	505.7	1848
4320 min Winter	2.641	0.0	527.1	2464
5760 min Winter	2.040	0.0	542.8	3000
7200 min Winter	1.670	0.0	555.3	3672
8640 min Winter	1.418	0.0	565.8	4408
10080 min Winter	1.234	0.0	574.7	5024

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Figure 15.3 (continued)
Summary of Design Calculations for Attenuation Basin



16.0 Catchment 6 – Slopes East - RTH Drainage Considerations

Catchment 6 is the Slopes East which has a surface area 4,476 m² (Fig. 16.1)

A significant drain for the surface runoff from the slopes east catchment is next to the base of the church steps (Fig. 16.1).

As previously mentioned for the other slopes catchments, it should be determined if storm runoff flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.

Fig. 16.1
Catchment 6 – Slopes East

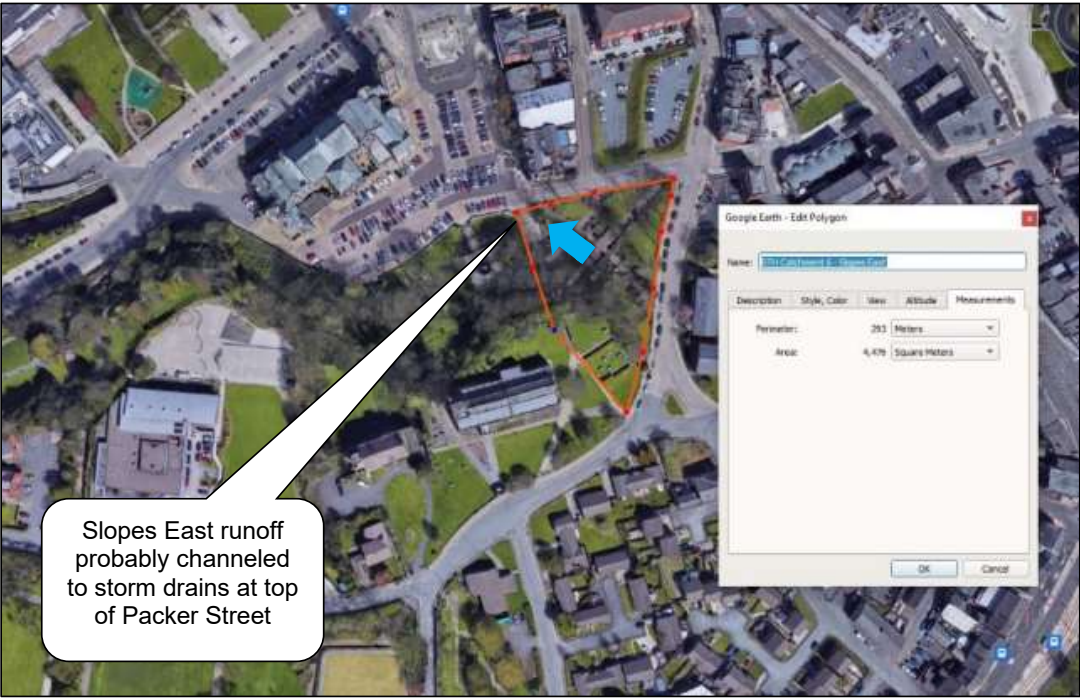


Fig. 16.2
Significant Drain for Catchment 6 – Slopes East Next to Church Steps



17.0 Summary of Conclusions & Recommendations

This section presents the outline plan for restoring the two historic water supply systems on the slopes (Packer Spout and Fulling Mill) and for generally improving the slopes drainage and helping to avoid the overwhelming of the new RTH drainage during storm runoff conditions

The 6 slope runoff catchments defined for this report are shown in Fig. 17.1.

Recommendations for the restoration of the Packer Spout Water Supply are listed in Fig. 17.2.

Recommendations for the restoration of the Fulling Mill Water Supply are listed in Fig. 17.3.

A Summary of RTH Slopes Catchments drainage problems and recommendations for remedies are listed in Fig. 17.4.

Figure 17.1
'3D' Aerial Photo Showing 6 Main Water Runoff Catchments for the RTH Slopes
(Blue arrows indicate main exit routes for runoff from catchments)

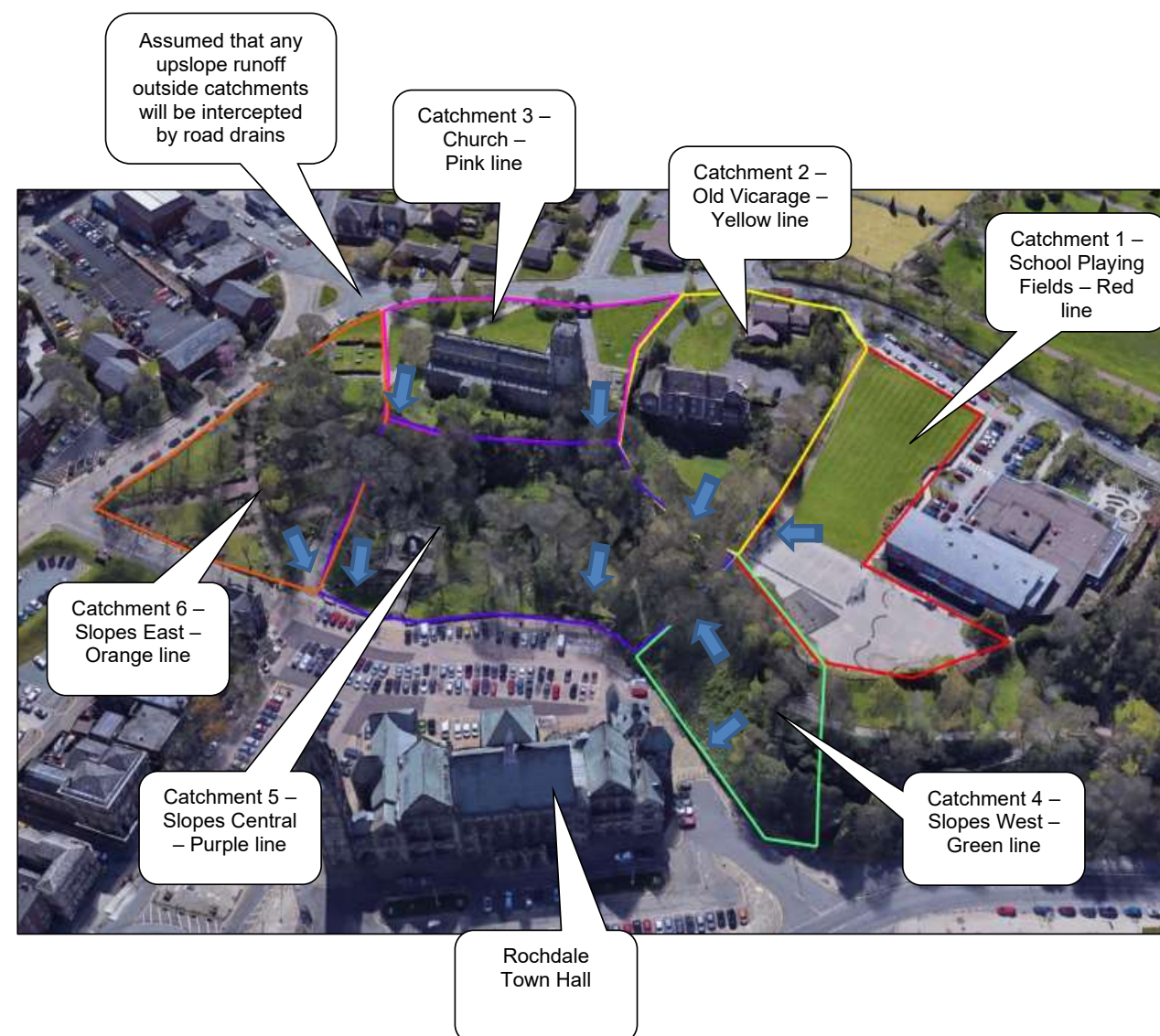


Figure 17.2
Recommendations for Restoration of the Packer Spout Water Supply

Item No.	Description
1	Restoring the catchpit for the main spring source so that sand is trapped in the catchpit and the spring water flows directly into half-round culvert and so removing the pond
2	Restoring the other 5 spring catchpits which can also act as sand/silt traps
3	Removing the back plastic pipes and other debris from the half round pipes
4	Refilling the round holes in the main west-east half-round culvert with lime mortar to stop water leaking to the slopes
5	Recovering the half-round culverts with the 90 cm natural stone slabs
6	Restoring the round brick sand/silt trap at the eastern end of the main half-round culvert
7	Restoring the stone culvert from the round brick chamber to the Packer Spout reservoir
8	Cleaning out the Packer Spout reservoir
9	Restoring the inflow and outflow controls for the Packer Spout reservoir
10	Laying new pipe from the Packer Spout Reservoir to the new RTH drainage to avoid current leaking drainage pipes
11	Determine if flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
12	Prepare maintenance plan including prevention of build-up of sand, silt and algae.

RTH Initial Hydrology Studies – Final Report

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Rev 00. August 2022.

Page 117 of 118

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Figure 17.3

Recommendations for Restoration of the Fulling Mill Water Supply

Item No.	Description
1	Restoring the complete length of the stone culvert from the western side of St Chad's church to the underground brick chamber (which is 11 m from the base of the slope)
2	Restoring the system catchpits and culverts which is draining the lower spring line and channelling water to the underground brick chamber.
3	Determine if flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
4	Prepare maintenance plan

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Page 118 of 118

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Figure 17.4

Summary of RTH Slopes Catchments Drainage Problems and Remedy Recommendations

Catchment No.	Catchment Name	Problems	Remedy
1	School Playing Fields	Possible uncontrolled runoff from the school playing fields is possibly reaching the RTH slopes via the vicarage garden	If confirmed, divert runoff away from RTH slopes or if required install flow attenuation prior to water reaching RTH slopes
2	Old Vicarage (Partnership House)	Foul water from vicarage probably reaching RTH slopes combined with runoff	If confirmed, connect vicarage to mains drains for install PWTP
		Possible uncontrolled runoff from the vicarage is reaching the RTH slopes via the vicarage garden	If confirmed, install flow attenuation if required prior to water reaching RTH slopes
3	Church	Foul water from church probably reaching RTH slopes combined with runoff	If confirmed, connect church to mains drains for install PWTP
		Uncontrolled runoff from the church is reaching the RTH slopes	Install flow attenuation if required at the base of the slopes
4	Slopes West	Unknown extent and condition of drains on these very steep slopes	Monitor drainage of catchment. Avoid damage to any drains during new planting
5	Slopes central	Blocked lateral field drains at base of slope	The lateral land drains at the base of the slope need to be replaced, with the land drain gravel covered by a permeable geotextile membrane to prevent silt blocking the land drain gravel.
			Runoff attenuation should be installed if required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.
6	Slopes East	Unknown extent and condition of drains on these very steep slopes	Investigate condition of significant drain for the surface runoff next to the base of the church steps
			Determine if storm runoff flow attenuation is required based on the relative flow capacity of the new RTH drainage compared with the slopes storm runoff rates.

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