

STRUCTURAL ROOF LOADING APPRAISAL



SHAWCLOUGH COMMUNITY PRIMARY SCHOOL THRUM HALL LANE ROCHDALE OL12 6DE

CALCULATIONS CARRIED OUT IN ACCORDANCE WITH MCS STANDARDS

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NOTE: This report has been carried out in accordance with BS EN 1991-1 BRE, the Digest 489 (2014), MIS 3002 & specifically Sections 4.3.6 to 4.3.8 of the Guide to the Installation of Photovoltaic Systems as required.

1.0 SITE SURVEY INFORMATION

BMG Surveys Ltd have been commissioned to carry out a structural roof loading appraisal of the afore mentioned commercial property for its suitability for a Solar PV Installation. Our checks included an onsite inspection and following the collation of the required information a set of calculations have been carried out in respect of the existing roof loadings and the proposed loadings for a Solar PV installation.

Our on-site inspection was carried out on a cloudy day. Our inspections cover all areas of the property and particular attention was paid to the roof and its structure.

Our visual inspection was carried out of all visible aspects of the properties structure including walls & roof both externally and internally. It was found that the properties primary structure and roof structure look to be in good condition with no signs of failure or deflection.

We conclude that the onsite inspection of the property and our subsequent design checks within sections 2.0, 3.0 & 4.0 of this report confirm that the roof and supporting structure can safely accept the loadings from the proposed Solar PV installation and we have no concerns over the structures ability to accept the loadings.



2.0 APPLIED LOADING

In considering the applied loading we have designed as noted below:

Dead loads are based on the actual specified make up for the existing roof.

Imposed floor loads are based on the loadings within BS 6399 & Eurocode- 1 (BS EN 1991-1).

Wind loadings are calculated on a site-specific basis.

Applied loads are as follows:

EXISTING ROOF MAKE UP:

DEAD LOADS

Existing Membrane	= 0.05kN/m ²
Existing Insulation	= 0.39kN/m ²
Existing Metal Sheets	= 0.45kN/m ²
Existing Steel Frame	= 0.55kN/m ²

Total DL = 1.44kN/m

IMPOSED LOADS

CP3 Chapter V or BS 6399:PT1:1984
(Building was constructed Prior to 1988.)

Roof Load = 0.75kN/m²

EXISTING ROOF WIND LOADINGS:

Calculated using TEDDS design software for both positive and negative internal pressure and for wind acting both perpendicular and parallel to the front elevation of the building.

EXISTING ROOF SNOW LOADINGS:

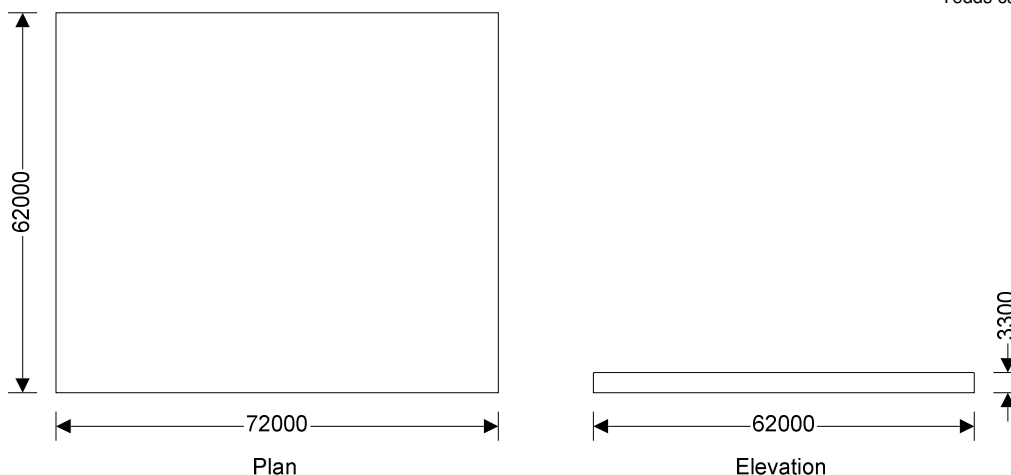
Calculated using TEDDS design software for both basic and where appropriate complex snow loadings.

CALCULATIONS:

WIND LOADING

In accordance with EN1991-1-4:2005+A1:2010 and the UK national annex

Tedds calculation version 3.0.28





Building data

Type of roof;	Flat
Length of building;	L = 72000 mm
Width of building;	W = 62000 mm
Height to eaves;	H = 3300 mm
Eaves type;	Sharp
Total height;	h = 3300 mm

Basic values

Location;	Manchester
Wind speed velocity (Figure NA.1);	$V_{b,map} = \mathbf{22.0}$ m/s
Distance to shore;	$L_{shore} = \mathbf{52.10}$ km
Altitude above sea level;	$A_{alt} = \mathbf{185.0}$ m
Altitude factor;	$C_{alt} = A_{alt}/1\text{m} \times 0.001 + 1 = \mathbf{1.185}$
Fundamental basic wind velocity;	$V_{b,0} = V_{b,map} \times C_{alt} = \mathbf{26.1}$ m/s
Direction factor;	$C_{dir} = \mathbf{1.00}$
Season factor;	$C_{season} = \mathbf{1.00}$
Shape parameter K;	$K = \mathbf{0.2}$
Exponent n;	$n = \mathbf{0.5}$
Air density;	$\rho = \mathbf{1.226}$ kg/m ³
Probability factor;	$C_{prob} = [(1 - K \times \ln(-\ln(1-p)))/(1 - K \times \ln(-\ln(0.98)))]^n = \mathbf{1.00}$
Basic wind velocity (Exp. 4.1);	$V_b = C_{dir} \times C_{season} \times V_{b,0} \times C_{prob} = \mathbf{26.1}$ m/s
Reference mean velocity pressure;	$q_b = 0.5 \times \rho \times V_b^2 = \mathbf{0.417}$ kN/m ²

Orography

Orography factor not significant;	$C_o = 1.0$
Terrain category;	Country
Displacement height (sheltering effect excluded);	$h_{dis} = 0$ mm

The velocity pressure for the windward face of the building with a 0 degree wind is to be considered as 1 part as the height h is less than b (cl.7.2.2)

The velocity pressure for the windward face of the building with a 90 degree wind is to be considered as 1 part as the height h is less than b (cl.7.2.2)

Peak velocity pressure - windward wall - Wind 0 deg and roof

Reference height (at which q is sought);	$z = \mathbf{3300}$ mm
Displacement height (sheltering effects excluded);	$h_{dis} = \mathbf{0}$ mm
Exposure factor (Figure NA.7);	$C_e = \mathbf{1.70}$
Peak velocity pressure;	$q_p = C_e \times q_b = \mathbf{0.71}$ kN/m ²

Structural factor

Structural damping;	$\delta_s = \mathbf{0.100}$
Height of element;	$h_{part} = \mathbf{3300}$ mm
Size factor (Table NA.3);	$C_s = \mathbf{0.836}$
Dynamic factor (Figure NA.9);	$C_d = \mathbf{1.000}$
Structural factor;	$C_s C_d = C_s \times C_d = \mathbf{0.836}$

Peak velocity pressure - windward wall - Wind 90 deg and roof

Reference height (at which q is sought);	$z = \mathbf{3300}$ mm
Displacement height (sheltering effects excluded);	$h_{dis} = \mathbf{0}$ mm

Exposure factor (Figure NA.7); $C_e = 1.70$
 Peak velocity pressure; $q_p = C_e \times q_b = 0.71 \text{ kN/m}^2$

Structural factor

Structural damping; $\delta_s = 0.100$
 Height of element; $h_{part} = 3300 \text{ mm}$
 Size factor (Table NA.3); $C_s = 0.845$
 Dynamic factor (Figure NA.9); $C_d = 1.000$
 Structural factor; $C_s C_d = C_s \times C_d = 0.845$

Peak velocity pressure for internal pressure

Peak velocity pressure – internal (as roof press.); $q_{p,i} = 0.71 \text{ kN/m}^2$

Pressures and forces

Net pressure; $p = C_s C_d \times q_p \times C_{pe} - q_{p,i} \times C_{pi}$
 Net force; $F_w = p_w \times A_{ref}$

Roof load case 1 - Wind 0, $C_{pi} 0.20$, $-C_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
F (-ve)	-2.00	0.71	-1.33	2.18	-2.89
G (-ve)	-1.40	0.71	-0.97	45.34	-44.04
H (-ve)	-0.70	0.71	-0.56	190.08	-105.79
I (-ve)	-0.20	0.71	-0.26	4226.40	-1099.79

Total vertical net force; $F_{w,v} = -1252.51 \text{ kN}$

Total horizontal net force; $F_{w,h} = 0.00 \text{ kN}$

Walls load case 1 - Wind 0, $C_{pi} 0.20$, $-C_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
A	-1.20	0.71	-0.85	4.36	-3.71
B	-0.80	0.71	-0.62	17.42	-10.73
C	-0.50	0.71	-0.44	182.82	-80.08
D	0.70	0.71	0.27	237.60	64.90
E	-0.30	0.71	-0.32	237.60	-75.91

Overall loading

Equiv leeward net force for overall section; $F_l = F_{w,wE} = -75.9 \text{ kN}$
 Net windward force for overall section; $F_w = F_{w,wD} = 64.9 \text{ kN}$
 Lack of correlation (cl.7.2.2(3) – Note); $f_{corr} = 0.85$; as h/W is 0.053
 Overall loading overall section; $F_{w,D} = f_{corr} \times (F_w - F_l + F_{w,h}) = 119.7 \text{ kN}$

Roof load case 2 - Wind 0, $C_{pi} -0.3$, $+C_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
F (+ve)	-2.00	0.71	-0.97	2.18	-2.12

G (+ve)	-1.40	0.71	-0.62	45.34	-27.98
H (+ve)	-0.70	0.71	-0.20	190.08	-38.45
I (+ve)	0.20	0.71	0.33	4226.40	1399.23

Total vertical net force; $F_{w,v} = 1330.68 \text{ kN}$

Total horizontal net force; $F_{w,h} = 0.00 \text{ kN}$

Walls load case 2 - Wind 0, $c_{pi} -0.3$, $+c_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
A	-1.20	0.71	-0.50	4.36	-2.17
B	-0.80	0.71	-0.26	17.42	-4.56
C	-0.50	0.71	-0.08	182.82	-15.31
D	0.70	0.71	0.63	237.60	149.07
E	-0.30	0.71	0.03	237.60	8.26

Overall loading

Equiv leeward net force for overall section;

$$F_l = F_{w,wE} = 8.3 \text{ kN}$$

Net windward force for overall section;

$$F_w = F_{w,wD} = 149.1 \text{ kN}$$

Lack of correlation (cl.7.2.2(3) – Note);

$$f_{corr} = 0.85; \text{ as } h/W \text{ is } 0.053$$

Overall loading overall section;

$$F_{w,D} = f_{corr} \times (F_w - F_l + F_{w,h}) = 119.7 \text{ kN}$$

Roof load case 3 - Wind 90, $c_{pi} 0.20$, $-c_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
F (-ve)	-2.00	0.71	-1.34	2.18	-2.92
G (-ve)	-1.40	0.71	-0.98	38.74	-37.95
H (-ve)	-0.70	0.71	-0.56	163.68	-91.76
I (-ve)	-0.20	0.71	-0.26	4259.40	-1113.35

Total vertical net force; $F_{w,v} = -1245.98 \text{ kN}$

Total horizontal net force; $F_{w,h} = 0.00 \text{ kN}$

Walls load case 3 - Wind 90, $c_{pi} 0.20$, $-c_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
A	-1.20	0.71	-0.86	4.36	-3.75
B	-0.80	0.71	-0.62	17.42	-10.81
C	-0.50	0.71	-0.44	215.82	-95.16
D	0.70	0.71	0.28	204.60	56.72
E	-0.30	0.71	-0.32	204.60	-65.72

Overall loading

Equiv leeward net force for overall section;

$$F_l = F_{w,wE} = -65.7 \text{ kN}$$

Net windward force for overall section;

$$F_w = F_{w,wD} = 56.7 \text{ kN}$$

Lack of correlation (cl.7.2.2(3) – Note);

$f_{corr} = 0.85$; as h/L is 0.046

Overall loading overall section;

$F_{w,D} = f_{corr} \times (F_w - F_l + F_{w,h}) = 104.1 \text{ kN}$

Roof load case 4 - Wind 90, $c_{pi} -0.3$, $+c_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
F (+ve)	-2.00	0.71	-0.98	2.18	-2.14
G (+ve)	-1.40	0.71	-0.63	38.74	-24.23
H (+ve)	-0.70	0.71	-0.21	163.68	-33.78
I (+ve)	0.20	0.71	0.33	4259.40	1415.12

Total vertical net force;

$F_{w,v} = 1354.97 \text{ kN}$

Total horizontal net force;

$F_{w,h} = 0.00 \text{ kN}$

Walls load case 4 - Wind 90, $c_{pi} -0.3$, $+c_{pe}$

Zone	Ext pressure coefficient C_{pe}	Peak velocity pressure q_p , (kN/m ²)	Net pressure p (kN/m ²)	Area A_{ref} (m ²)	Net force F_w (kN)
A	-1.20	0.71	-0.51	4.36	-2.20
B	-0.80	0.71	-0.27	17.42	-4.64
C	-0.50	0.71	-0.09	215.82	-18.71
D	0.70	0.71	0.63	204.60	129.20
E	-0.30	0.71	0.03	204.60	6.75

Overall loading

Equip leeward net force for overall section;

$F_l = F_{w,wE} = 6.8 \text{ kN}$

Net windward force for overall section;

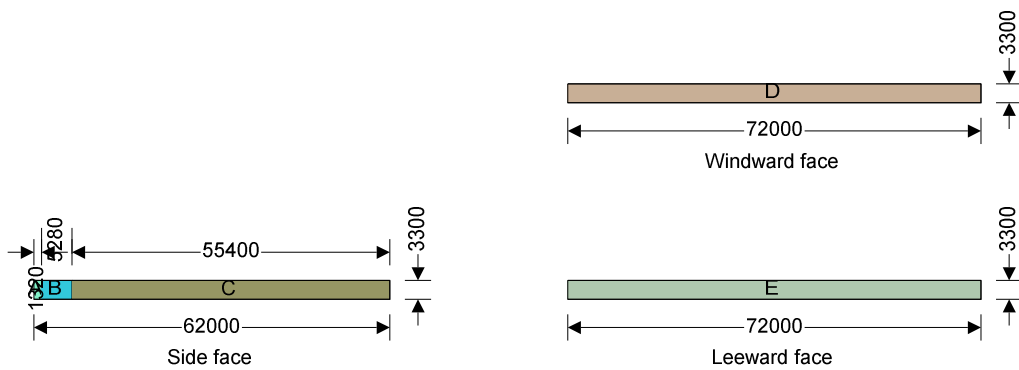
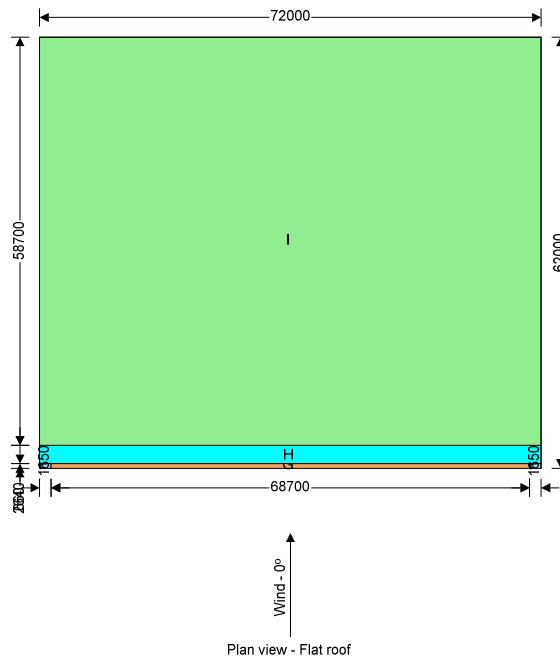
$F_w = F_{w,wD} = 129.2 \text{ kN}$

Lack of correlation (cl.7.2.2(3) – Note);

$f_{corr} = 0.85$; as h/L is 0.046

Overall loading overall section;

$F_{w,D} = f_{corr} \times (F_w - F_l + F_{w,h}) = 104.1 \text{ kN}$



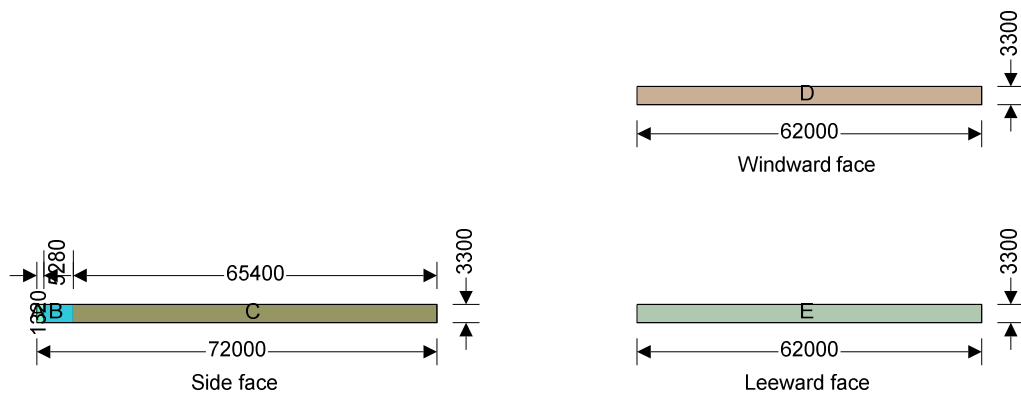
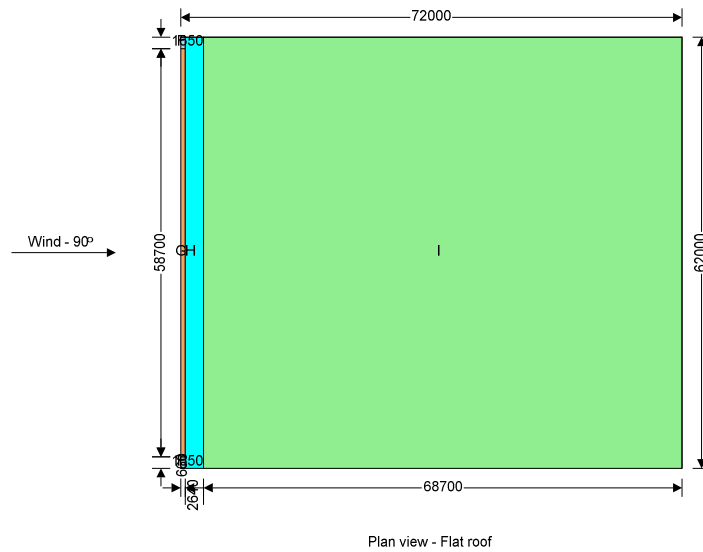
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3.0 JUSTIFICATION OF PANELS FOR GRAVITY LOADINGS

From the calculated loads we see that each panel weighs 21kg and is 1754mm by 1096mm.

Therefore the weight per m² = 10.9kg/m².

The support frame weighs 2kg/m²

The allowable vertical imposed load is 0.75kN/m² or 75kg/m² which is far more than the weight of the panel being placed on the roof.

Once the panel is in situ this area of roof will not be trafficked and so there is no need to consider the actual weight of the panel as being an additional imposed load on the roof. Should anyone stand on the panel it will destroyed, the owner of the property will therefore take strict steps to ensure that no one at any time stands on the panel. Therefore this area of roof can be considered as carrying less than the design-imposed load indicated in BS EN 1991-1. Therefore there is no requirement for strengthening as a result of combined imposed load and panel load.

The combined loadings on the roof during the installation of the solar panels are within the allowable loading limits and we consider these combined loads to be acceptable.

With regards snow loading we see that the snow load is 0.66kN/m². This load will be cumulative to the weight of the panel. However, with the panel and frame weighing 12.9kg/m² this combined loading is equal to or less than the design-imposed load of 0.60kN/m².

The combined snow and panel load would therefore require no additional strengthening works in order to carry this increase in load. The existing roof structure is therefore adequate as it stands at present.

SNOW LOADING

In accordance with EN1991-1-3:2003+A1:2015 incorporating corrigenda dated December 2004 and March 2009 and the UK national annex NA+A1:2015 to BS EN 1991-1-3:2003+A1:2015 incorporating Corrigendum No.1

Tedds calculation version 1.0.14

Characteristic ground snow load

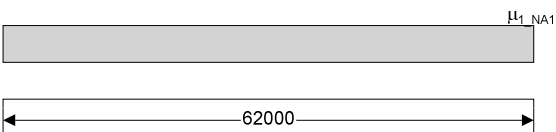
Location;	Manchester
Site altitude above sea level (user modified value);	A = 185 m
Zone number;	Z = 3.0
Density of snow;	$\gamma = 2.00$ kN/m ³
Characteristic ground snow load;	$s_k = ((0.15 + (0.1 \times Z + 0.05)) + ((A - 100m) / 525m)) \times 1kN/m^2 = 0.66$ kN/m ²
Exposure coefficient (Normal);	C _e = 1.0
Thermal coefficient;	C _t = 1.0
Snow fence;	Not present

Building details

Roof type;	Flat
Width of roof;	b = 62.00 m

Shape coefficients

Shape coefficient roof (Table UK NA.1);	$\mu_{1_NA1} = 1.00$
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Case (i)		Shape coef μ_{1_NA1}	Coef 1.000	Loading (kN/m ²) 0.66
	Elevation on gable roof			

Loading to roof 1;

$$s_{1_1} = \mu_{1_NA1} \times C_e \times C_t \times s_k = \mathbf{0.66 \text{ kN/m}^2}$$

Snow Drift

The proposed Solar PV panels are not located next to or near any areas where snow drift is likely to occur therefore snow drift is not considered a potential issue on this project.



4.0 JUSTIFICATION OF PANELS FOR UPLIFT LOADINGS

From the calculations we see that the panels should ideally be placed within the central zones of the roof where there is little or no wind uplift.

Given that the panel fixings will transfer the load into the existing roof and the roof was originally designed for this wind load, no strengthening works will be required to the roof structure.

To calculate the actual wind uplift on the PV Array we refer to BRE Digest 489.

From our calculations above we know that $q = 0.81\text{kN/m}^2$ and where a module is less than 0.3m from the roof surface the Wind Uplift Net Pressure Coefficients for the panels in the centre of the roof is -1.50

$$\Rightarrow -0.71 \times -1.50 = -1.07\text{kN/m}^2 \text{ (All roof fixings have to be able to withstand this wind uplift load.)}$$

5.0 CONCLUSIONS

From the calculations we see that the combined system load (0.13kN/m^2) & snow load (0.66kN/m^2) = (0.79kN/m^2) is in excess of the design-imposed load of 0.75kN/m^2 . This 0.04kN/m^2 loading equates to around 3% exceedance of the design-imposed load.

Given the loading factors of safety built into the structure that the loading exceedance may only occur for short periods of adverse weather we do not consider the additional loadings an issue and confirm that the proposed solar panels can safely be fixed to the existing roof structure with no strengthening works being required.

Appendix A Photographs











